

**Qualifications Due**  
**On March 15, 2023 @ 2:00 P.M.**

**Advertisement**

**Request for Qualifications (RFQ) To Provide Engineering Services  
Chattahoochee River Pump Station & Raw Water Transmission Main**

The Coweta County Water & Sewerage Authority, as "Owner", is seeking Statements of Qualifications (SOQ) from firms interested in providing Engineering Services. This RFQ is issued to identify the most qualified potential providers. Firms who are determined by the Owner to be especially qualified may be deemed eligible and may be invited to discuss and negotiate a contract for these services. All respondents to the RFQ are subject to instructions communicated in this document and additional terms and conditions listed in the Owner's RFQ. The Owner reserves the right to reject any or all statements of qualifications, and to waive technicalities and informalities at the discretion of the Owner.

The RFQ Package may be obtained at Coweta County Water & Sewerage Authority, 545 Corinth Rd, Newnan, Georgia 30263. Statements of Qualifications must be delivered to the Owner at the listed address and should be delivered in accordance with instructions in the RFQ package.

**Restriction of Communication:** From the issue date of this RFQ until a successful proposer is selected, proposers are not allowed to communicate for any reason with any members of the Selection Committee or the Owner's staff, except for:

- a) Submission of questions as instructed in the RFQ,
- b) As provided by any existing work agreement(s).

For violation of this provision, the Owner reserves the right to reject the proposal of the offending proposer.

Written questions may be directed to Mr. Rick Jones via email at [rjones@cowetawater.com](mailto:rjones@cowetawater.com). Questions will be answered via written addendum only. The deadline for submitting questions is March 3, 2023. The Owner reserves the right to reject any and all statements of qualifications, as well as to waive technicalities and informalities.

Portions of this and future Work may be funded, in part, by Drinking Water SRF Loan No. DW-2022030 from the Georgia Environmental Finance Authority (GEFA). It is the policy of the Owner and GEFA that small businesses, female-owned businesses and minority businesses have a fair and equal opportunity to participate in these opportunities. Contractors and subcontractors who utilize qualified minority subcontractors may qualify for a state income tax credits for qualified payments made to minority subcontractors. See Official Code of Georgia Annotated (O.C.G.A.) Section 48-7-38.

Coweta County Water & Sewerage Authority  
545 Corinth Rd.  
Newnan, Georgia 30263  
Phone: (770) 254-3710

**Coweta County Water & Sewerage Authority**

**Request for Qualifications to Provide**

**Engineering Services**

**For**

**Chattahoochee River Pump Station & Raw Water Transmission Main  
Coweta County, Georgia**

# **Chattahoochee River Pump Station & Raw Water Transmission Main**

The Coweta County Water & Sewerage Authority, (“Owner”), solicits Statements of Qualifications (SOQ) from professional firms interested in providing Engineering Services for the above-mentioned services. One or more firms that respond to this RFQ and who are determined by the Owner to be qualified, may be deemed eligible and may be invited to discuss and negotiate for these services. All respondents to this RFQ are subject to instructions communicated in this document and are cautioned to completely review the entire RFQ and follow instructions carefully. The Owner reserves the right to reject any or all statements of qualifications or proposals, and to waive technicalities and informalities at the discretion of the Owner.

Please note: The issuance of this RFQ invokes a Restriction of Communication on potential respondents, which, if violated, may result in proposal rejection (See Section 8 for details).

## **1. General Project Information**

### **1.1 - Project Description**

The Authority has submitted a withdrawal permit application to the Georgia Environmental Protection Division (EPD). Communications indicate a draft permit will be issued for 27.5 MGD raw water withdrawal on the Chattahoochee River. The raw water is intended to be stored in B.T. Brown Reservoir. The Authority seeks an engineering firm to design a new river intake on the Chattahoochee River south of Highway 27, and a raw water pipeline to deliver water to the B.T. Brown Reservoir.

The construction delivery method for this Project will be traditional design, bid, and construct. The Owner will employ the services of a General Contractor in addition to the Engineering Consultant.

### **1.2 - Project Goals**

The successful Project will provide a design to withdraw water from the Chattahoochee River and pump to the B.T. Brown reservoir. A Basis of Design Report (BODR) was prepared for the draft permit submission to GA EPD and property acquisition and has been included as Attachment “A”. The Authority is seeking an engineer to continue preliminary designs to final designs, then prepare construction documents consisting of drawings, technical specifications and contracting documents.

### **1.3 - Project Schedule**

The Coweta County Water & Sewerage Authority anticipates the completion of the improvements requested herein as follows:

Notification of Award to the Selected Consultant	April 14, 2023
Notice to Proceed to Selected Consultant	April 27, 2023
EPD Approved Plans Ready to Advertise	June 3, 2024
Notice to Proceed to Construction Contractor	August 2, 2024
Construction Completion	August 5, 2026

All of the dates above are estimates and subject to change.

## 2. RFQ Schedule Of Events

The following Schedule of Events represents the Owner's best estimate of the schedule that will be followed. All times indicated are prevailing times in Georgia. The Owner reserves the right to adjust the schedule as necessary.

a. Public advertisement of <b>RFQ</b> for a period of 30 days	02/14/2023	-----
b. Deadline for written questions/requests for clarification ( <i>see section 6</i> )	03/03/2023	5:00 PM
c. Deadline for submission of Statements of Qualifications ( <i>see sections 5, 7</i> )	03/15/2023	2:00 PM
d. Evaluation and issues invitation for interview for Finalist firms	03/29/2023	-----
e. Selection Committee interviews finalist firms	04/12/2023	TBA
f. Notification to Selected Firm	04/14/2023	TBA
g. Notice to Proceed	04/27/2023	TBA

## 3. Engineering Consultant Requirements

The successful firm will demonstrate experience in the successful completion of projects of similar size and equipment. Important criteria in the selection for this project will include the following:

- Demonstrate the firm and project specific personnel have the technical ability to perform the required work to properly execute the scope of the project. This ability includes a detailed description of all disciplines available to the firm including in-house assets as well as sub-consultants required to provide the services.
- A description and detailed schedule of how the project will be executed.
- Resumes and experience of Project Staff proposed to perform the requested Engineering and Project Management for this project. An organizational chart showing the Project staff and the role that each is expected to perform.
- The firm should list experience with the design of at least three projects of similar scope and scale.
- Experience and knowledge of Local, Georgia Environmental Protection Division (GA EPD), and U.S. Army Corp of Engineers (USACE) 404 Permitting issuances.
- The firm's approach to developing and implementing a design and construction quality control program.
- The selected firm will be required to provide full time resident inspection on site for the duration of the construction. The inspector shall be an employee of the selected firm.



## **4. Instructions for Preparing Statements of Qualifications**

Each Statement of Qualifications shall provide a straightforward, concise delineation of respondent's capabilities. Fancy, colored displays, and promotional materials are not desired. Emphasis must be on completeness, relevance, and clarity of content. The content of all Statements of Qualifications must be categorized and numbered as outlined below, and responsive to all requested information:

### **4.1 - Description and Resources of Firm**

- Provide basic company information: Company name, address, name of primary contact, telephone number, email address, and company website. If the firm has multiple offices, the qualification statement shall include information about the parent company and branch office separately. Identify office from which project will be managed and this office's proximity to the project site. Provide form of ownership, including state of residency or incorporation, and number of years in business. Is the offeror a sole proprietorship, partnership, corporation, Limited Liability Corporation (LLC), joint venture, or other structure?
- Briefly describe the history and growth of your firm(s). Provide general information about the firm's personnel resources, including disciplines and numbers of employees and locations and staffing of offices.
- Provide a statement of disclosure, which will allow the Owner to evaluate possible conflicts of interest. Respondents must provide, in their own format, a statement of all potential legal or otherwise significant conflicts of interests possibly created by the respondent's being considered in the selection process or by the respondent's involvement in the project. Respondents should include information as to the nature of relationship(s) with parties in such potential conflict.
- Has the firm ever been removed from a Service Contract, had a contract terminated for default, or failed to complete a contract as assigned?

### **4.2 – Project Understanding and Approach**

- Provide a narrative description of the project based on your understanding of all the aspects of the project. Include any issues that you believe will required special consideration.
- Provide a description and detailed schedule of how the work will be performed.
- Discuss the firm's approach to Quality Assurance and Quality Control with specific reference to implementation of QA/QC procedures on this project.

### **4.3 - Experience and Qualifications**

- Provide professional qualifications and description of experience for principal Consulting Services personnel. Firms that respond as joint teams will be expected to clearly demonstrate their team members' shared experience, as a joint team, on prior projects of similar magnitude and complexity.
- Provide information on the firm's A/E Consulting Services experience on projects of similar type, size, function, and complexity. Describe no more than and no less than three projects completed in the last 10

years, in order of most relevant to least relevant, which demonstrate the firm's capabilities to perform the project at hand. For each project, the following information should be provided:

- a. Project name, location, and dates during which services were performed.
  - b. Brief description of project and physical description (buildings, square footage, number of stories, site areas).
  - c. Services performed by your firm.
  - d. Owner contact information including email and telephone number.
- Describe the firm's experience and qualifications in a leadership role over multiple providers and phases of construction for similar designs. Include any oversight of projects of relevant complexity, including experience in providing leadership in projects that utilize similar building methods and applications to that of incumbent project. Include any certifications, industry ratings, and achievement recognitions, etc., to attest to the level of experience and success. Describe innovations that the firm might have introduced or employed to increase the project's adherence to technical standards.

#### **4.4 - Statement of Suitability**

- Provide any information that may serve to differentiate the firm from other firms in suitability for the project. Suitability may include, but is not limited to, the firm's fit to the project and/or needs of the Owner, any special or unique qualifications for the project, current and projected workloads, the proximity of office to project location, and any services offered by the firm that may be particularly suitable for this project.
- Provide any non-discrimination and affirmative action policies of the firm and the firm's history of inclusion in sourcing for projects.

#### **5. Submittal of Statements of Qualifications**

Five bound Statements of Qualifications must be delivered to the Owner at the listed address and should be delivered in accordance with instructions in the RFQ package prior to the submittal deadline.

- Provide one original, signed in wet ink, with 4 copies stamped "COPY."
- Include 1 published copy saved to a memory stick.
- Bind submittals in 1 or 1-1/2-inch, 3-ring binders with dividers separating the Sections, as needed.
- Publish pages on standard 8 1/2 x 11-inch paper; number each page.
- Include a table of contents with corresponding tabs to identify Sections.
- Limit SOQs to 30 pages or less using minimum 11-point font.
- Up to two 11" x 17" folded pages may be used for the schedule referenced in Section 3.
- 11 x 17-inch pages shall count as 1 page each.
- Resumes, exhibits, affidavits, or other enclosure information called for may be included in an appendix and will not count toward the page limit.

SOQs must be received by the Owner by the deadline in the Schedule of Events at the Customer Service Counter at the following address:

Mr. Rick Jones, COO  
Coweta County Water & Sewerage Authority  
545 Corinth Rd.  
Newnan, GA 30263

## **6. Selection Process**

A Selection Committee, consisting of representatives of the Owner, will identify a selection of finalist firms through a QBS process. The Selection Committee will receive and evaluate SOQs submitted in response to this RFQ using the following criteria:

### **6.1 - Criteria for evaluation of Statements of Qualifications**

- 25% Description and Resources of Firm – Stability of the firm, including the firm’s corporate history, growth, resources, form of ownership, financial information, and other evidence of stability.
- 30% Project Understanding and Approach – Does the firms Statement of Qualifications demonstrate a firm understanding of the proposed project? Is the approach to the overall design and construction of the project consistent with the needs of the Authority?
- 25% Experience and Qualifications - Including the demonstrated ability of firm in effective Consulting Services of facilities comparable in complexity, size, and function meet the Engineering Consultant Requirements as outlined. This includes relevant experience and qualifications of the principal Consulting Services lead staff and level of experience in a leadership role over multiple providers and phases of construction for this type of project.  
  
This item to also consider the Firm’s apparent suitability to provide services for Project, including the firm’s apparent fit to the project type and/or needs of the Owner, any special or unique qualifications for the Project, current and projected workloads, proximity of office or lead staff to Project location, and services offered by the firm. The firm’s non-discrimination policies, any affirmative action policies and past efforts for W/MBE inclusion will be a part of this evaluation, as well as the firm’s record and methodology of addressing public safety and environmental concerns.
- 20% Statement of Suitability – Does the firm’s current workload and staffing allow for the firm to meet the schedule for the proposed project?

### **6.2 - Finalist Notification**

Up to three (3) finalist firms will be selected to interview. The names of the firms selected as finalists will be posted on the Georgia Procurement Registry. The firms selected as finalists will receive written notification from the Owner which will address the necessary elements of the remainder of the selection process. These elements include, but may not be limited to, the following:

### **6.3 - Interview**

Each finalist firm shall be informed of the place and time for the interview session. The time allotted to each firm will not exceed 60 minutes to include: 5 minutes for setup, 25 minutes for proposer presentation, 25 minutes for Committee questions, and 5 minutes for knockdown. Although the Owner will have a screen and projector available in the interview room, presenter must be prepared with own projector and laptop for quick setup within the allotted 5 minutes as a backup. The remainder of the presentation may involve flip charts or boards along with oral presentation. Additional interview instructions and guidelines will be provided in the Notice to Finalists, as well as the criteria which will be used to evaluate the interview.

## **6.4 - Consultant Selection**

Upon completion of the evaluation and interview process by the Selection Committee, the firms will be ranked using criteria described herein in descending order of recommendation. Negotiations may then be initiated with the highest-ranking firm to finalize the terms and conditions of the contract, including the fees to be paid. In the event a satisfactory fee agreement cannot be reached with the highest-ranking firm, the Owner will formally terminate the negotiations in writing and enter negotiations in turn with the second highest ranking firm, and so on, until a mutual agreement is established and the Owner awards a contract. The actual Form of Contract shall be developed later between the Owner and Consultant, after selection.

## **7. Submittal of Questions and Requests for Clarification**

Questions about any aspect of the RFQ, or the project, must be submitted in writing (email is preferable) to: Mr. Rick Jones, Chief Operating Officer: [rjones@cowetawater.com](mailto:rjones@cowetawater.com). The deadlines for submission of questions relating to the RFQ are the times and dates shown in the RFQ Schedule of Events. Proposers are cautioned to review and adhere to the Restriction of Communication for this Project.

## **8. Additional Terms and Conditions**

### **8.1 - Restriction of Communication**

From the issue date of this (RFQ) solicitation until a successful proposer is selected and the selection is announced, proposers are not allowed to communicate about this solicitation or this Project for any reason with any members of the Selection Committee, the Owner, except for submission of questions as instructed in the RFQ, or during the proposer's conference (if applicable), or as provided by any existing work agreement(s). For violation of this provision, the Owner reserves the right to reject the proposal of the offending proposer.

### **8.2 - Submittal Costs and Confidentiality**

All expenses for preparing and submitting responses are the sole cost of the party submitting the response. The Owner is not obligated to any party to reimburse such expenses. All submittals upon receipt become the property of the Owner. Labeling information provided in submittals "proprietary" or "confidential", or any other designation of restricted use will not protect the information from public view.

Subject to the provisions of the Georgia Open Records Act, the details of the proposal documents will remain confidential until final award.

### **8.3 - Award Conditions**

This request is not an offer to contract or a solicitation of bids. This request and any statement of qualifications or proposal submitted in response, regardless of whether the proposal is determined to be the best proposal, is not binding upon the Owner and does not obligate the Owner to procure or contract for any services. Neither the Owner nor any party submitting a response will be bound unless and until a written contract mutually accepted by both parties is negotiated as to its terms and conditions and is signed by the Owner and a party containing such terms and conditions as are negotiated between those parties. The Owner reserves the right to waive non-compliance with any requirements of this Request for Proposal and to reject any or all proposals submitted in responses. Upon receipt and review of responses, the Owner will determine the party(s) and proposal that in the sole judgment of the Owner is in the best interest of the Owner (if any is so determined), with respect to

the evaluation criteria stated herein. The Owner then intends to conduct negotiations with such party(s) to determine if a mutually acceptable contract may be reached.

#### **8.4 - Statement of Agreement**

With submission of a statement of qualifications or proposal, the Proposer agrees that the firm has carefully examined the solicitation, and the Proposer agrees that it is the Proposer's responsibility to request clarification on any issues in any section of the solicitation with which the Proposer disagrees or needs clarified. The Proposer also understands that failure to mention these items in the proposal will be interpreted to mean that the Proposer is in full agreement with the terms, conditions, specifications, and requirements therein. With submission of a proposal, the Proposer hereby certifies: (a) that this proposal is genuine and is not made in the interest or on behalf of any undisclosed person, firm, or corporation; (b) that Proposer has not directly or indirectly included or solicited any other Proposer to put in a false or insincere proposal; (c) that Proposer has not solicited or induced any person, firm, or corporation to refrain from sending a proposal.

# **CHATTAHOOCHEE RIVER INTAKE FACILITY, PUMP STATION AND RAW WATER PIPELINE BASIS OF DESIGN REPORT**

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**Prepared by:**



**200 Galleria Drive, SE  
Suite 520  
Atlanta, GA 30339**

**September 2022**

**Garver Project No.: 22W38085**



### **Engineer's Certification**

I hereby certify that this Basis of Design Report for the Chattahoochee River Intake Pump Station was prepared by Garver under my direct supervision for Coweta County Water & Sewerage Authority.

A handwritten signature in blue ink that reads "Andy Pruitt".

---

Andy Pruitt, PE  
State of Georgia PE License 048205



Georgia State License Number:  
PEF007804  
Exp. 6/30/2024



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## **1.0 Introduction & Project Background**

### **1.1 Project Scope & Need**

To provide resiliency to the region and support future projected water demands, the Coweta County Water and Sewer Authority (CCWSA) is moving forward with the design of a new river intake system. This system will route raw water from the Chattahoochee River to the B.T. Brown Reservoir, where it can be treated and distributed through the B.T. Brown Water Treatment Plant. The river intake system includes a river-side intake screen structure, a collection wet well, a pump station, and a 9.6-mile-long raw water transmission main.

CCWSA submitted a Municipal Surface Water Withdrawal Permit to the Georgia Environmental Protection Division (EPD) in May 2022. This Basis of Design Report is intended to support the permit by identifying key design criteria for the construction of a new river intake system, as well as documenting our process for initial equipment and structure design.

### **1.2 River Withdrawal Location Identification**

Originating from the southern Appalachian Mountains, and flowing south along the southwest Georgia state border, the Chattahoochee River represents one of the largest raw water sources in the state. The Chattahoochee River runs 434 miles across the state of Georgia before joining the Apalachicola River and discharging to the Gulf of Mexico.

The target intake section of the river in Coweta County has well-documented river flow data, with daily river levels published on the United States Geological Survey (USGS) website<sup>1</sup>. Historical gage heights from 1990 through 2022 were used to inform the design and were supported by FEMA (Federal Emergency Management Agency) floodplain maps for the region<sup>2</sup>.

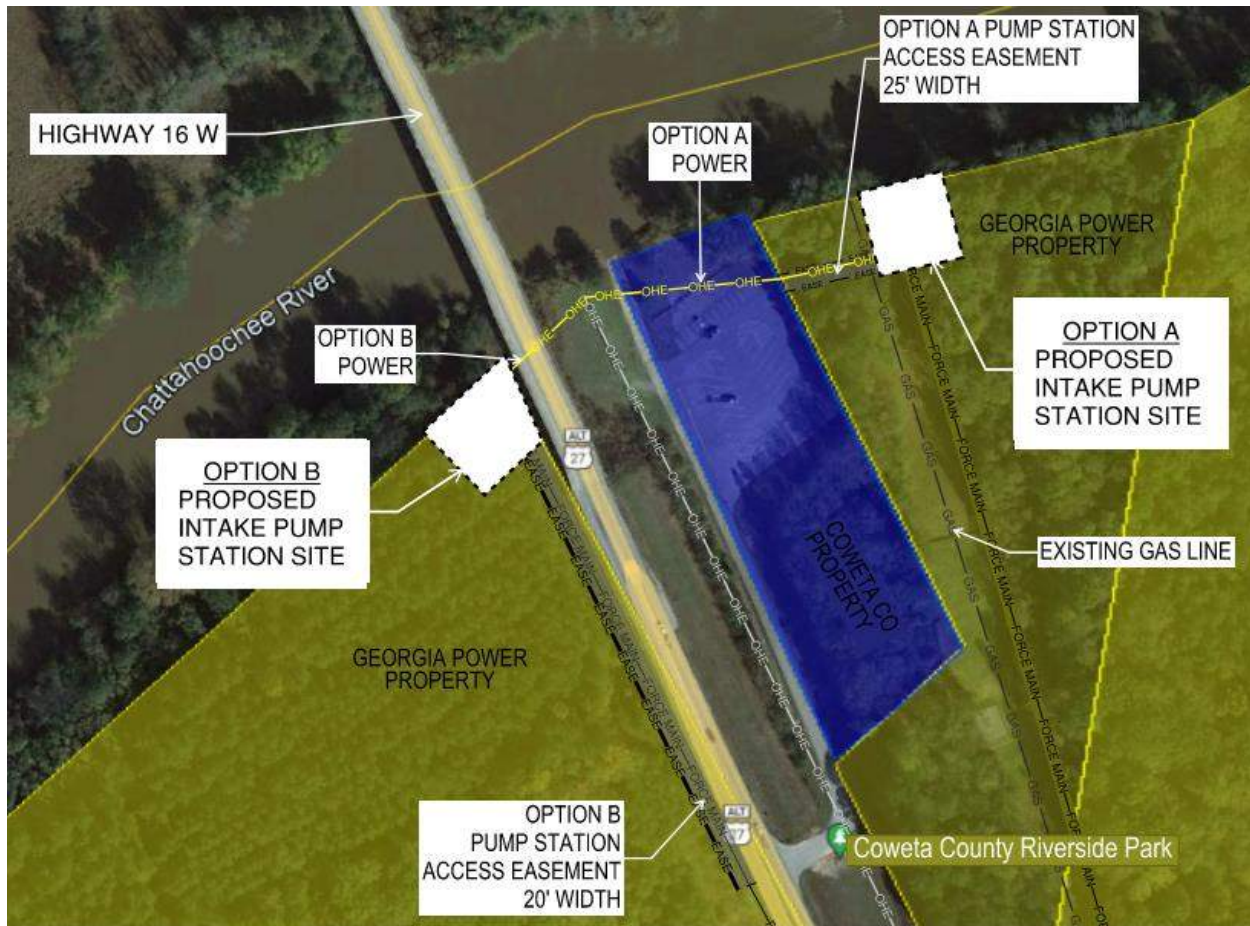
The 7Q10 is a hydrological design flow based on the lowest 7-day average flow that occurs on average, every 10 years. 7Q10 flowrates for the Coweta Intake were provided by the Georgia Department of Natural Resources and delineated by month of the year. According to this design flow, the 7Q10 ranges from a minimum of 948 cfs in November to a maximum flowrate of 1312 cfs in April.

Based on the results of a previous study performed by AECOM, a river intake location near the Highway 16 W bridge and Riverside Park was selected. Within this intake region, we identified two potential withdrawal locations owned by Georgia Power, shown in Figure 1-1. The first option withdraws from the river along the west side of Highway 16 W, while the second option withdraws from the east side. Following discussions with Georgia Power, the western parcel was selected as the preferred intake location. The site layout and building design will be informed by the topography around the intake location.

---

<sup>1</sup> USGS 02338000 Stream Data | Chattahoochee River near Whitesburg, GA  
[https://waterdata.usgs.gov/nwis/inventory/?site\\_no=02338000&agency\\_cd=USGS](https://waterdata.usgs.gov/nwis/inventory/?site_no=02338000&agency_cd=USGS)

<sup>2</sup> FEMA Flood Insurance Rate Map | Flood Map Service Center  
<https://msc.fema.gov/portal/search?AddressQuery=-84.9013025821908,%2033.47592936647495>



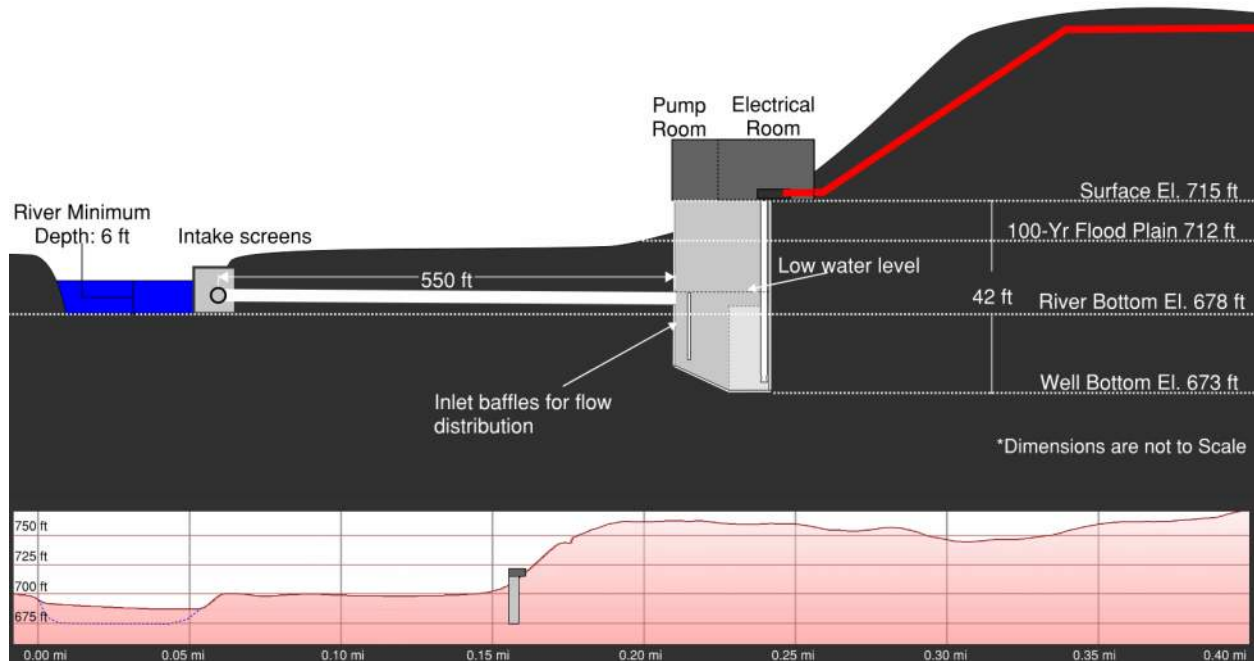
**Figure 1-1 Potential Intake Locations from the Chattahoochee River**

Two intake locations were considered along the Chattahoochee River. Option B was selected as the preferred location with Georgia Power, the property owner.

### 1.3 Location and Preliminary Layout

The site layout consists of three primary structures: the passive intake screens, the wet well/pump building, and the electrical building. The intake structure will be constructed along the edge of the Chattahoochee River, protecting the intake screens and directing water to the pump station. Based on the intake location selection described in the previous section, the structure will be built within the parcel of land west of Highway 16 W (Option B).

The passive intake screens direct water via gravity lines to the wet well and pump building. Because of the flat topography along the riverbank, the gravity lines will run 550 feet to a neighboring hill to keep the pump building out of the floodplain. The pump and electrical building floor and equipment will be elevated to 715 ft, three feet above the 100-year floodplain, with the structure partially cutting into the hillside. A profile view of this configuration is represented in Figure 1-2.

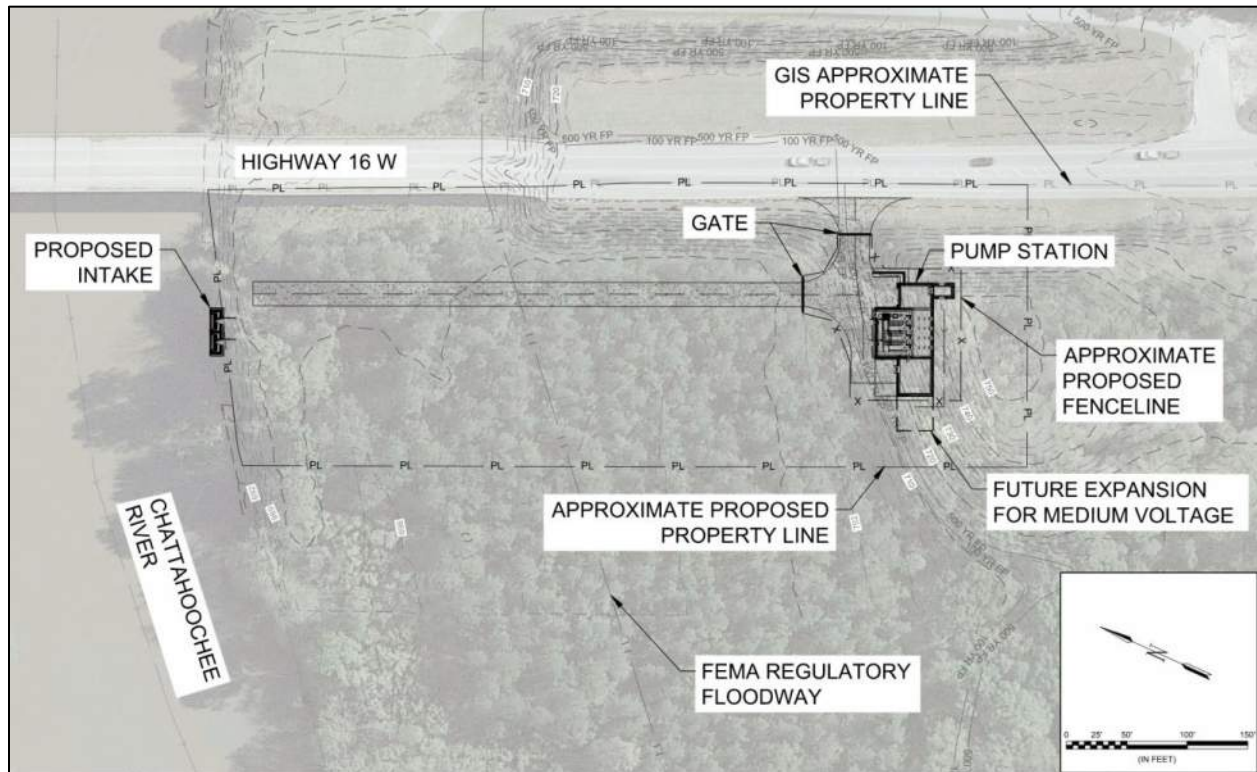


**Figure 1-2 Representative Profile of Pump Station Configuration**

The topography around the intake site is relatively flat before an abrupt rise in grade along the hillside. To minimize excavation volumes while also staying out of the floodplain, the pump station will be constructed within the hillside just above the floodplain.

An access driveway will be built along the edge of the highway, near a naturally flat region between the hillside and riverbank. The driveway will connect with a concrete pad around the pump station, providing access to the side of electrical room and to the unloading area in the pump building. A temporary construction road within the floodplain provides access to the intake structure and construction staging area. The site layout is shown in Figure 1-3.





**Figure 1-3 General Site Layout of Intake Structure and Pump Station**

The pump station will be accessible via a driveway along the western side of the highway. Temporary driveways provide access to the intake screens and construction staging area within the floodplain, while the pump station and unloading areas remain outside of the floodplain.

#### **1.4 Permitting Process**

Because of the unique placement of the intake structure along a major river and within the floodplain, several permits must be submitted to support the project. The major permits and requirements include:

- Georgia Environmental Protection Department
  - Municipal Surface Water Withdrawal Permit
- US Army Corps of Engineers
  - 404 permitting - Nationwide permit is anticipated; coordinated with pipeline permitting
- Coweta County Floodplain Management
  - Study must show “No-Rise” condition
- Georgia Department of Transportation
  - Driveway access to Highway 16 W
- Georgia Power
  - Land acquisition for river intake site

The Municipal Surface Water Withdrawal Permit was submitted in May 2022. Beyond the permits listed for the US Army Corps of Engineers, additional shoreline stabilization will likely be required around the



intake structure. Provided that the Coweta County Floodplain Management permit can demonstrate a “No-Rise” condition, no additional conditional letter of map revision (CLOMR/LOMR) process with FEMA is expected. Drawings for the upstream bridge along Highway 16 W will also need to be coordinated to support equipment supply and construction activities.

Because of the impact to downstream river users, CCWSA will be required to cease pumping with the river flow is below a limiting threshold, measured as 1,500 to 1,600 cfs at Whitesburg. This will limit the amount of water that may be withdrawn at a given time, particularly when the river level is low.



## 2.0 Design Criteria

### 2.1 General

The river intake structure consists of an intake screen system, a pump station and associated equipment and structures. The intake screens will be located along the river edge and contain a cleaning system to prevent sediment buildup and fouling. The intake screens will direct water from the river to the wet well. From the wet well, pumps will transport the water along a raw water transmission main to the B.T. Brown Reservoir.

The river intake and pump station should be designed to meet the water demands for B.T. Brown Water Treatment Plant and the surrounding water district. The district planning flow rates show an initial design of 14 MGD, with a future scale-out to 21 MGD. The pump station initial design will be sized at 21 MGD with the opportunity to upgrade the pumps and equipment to match growth as necessary. This multi-phase approach helps to reduce the initial cost and improves the efficiency of the design by not oversizing equipment.

Based on the 7Q10 values for the Coweta Intake from the Chattahoochee River, the Georgia Environmental Protection Division (EPD) found that a pumping capacity of 24.5 MGD was sufficient to meet a safe yield of 26.5 MGD for the region. Considering that the station will be required to cease pumping at low river flows, an increased instantaneous pumping rate of 27 MGD was determined to meet the 26.5 MGD safe yield. In the future, CCWSA may wish to increase their safe yield further. To support this growth, the buildings and intake structure will be sized to support equipment up to 30 MGD. An additional upgrade to medium-voltage electrical equipment may be required.

Table 2-1 summarizes the design flowrates for an initial and future build.

**Table 2-1 Raw Water Supply and Conveyance Design Flow Rates**

Phase (Year)	Initial (2022)	Future (2050)
Design Flow Rate	21 MGD	30 MGD
Static Head to B.T. Brown Reservoir	El. 687 Chattahoochee River Bottom to El. 890 B.T. Brown Reservoir High Point 203 ft (To be confirmed)	

In addition to the previous design criteria, the following assumptions were made for the Basis of Design:

- Offsite power provided to the site by others. Next phase of design will coordinate with power service providers to develop strategy for power service for pumps.
- No standby generator is included. Because the Chattahoochee River Pump Station supplies water to B.T. Brown Reservoir, the reservoir will provide storage adequate for any power outage.





## 2.2 Intake Screens

### 2.2.1 Design Criteria

The intake screen system should be capable of operating within a river environment and be protected from floating debris and recreational activities within the river. Furthermore, the configuration should be designed to prevent debris and large aquatic life from entering the pumping system and causing damage. To meet these goals, it is recommended that the screens be installed in a protected concrete vault, outside of the main river profile. It is anticipated that the intake structure will require a temporary coffer dam for construction. The intake screen system design criteria are summarized in Table 2-2.

**Table 2-2 Intake Screen System Design Criteria**

Property	Requirement
Minimum Number of Screen Systems	2
Max Screen Velocity	0.5 ft/s
Screen Material	Copper-Nickel Alloy for Zebra Mussel Mitigation
Screen Cleaning	Airburst
Minimum water depth over screen	18"
Bar Screen Opening Size	4"

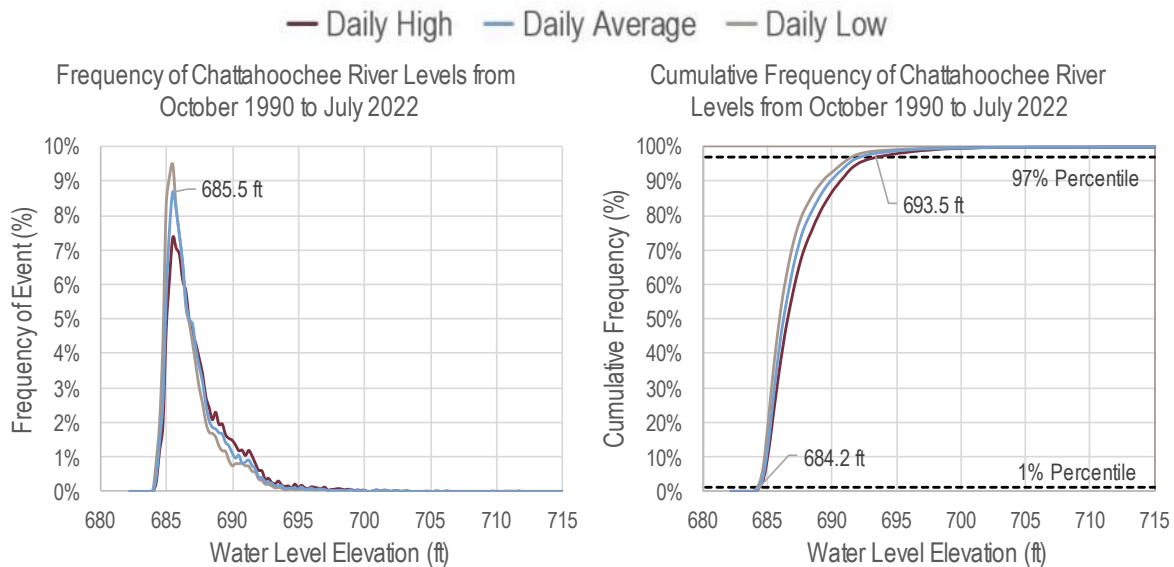
### 2.2.2 Intake Structure Design

The intake screens require a minimum submergence of 1.5 feet to reduce flow problems and are recommended to be raised above the riverbed to reduce sediment intake and build-up. The depth of the riverbed changes over time, but is estimated to be near 678 feet elevation, based on the Federal Emergency Management Agency (FEMA) flood insurance rate map (FIRM). This depth should be confirmed with river profile measurement.

The United States Geological Survey (USGS) has maintained over 30 years of daily stream data for the Chattahoochee River, including the water level of the river as measured above the river gage<sup>1</sup>. Based on the frequency analysis shown in Figure 2-1, the most typical observed river level is between 3 and 7 feet above the datum, or 684 to 689 ft elevation. The screens should be positioned in the center of the river during the lowest expected flow, or 681 feet.

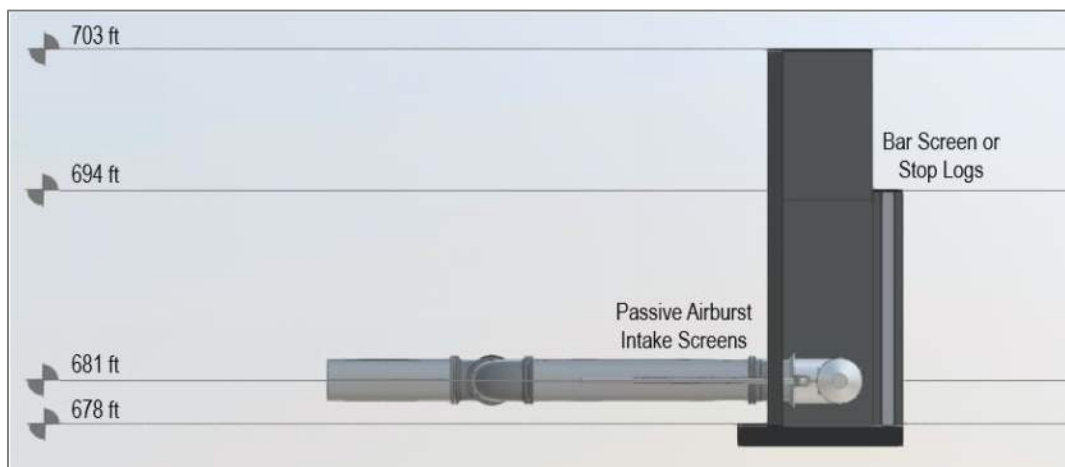
The vault may be designed with a removable steel bar screen on the river-facing wall, and concrete walls separating each screen. The bar screens can be substituted with stop logs or bulkheads to isolate each individual chamber to allow for draining and access when new installation or maintenance activities are needed. To reduce the structure size and cost, we recommend sizing the bar screens and stop logs to account for 97% of the maximum observed river levels, or 694 feet. A profile of the intake structure with marked elevations is shown in Figure 2-2. This design may be revised in later design to improve water flow through the chambers for silt removal.

Two to three screens will be necessary to support the 30 MGD design water flow. To reduce the pipe size and provide redundancy, we recommend routing an individual pipe from each intake screen to the wet well. An example layout is shown in Figure 2-3.



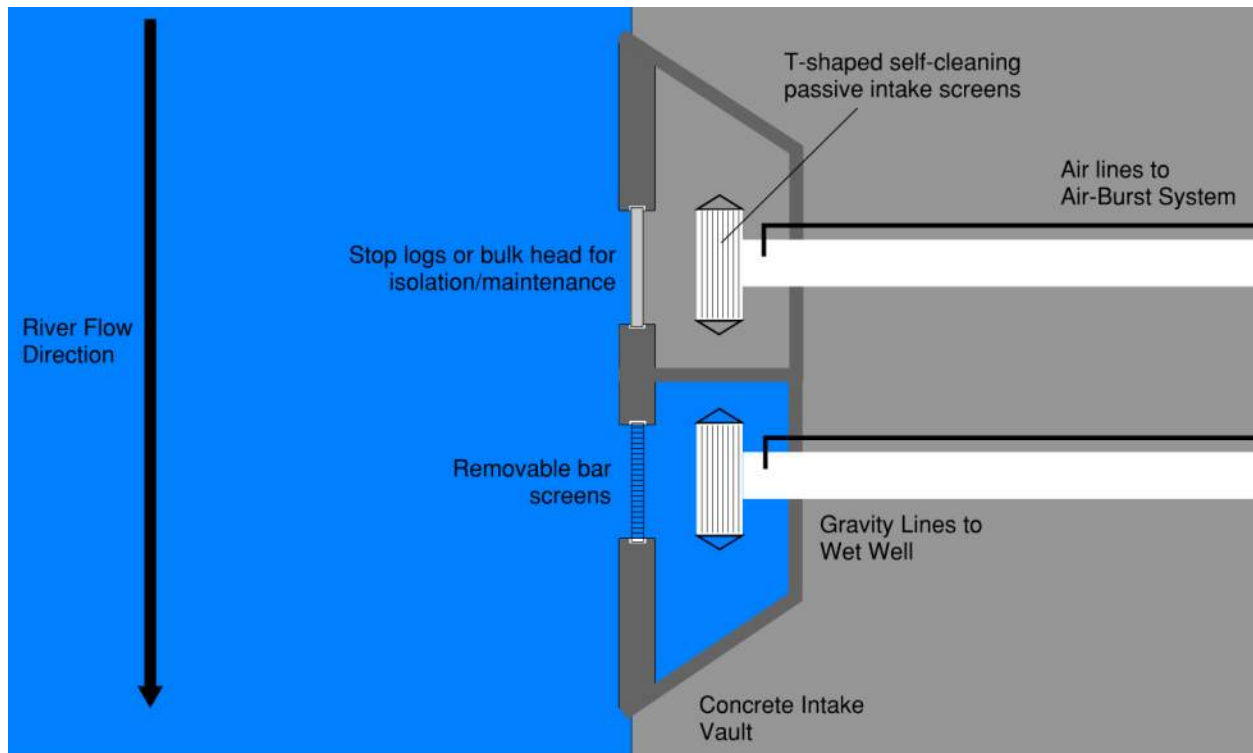
**Figure 2-1 Historical Chattahoochee River Levels – USGS October 1990 to July 2022**

The daily water level ranges between 684 and 694 ft, with less than 1% of observed river levels below this range, and 3% above this range. This range will be used to inform the intake structure design. The most common river level is 685.5 ft, or approximately 1.3 feet above the minimum observed.



**Figure 2-2 Intake Structure Profile View**

The intake screens are positioned above the river bottom and are equipped with an airburst system to reduce silt build-up and blockage. The structure is protected by removable bar screens or stop logs for maintenance.



**Figure 2-3 Plan View of Protective Concrete Intake Screen Vault**

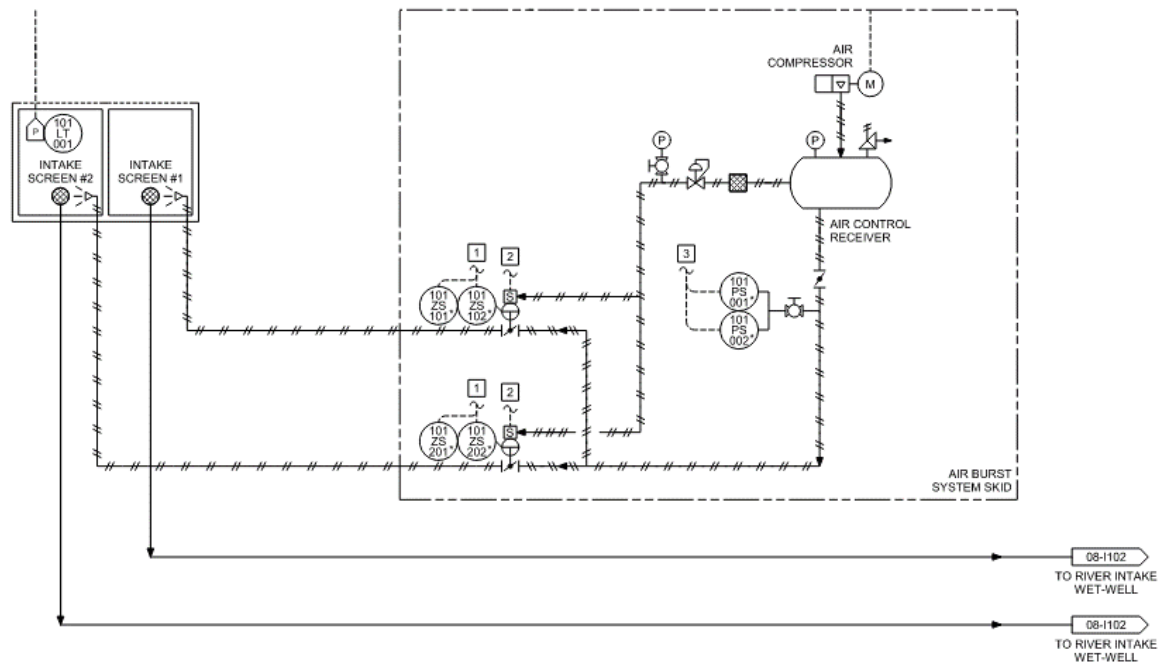
Two passive intake screens provide water from the Chattahoochee River to the pump station. A concrete vault with removable bar screens protects the intake screens from river debris and public access. An air-burst system helps to keep the screens clean, while stop logs allow for isolation/maintenance.

### 2.2.3 Air Burst Control System

The air burst system consists of an air compressor, a large air receiver, approximately 550 feet of 3" air piping, and air-actuated control valves and controller to send the bursts of air through the screens. The tank and valves will be located within the pump building, on the concrete pad above the wet well. The tank is sized large enough to maintain pressure to clean the intake screens 550 feet away.

The air burst control system will be supplied by the screen manufacturer. The control system will initiate an air burst by opening a pneumatic actuated valve and releasing the air stored in a receiver tank through the screen. The air compressor will start and stop using pressure switches and refill the receiver tank for the next air burst.

Air burst for screen cleaning will be initiated on either an elapsed time basis or based on the differential head across the screen. The river level will be measured with a submersible pressure transducer. Differential level will be calculated using difference in river level and wet well level. Stilling pipes may be used to reduce measurement error in the river level.



**Figure 2-4 Excerpt of Airburst System Process and Instrumentation**

The intake screens are equipped with an airburst system for self-cleaning. A pneumatic actuated valve controls the release of pressurized air from a receiver tank through the screen, based on elapsed time or differential head across the screen. The air compressor refills the receiver tank between air bursts.

## 2.3 Pump Station

### 2.3.1 General Design

Raw water is directed from the intake screens to the pump station for conveyance to the B.T. Brown Reservoir. The pump station consists of a wet well, pumps, and valves and piping, and supporting electrical equipment. The station design has a phased design construction approach, based on the flow design criteria. The initial construction is designed to handle 21 MGD of water, with an ultimate firm capacity of 30 MGD. Table 2-3 summarizes some of the key differences between the initial and full build-out.

**Table 2-3 Pump Station Flow Design Criteria**

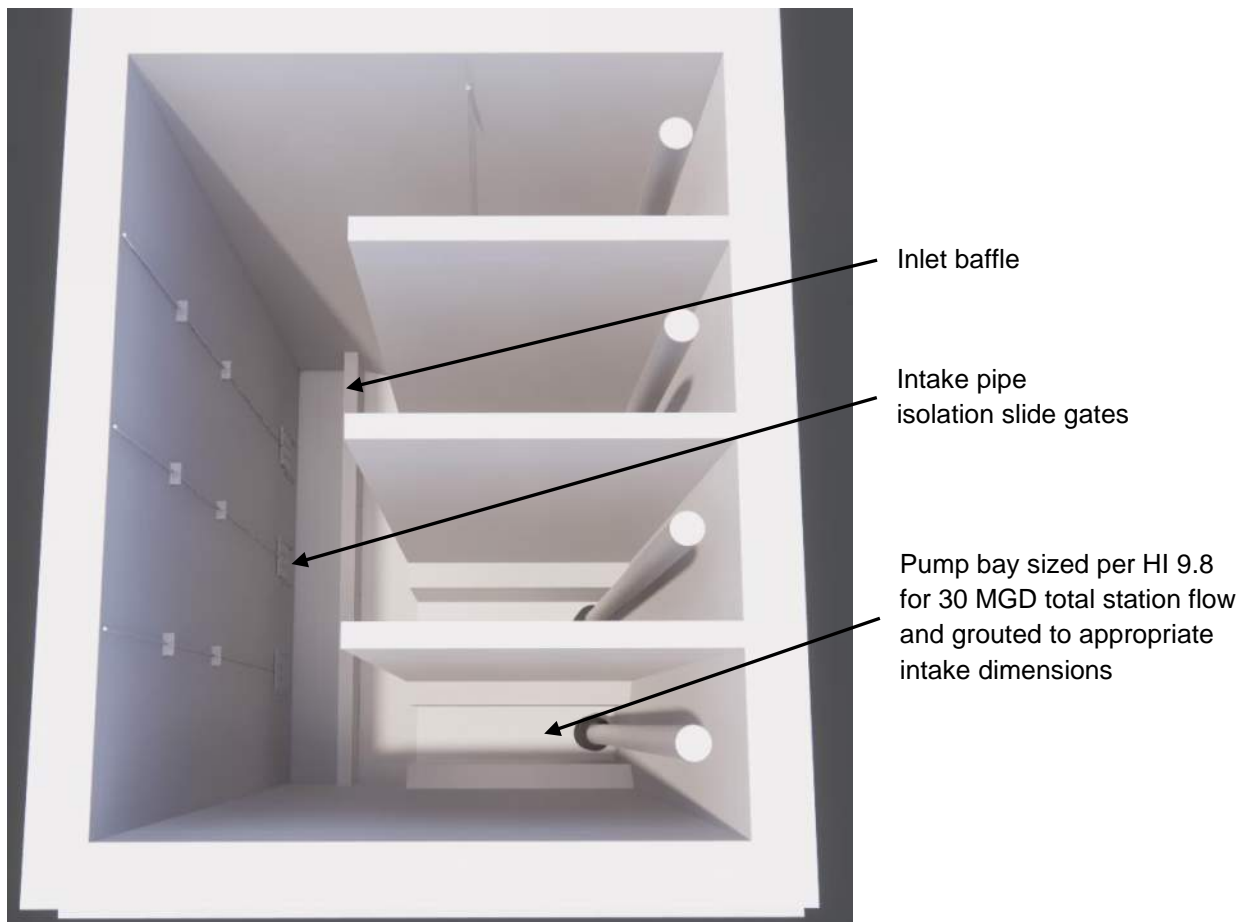
Property	Initial Construction	Full Build-Out
Water Flowrate	21 MGD	30 MGD
Individual Pump Size	4 x 5.25 MGD	4 x 7.5 MGD
Design Head	310 ft	386 ft
Pump Motor Size	450 hp	700 hp
Valve Headers Size	14"	20"
Pipeline Header Size	36"	36"

### 2.3.2 Wet Well Design

The pump station wet well configuration is designed per Hydraulic Institute (HI) 9.8 for Pump Intake Design and be sized for the full build out flow. The well dimensions are sized to be greater than the minimum calculated values from HI, with the individual pump bays grouted to maintain an optimal spacing of 2.5 pump bell diameters around each pump intake.

Modifications from the standard HI design were made to allow for conduit-fed inlets, and a rectangle-shaped well for simpler construction. Inlet baffles were included to help evenly disperse the inlet flow to each of the four bays, but this may require additional modeling or review in future design. Other directional flow control devices could include multi-tiered baffling, or directional inlet vanes.

A layout rendering of the wet well design is shown in Figure 2-5.

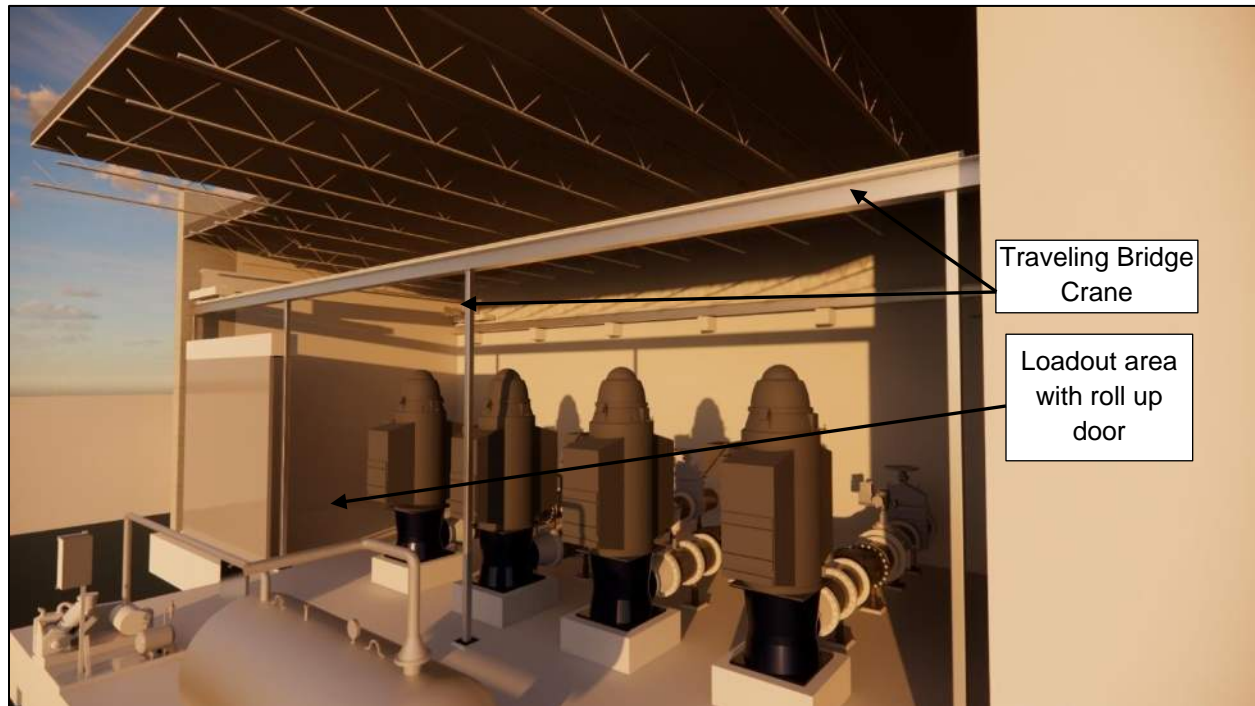


**Figure 2-5 3D Layout of Pump Station Wet Well**

The wet well is designed in a rectangular form factor for simple construction, with intake slide gates for isolation, and an inlet baffle for flow distribution. Grouting is included in each pump bay to optimize the flow conditions around the pump intake.

### 2.3.3 Pump Removal

The pumps will be removed with a 5-ton traveling bridge crane within the pump station. The pumps will be removed in segments, conveyed to the loadout area for shipping or on-site maintenance. A rendering of the traveling bridge crane system is shown in Figure 2-6. If required for servicing, cat walks will be installed around each pump motor for maintenance access.



**Figure 2-6 Traveling Bridge Crane for Pump Removal**

**The pump building is designed with an indoor loadout area for unloading heavy pieces of equipment. A traveling bridge crane extends across the room to assist in installing or removing pumps, valves, and piping.**

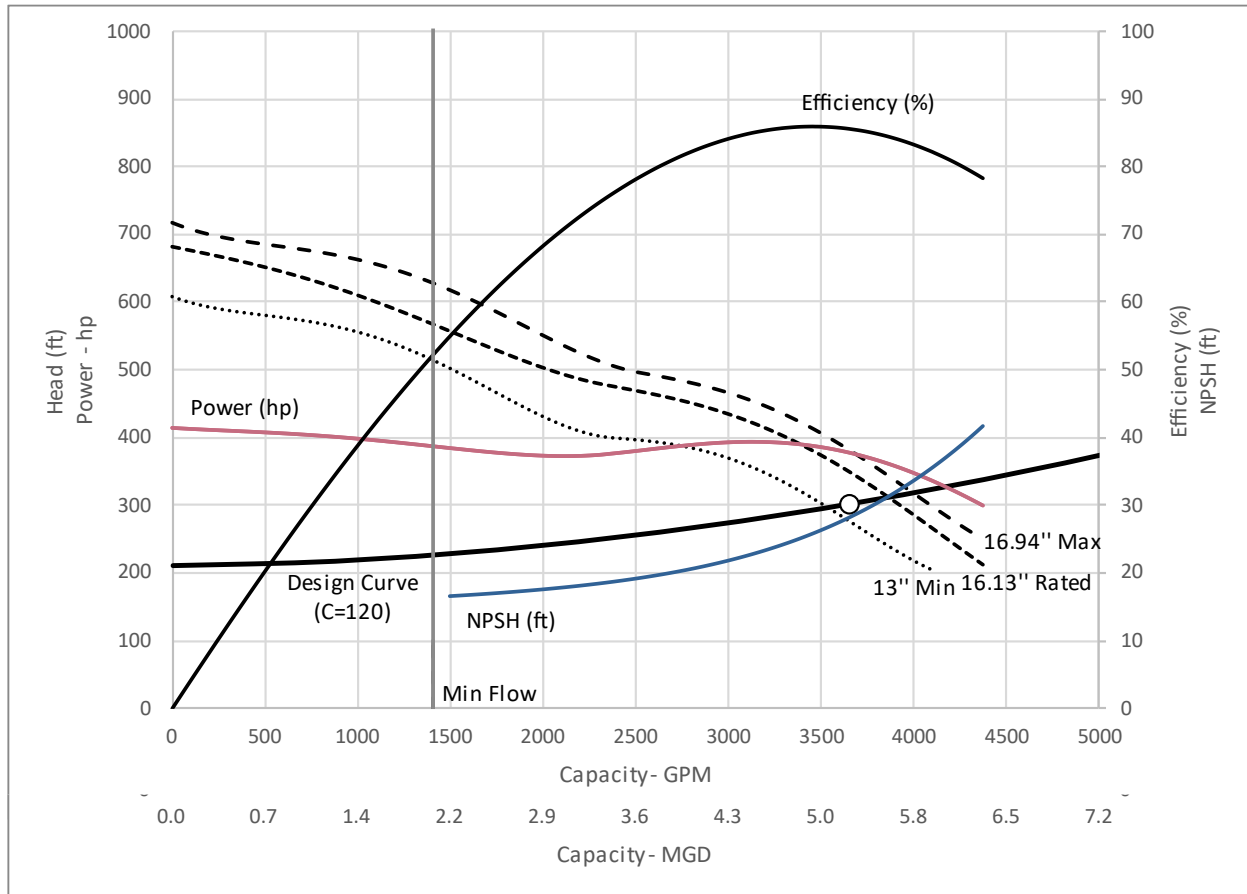
### 2.3.4 Pump Selection

Two pump types were considered for the pump station: vertical turbines and submersible centrifugal pumps. Submersible centrifugal pumps are generally less efficient than vertical turbines and can be more susceptible to failure due to overloading or voltage surges. Submersible pumps also have limited selection options for the pipeline due to maximum head limitations. Discussing the two pump types with CCWSA staff, submersible pumps were not preferred.

Vertical turbines were selected as the preferred pumps and will be used as the basis of design. Table 2-4 summarizes the pump design criteria for the two buildouts. We selected two pumps based on these criteria, optimizing for efficiency and operation within the system envelope. These pumps and motors will be required to undergo job testing prior to operation.

**Table 2-4 Pump Design Criteria**

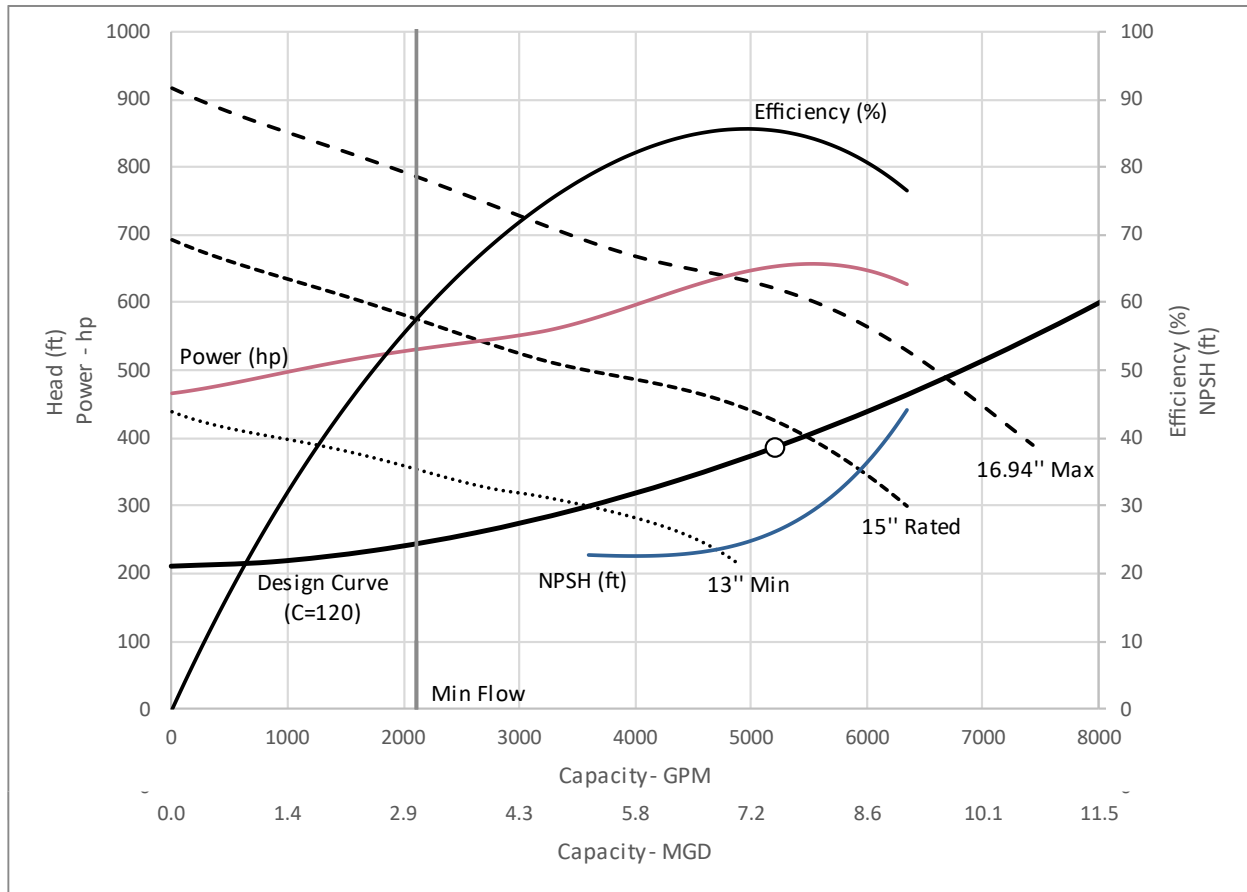
Property	Requirement
Initial Build Design Point	4 pumps with total capacity 21 MGD @ 310 ft
Full Buildout Design Point	4 pumps with total capacity 30 MGD @ 386 ft



**Figure 2-7 Pump Curve for 21 MGD Build – 5.25 MGD at 310 ft Head**

At the design flowrate of 5.25 MGD (3646 GPM), an impeller sized between 13" and 16.13" would provide the most efficient operation up to 85.8%. The shaft power of the pump and NPSH range between 300 to 400 hp and 16 to 42 ft across the pump envelope, respectively. A maximum of 510 to 610 ft of head can be provided at a minimum turndown flow of 1410 GPM (2 MGD) based on impeller size.





**Figure 2-8 Pump Curve for 30 MGD Build – 7.5 MGD at 386 ft Head**

At the design flowrate of 7.5 MGD (5208 GPM), an impeller sized between 13" and 15" would provide the most efficient operation up to 85.7%. The shaft power of the pump and NPSH range between 470 to 660 hp and 23 to 44 ft across the pump envelope, respectively. A maximum of 350 to 790 ft of head can be provided at a minimum turndown flow of 2119 GPM (3.1 MGD) based on impeller size.

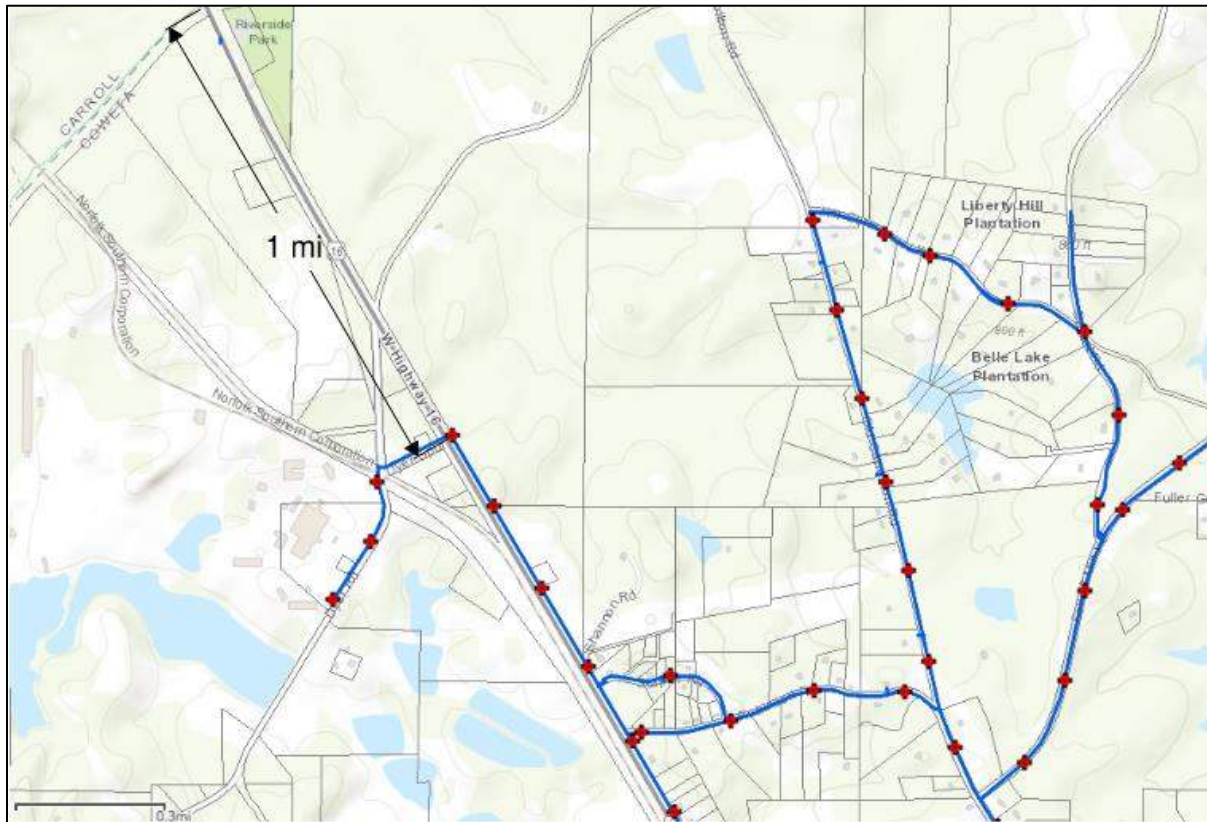
### 2.3.5 Vertical Turbine Pump Design Criteria

The vertical turbine pumps will have the follow features:

- Anti-vortex inlet screens
- Non-reverse ratcheting motors
- Fabricated steel discharge heads
- Enclosed line shafts, with potable water lubrication
- Split mechanical seals, with potable water seal flush
- Bowls and impeller materials will meet NSF 372
- Coatings and other pump materials in contact with water will be NSF 61



Potable water lubrication and seal water is available at Dyer Road approximately 1 mile south of the site, as shown in Figure 2-9. A new water line extension will be included in the design and will be installed alongside the new raw water transmission main.



**Figure 2-9 Coweta Water Authority Map of Potable Water System Near Site**

**The Chattahoochee River is approximately 1 mile north of the nearest potable water main. An extension will be constructed to provide seal water for the pumps.**

## 2.3.6 Pump Station SCADA and Communications

### 2.3.6.1 Pump Station Instrumentation and Communications

The station will include the following instruments:

- Pressure transducer on header
- High pressure switch on header
- Pressure gauge on header
- Electromagnetic flow meter on header
- Wet well low-level switch
- Wet well level measurement - ultrasonic with stilling well to avoid wall interference or radar
- Water quality analyzer (e.g. Turbidity, TOC, Temperature, pH and ORP, Hydrocarbons)
  - Measurement parameters to be determined during final design



Each pump will have the following instruments:

- Motor temperature switch
- Pump vibration switch
- Pressure gauge
- Check valve position switch
- Seal Water Flow Switches and Solenoids

The design basis for communication with the pump station is fiber communication to be routed alongside the raw water transmission main back to B.T. Brown WTP. Spare electrical and control conduits will be included or sized to allow additional cables to be pulled through in future design. Additional radio options are also available.

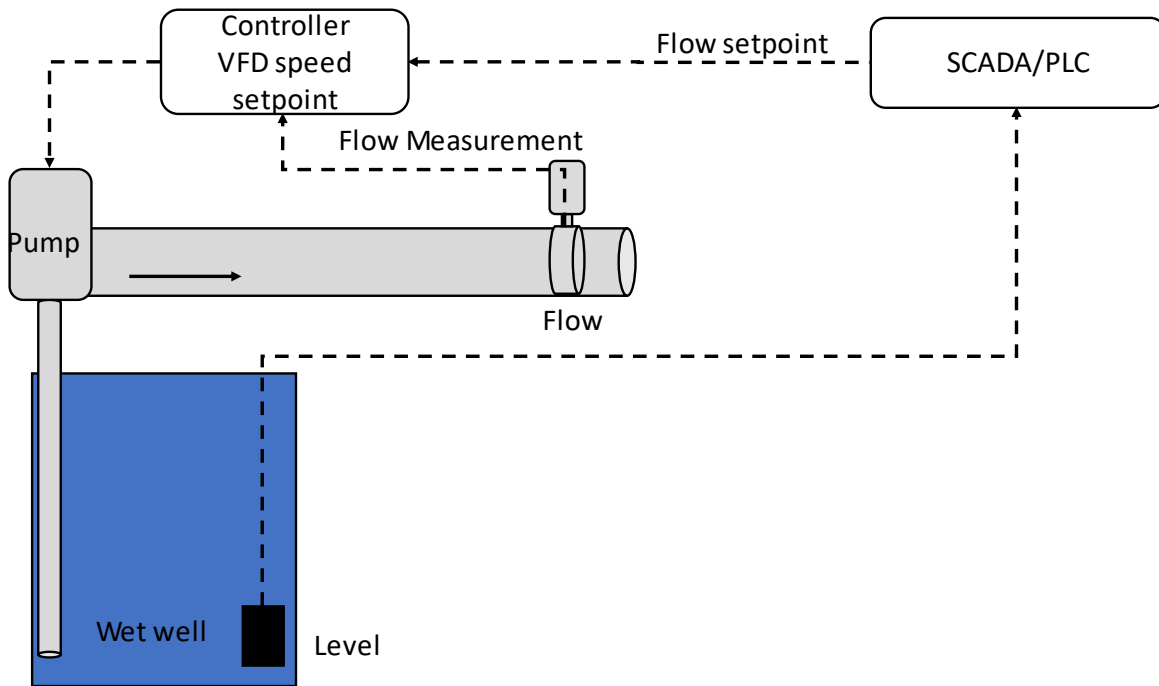
#### *2.3.6.2 Power Criteria and Pump Control*

The pumps have been selected for the initial buildout to stay within a standard 460V motor size to avoid installing medium voltage electrical gear. At the full buildout, the motors may be too large to stay within the low voltage motor sizes, and the electrical gear may need to be replaced with medium voltage gear. This determination should be evaluated at the time of the expansion based on actual and projected demands at the time of the upgrade. Flow or head conditions may change and the build out pumps may still fall in low-voltage range.

The pump station primary control will be a flow setpoint. The operator will set the desired pumping rate, and the pump control system will bring on the required number of pumps and vary the speed of the pumps to meet the desired flow rate.

The pumps have been selected to all be the same size. With variable head conditions at varying flow rate, pump safety interlocks should be included in the control logic to protect the pumps from operating outside their allowable operating regions. The pump controls will be interlocked with the wet well level to shutdown pumps on low level. The pumps will also be interlocked to shut down in the case that the B.T. Brown Reservoir is full. The ball valves on the discharge of the pumps will be electrically driven for additional control and isolation.

See Figure 2-10 for a representative pump control schematic.



**Figure 2-10 Pump Control Schematic**

The pump station will be controlled with a flow setpoint to modulate how many pumps are running and the pump speeds. A level indicator will provide SCADA alerts to the PLC.

Detailed process and instrumentation diagrams can be found in Appendix C – 10% Design Drawing Set

### 2.3.7 Station Piping & Surge Control

The station piping will be fabricated flanged ductile iron pipe and fittings meeting AWWA C110 and C115 standards, with a pressure rating of 250 psi. All valves shall have a 250-psi working pressure rating. Each pump discharge will have the following valves:

- Well service air release valve sized to exhaust the equivalent air flow within the selected pump intake can
- Check valve or pump control valve with check feature: the check valve type will be determined in final design in coordination with surge analysis
- Pump control valve as required for surge mitigation
- Isolation gate valve

The valve header is sized to keep flow velocities between 5 to 8 ft/s. For the 21 MGD installation, the headers are sized at 14", with an associated velocity of 7.6 ft/s from each 5.25 MGD pump. For the 30 MGD installation, the headers are sized at 20", with an associated velocity of 5.3 ft/s for each 7.5 MGD pump.



Surge mitigation should be implemented to avoid excessive surge pressures at the pump and to mitigate vacuum condition at high points in the pipeline. A detailed surge analysis should be performed during detailed design as the pipeline design progresses. Surge mitigation strategies should consider the following:

- Air vacuum relief along the pipeline
- Compressed air bladder surge tank at the pump station
- Pump control valve and check valve selection to mitigate valve slam and surge from sudden pump stops and starts

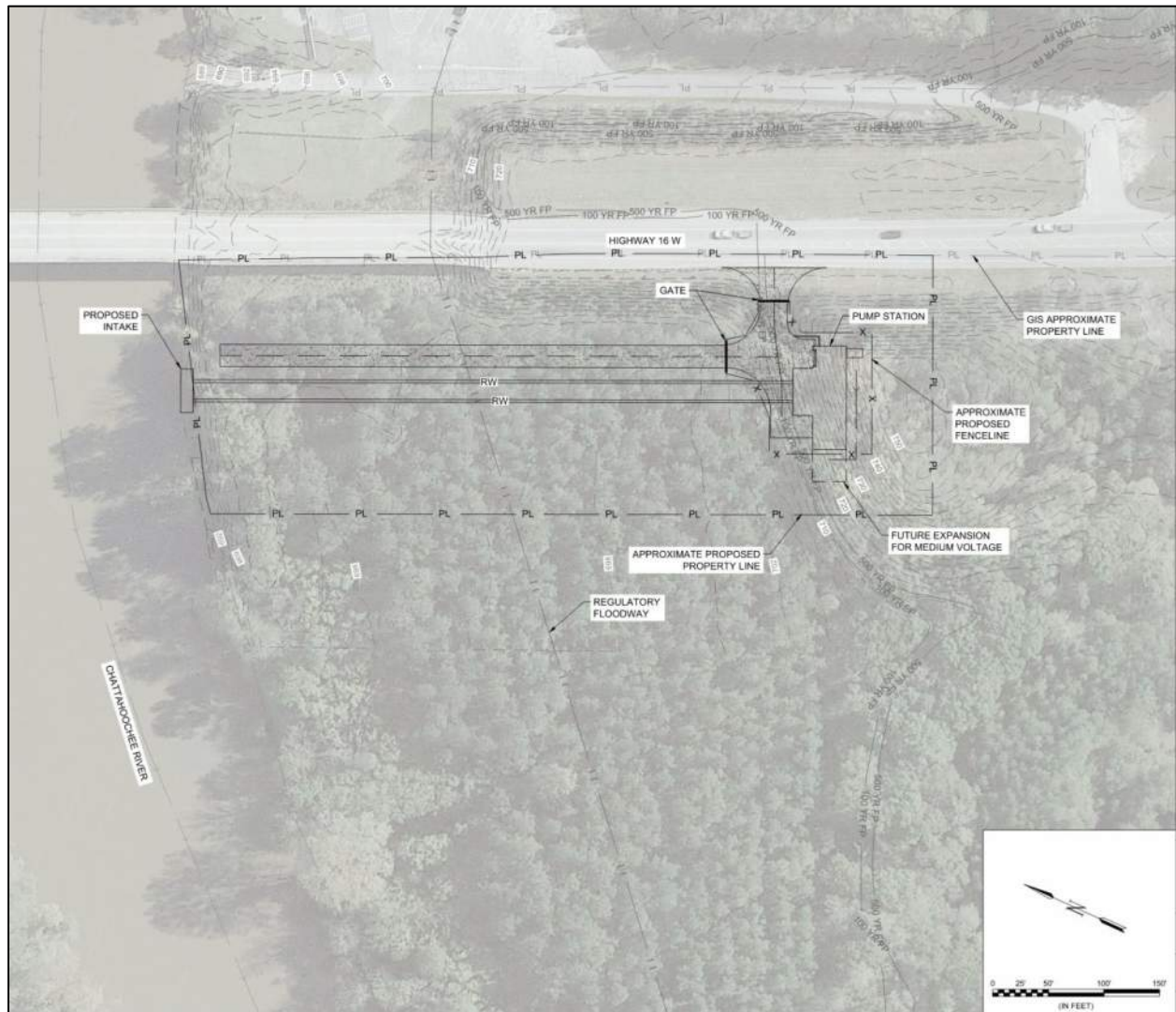
## **2.4 Civil Design Criteria**

### **2.4.1 Access**

The construction traffic route will generally be assumed to access the site from I-85 using highways. To access the site from the northwest, the route will leave I-85 using highways 14, 70, 5, and then 16. To access the site from the southeast, the route will leave I-85 using highways 34 and 16. Signage will be included along the highway in both directions to direct towards the site.

Access to the site will be directly from Highway 16 W and is anticipated to be a gravel surface. The proposed road will be a 20-foot minimum width with minimum 35-foot radii and laid out for WB-62 semi-trailers. The new driveway will require coordination and permitting with Georgia Department of Transportation as stated in Section 1.4. This access road will be designed with sufficient space to maneuver appropriate vehicles needed to haul pumps, motors, and other equipment from the site. The access road will allow for trucks to complete a three-point turn and back into the overhead door bay to access the internal traveling bridge crane area and then leave the site.

Intake access will be via an approximately 500-foot corridor paralleling the highway. A permanent roadway is not currently anticipated, but it is expected that the construction road installed by the Contractor will remain and be maintained for access. Space to completely turn around large vehicles will be allowed to the northwest of the pump station site. An overview of the site plan is shown in Figure 2-5.



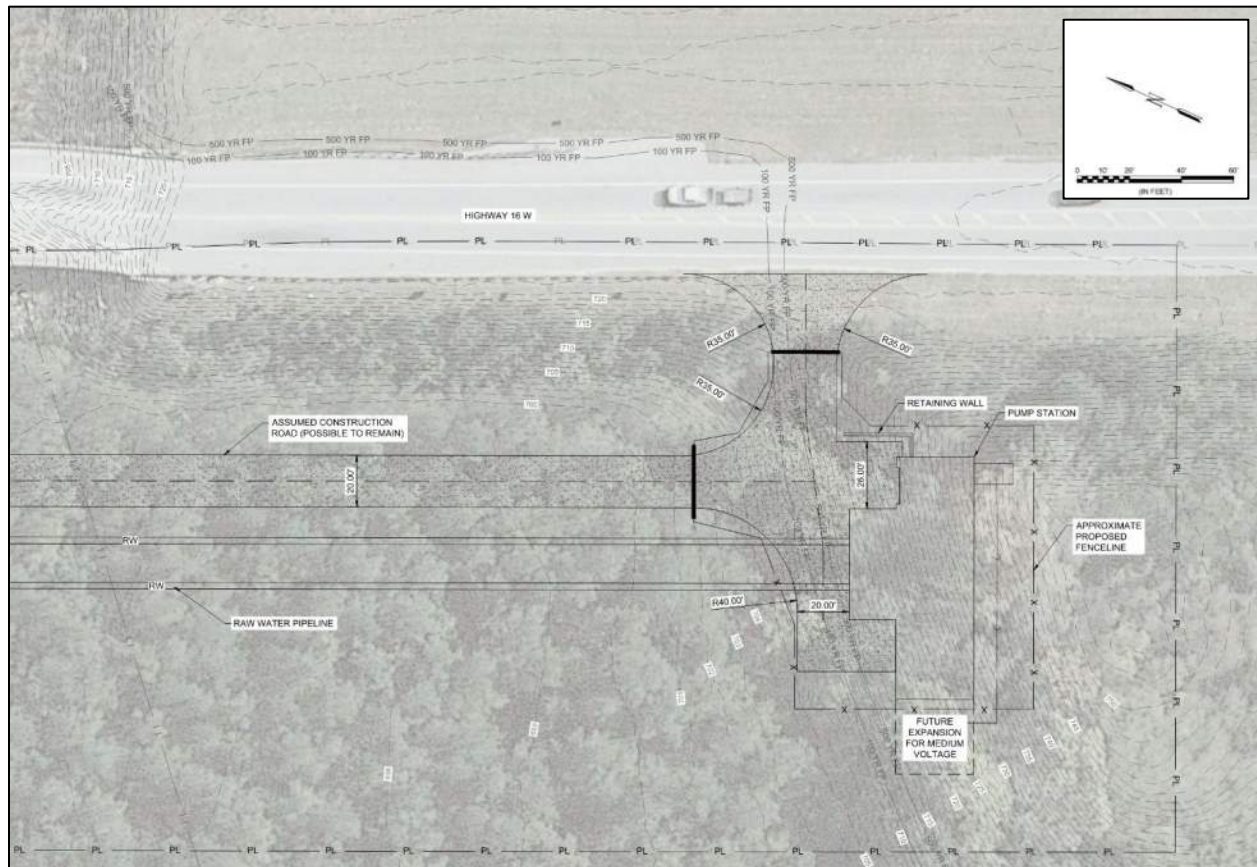
**Figure 2-11 Site Plan Overview**

The intake structure will be accessible during construction through a gravel road designed for WB-62 semi-trailers. Gates will control access to the pump station from the highway and intake structure.

#### 2.4.2 Grading and Paving

To keep the facility out of the floodplain, as discussed in Section 2.4.1, the pump station will be located away from the intake structure. The existing topography of the site becomes steep along the highway embankment and just as the existing grade rises above the flood elevation to the south. Access along Highway 16 is necessary for maintenance of the new facilities and the proposed location for the pump station was selected because this point offers the flattest point to connect to the road.





**Figure 2-12 Enlarged Pump Station Site Plan**

The pump station will cut into a hill to fall outside of the indicated 100-yr and 500-yr floodplains. If the station needs to upgrade to medium voltage, the electrical building may be expanded to the south.

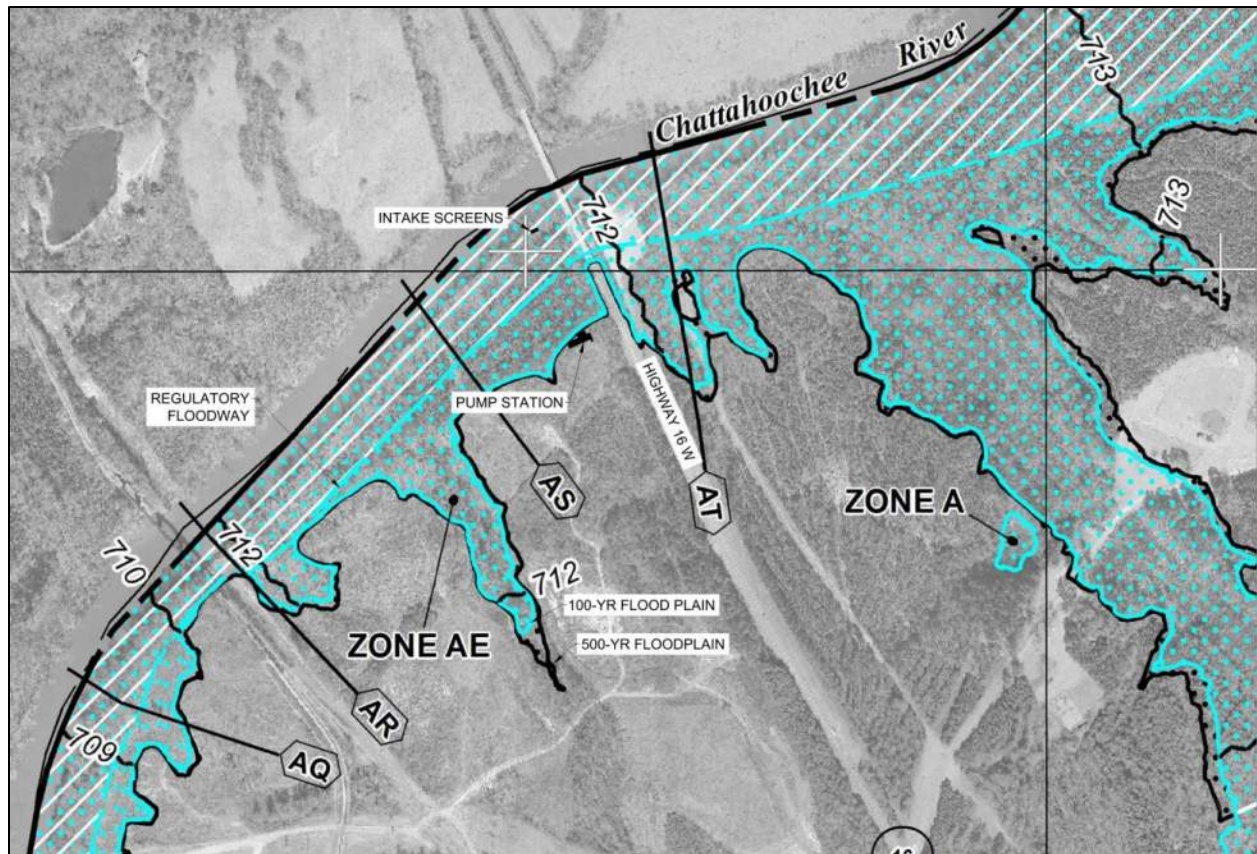
The placement of the pump station outside the floodplain forces it into a relatively steep hill at this location, so the pump station walls will serve as partial retaining walls and the building will be recessed into the hillside to minimize excavation as much as possible. An additional retaining wall will be necessary as shown in Figure 2-12 above to tie the excavated slope to existing grade. Maximum slopes of 3H:1V will be used on site, with 4H:1V preferred where possible. The finished grading will maintain positive slope away from the structures for drainage. A drainage culvert is anticipated at least across the driveway access to Highway 16.

As discussed in Section 2.4.1, a gravel roadway is anticipated to be placed around the pump station structure to allow access and maintenance of the facility. The final road section will be based on recommendations from the geotechnical engineer, but the section will be a minimum of six inches of aggregate base course underlain with suitable subgrade and a geotextile as necessary. The road will be designed anticipating light maintenance traffic with the occasional heavy-loaded vehicle with equipment.

### 2.4.3 Floodplain Considerations

Based upon the latest version of the published FEMA data, the approximate 100-yr flood elevation is 712 feet. The floodplain, as well as the regulatory floodway encompasses the necessary location of the intake structure. To keep the pump station and electrical room accessible and functional during flood events, the structure will be located outside the regulatory floodplain, approximately 550 feet from the intake. For additional safety, the floor elevation of the pump station and all equipment will be located at the 715-foot elevation or above, which is also above the 500-yr floodplain. The published FEMA map is shown below in Figure 2-13.

Fill will be required to be placed in parts of the floodplain near the pump station to account for the grade change, which will require a no-rise certification and additional permitting through Coweta County. Based on preliminary conversations with the County, it is not anticipated this process would need to go through FEMA, provided a no-rise in the flood elevation is shown.



**Figure 2-13 FEMA Floodplain Map**

The intake location for the screens falls between the AS and AT cross sections. The pump station will be located outside of the indicated 100-yr and 500-yr floodplains.



#### 2.4.4 Site Security

The new pump station site will have security fencing installed as shown in the site plan in Figure 2-12. The security fencing is expected to be 6-foot-tall chain link, topped by three strand barbed wire. Two gates are anticipated at the site, one off the highway and one directly in front of the maintenance bay, toward the intake structure. The highway gate will be offset enough to allow standard vehicles to completely pull off the highway to open and close the gate. The second gate will be used to access the intake, as well as be opened when necessary to allow large vehicles access to turn around. Methods for secured access and level of security shall be coordinated with the County.





### **3.0 Electrical System Concepts**

#### **3.1 Conductors and Raceway**

Low voltage power conductors (600V and less) will be copper, 600V rated, type THWN-2 for above grade locations and type XHHW-2 for below grade locations in conduit. Conductors used for motors operating with variable frequency drives (VFDs) will be shielded and intended for use in VFD applications. Low voltage signal cables will be shielded twisted pair with 20AWG tinned copper conductors for analog signals. Discrete input and output signals operating at 120VAC or 24VDC will utilize THWN-2 conductors.

Below grade conduit will be PVC schedule 40 in concrete encased duct bank with galvanized elbows. Below grade boxes will be utilized to provide access to below grade duct banks and provide pulling points for conductors. All below grade boxes will be pre-cast polymer concrete with traffic rated lids where deliberate or intermittent traffic is expected.

Above grade exposed conduit systems for indoor applications will be galvanized or aluminum, while above grade exposed conduit systems for outdoor applications will be PVC coated steel conduit. PVC coated steel conduit will be used for all transitions from below grade conduit system to an exposed conduit system. Conduit in chemical areas will be PVC coated for chemical corrosion resistance. Connections to motors and other vibrating equipment will be made with liquid-tight flexible metal conduit.

#### **3.2 Site Lighting and Security**

Illumination levels will be designed to meet the minimum levels established in the 10<sup>th</sup> edition of the Illuminating Engineering Society (IES) Lighting Handbook for the applicable spaces. Lighting will be designed for maximum efficiency in order to meet allowable energy density requirements as set forth by ASHRAE 90.1. Interior lighting will primarily utilize surface mounted, wet location rated LED fixtures as applicable. Emergency egress lighting will be provided using battery backup fixtures and exit signs. Architectural LED fixtures located near building entry and exit points will provide lighting around doorways. Pole-mounted LED light fixtures will be installed around outdoor equipment to provide area lighting as necessary for maintenance, security, and access around the site. All pole mounted outdoor lighting will be connected to a central lighting control system, with exterior building mounted lights being controlled locally at each building using a photocell and switch. Exterior fixtures will be cutoff type to reduce light pollution.

#### **3.3 Grounding and Lightning Protection**

The ground system will include a bare copper conductor ring around new structures with multiple ground rods using exothermic welds below grade and connected to building steel. Based on building functionality and building construction types, a lightning protection system will be installed in accordance with NFPA 780 and connected to this ground ring system. A green equipment grounding conductor will be installed in each feeder and branch circuit conduit. The overall grounding system will provide a low impedance circuit for proper operation of overcurrent protective devices.



### **3.4 Motors**

Electric motors will primarily consist of squirrel-cage, alternating-current induction type motors. All motors will be NEMA Premium Efficiency rated with copper windings and a minimum of Class F insulation. Motors to be operated with a variable frequency drive will be NEMA MG1 Part 31 rated to withstand voltage spikes up to 1600V. Motors will have a minimum service factor of 1.15. Motors will include winding thermostats for thermal protection. Motors larger than 50HP installed outdoors or in non-conditioned indoor areas will include 120V space heaters. Shaft grounding rings will be provided for motors larger than 50HP operating on variable frequency drives to mitigate VFD-induced bearing damage.

### **3.5 Power Distribution Equipment**

Several types of power distribution equipment will be utilized to distribute power to loads. It is expected that low voltage switchboards, motor control centers, panelboards, transformers, and disconnect switches will all be utilized. Power distribution equipment located indoors in air-conditioned environments will utilize NEMA 1 construction. Equipment located in wet locations indoors or outdoors will utilize NEMA 4X stainless steel construction to prevent corrosion and equipment deterioration. Power distribution equipment will be required to be provided from a single manufacturer for ease of maintenance and reduced spare parts inventory.

### **3.6 Variable Frequency Drives**

Variable frequency drives (VFDs) will be utilized for speed modulation of pump and blower motors for enhanced process control. VFDs will be equipped with harmonic mitigation features to prevent harmonic interference with other electronic equipment. VFDs larger than 50HP will utilize 18-pulse configuration to mitigate harmonic concerns. Load filters will be utilized as needed depending on conductor lengths between the VFD and the motor. Excessive conductor length applications will utilize dv/dt filters to mitigate reflected wave phenomenon that could cause operational issues and equipment damage. VFDs will be located in dedicated conditioned electrical rooms. VFDs will be required to be provided from a single manufacturer for ease of maintenance and reduced spare parts inventory.

### **3.7 Overcurrent Protection**

Circuit breakers and fuses will be utilized to protect conductors and electrical equipment from overcurrent conditions that could cause equipment damage. These devices will be rated for the maximum available fault current to ensure operation during all conditions. Circuit breakers 200A and larger will be equipped with electronic trip units with adjustable long-time, short-time, and instantaneous settings (LSI) to provide coordination flexibility for the system. Circuit breakers smaller than 200A will be thermal magnetic devices. Ground fault protection will be provided for all service entrance overcurrent devices and downstream devices as needed.

### **3.8 Surge Protection**

Surge Protection Devices (SPDs) will be provided at new facilities to provide protection from power system surges. Level 1 and Level 2 devices will be provided at service entrance panels and large distribution panels. Level 3 devices will be provided at local power panels and control panels.



Surge protection will be provided for analog signal cables through the use of surge protection terminal blocks at PLC inputs and outputs. Signal surge protection devices will also be located at each instrument to protect sensitive instrumentation equipment from surges that can occur on analog signal lines.

### **3.9 Power Metering**

Digital power meters will be provided at new main distribution panels to track real-time power conditions, assess power quality and reliability, and track historical usage trends. Tracking energy consumption through the use of connected power meters will provide the plant with the information needed to make process decisions that can maximize energy efficiency and minimize electrical consumption costs.

### **3.10 Power System Studies**

A power system study should be performed for new electrical equipment. The study will consist of a short circuit evaluation, protective device coordination study, equipment evaluation, and arc flash hazard analysis complying with the requirements of NFPA 70E. Each electrical panel requiring maintenance or servicing will receive an arc flash label indicating the associated arc flash and shock hazard. Personnel responsible for maintenance of electrical equipment will be trained to interpret the arc flash labels and utilize the proper personal protective equipment (PPE) to protect from arc flash and shock hazards.

### **3.11 Electrical Codes and Standards**

The following codes and standards will be utilized in the design of the electrical systems:

- National Electrical Code (NFPA 70) published by the National Fire Protection Association.
- Standard for Electrical Safety in the Workplace (NFPA 70E) published by the National Fire Protection Association.
- Life Safety Code (NFPA 101) published by the National Fire Protection Association.
- Standard for the Installation of Lightning Protection Systems: NFPA 780.
- Standard for Electrical Safety in the Workplace: NFPA 70E.
- Any applicable local amendments to the above referenced codes and standards.



## **4.0 Pipeline Criteria**

### **4.1 Discharge Locations**

During the kickoff meeting with CCWSA, multiple discharge locations were discussed including BT. Brown Reservoir, Vulcan mine, and a new WTP at location to be determined. B.T. Brown reservoir is the preferred location discharge and if future discharge locations for Chattahoochee water are needed, a pump station at B.T. Brown would be added to convey the flow to the desired location.

### **4.2 Route Criteria**

The route of the raw water pipeline was evaluated in depth in the Route Study Technical Memorandum (Appendix A). The criteria that were used in the evaluation included environmental impacts, permitting requirements, ground elevation profiles, and cost. Of these criteria, the topography along the profile of the pipeline has the largest impact on the required pump horsepower. Additional high points along the pipeline may require higher horsepower pumps at the river intake pump station.

### **4.3 Pipeline Sizing**

Pipeline sizing criteria include minimum and maximum pipe velocities, head loss, and cost. A larger diameter pipeline will have lower velocities and head loss but will be more expensive. Pipe diameters evaluated for the purposes of this report were 30", 36", and 42". A 30" pipe was determined to be too small based on the maximum water velocity of 9.54 ft/s and resulting head loss. Pipe diameters of 36" and 42" are feasible options based on the hydraulic evaluation. A 36" pipe was determined to be optimal based on the low velocities and extra cost of a 42" pipe. The Route Study Technical Memorandum includes further evaluation of the size of the raw water pipeline. See Appendix A.

## 5.0 Alternatives Evaluations

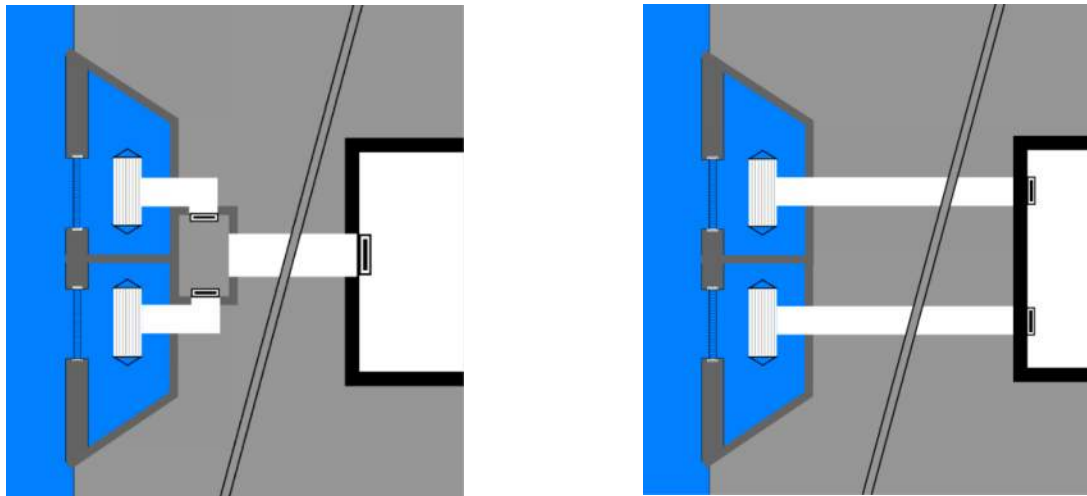
### 5.1 Intake Alternatives

The intake structure consists of two intake screens which direct water from the river to the pump station wet well. The design put forward in Section 2.2.2 recommends using two separate pipes for added redundancy, but there is a potential cost savings to combining the headers into a single larger pipe but with a loss of redundancy.

Figure 5-1 shows two alternative designs for routing water. The single pipeline alternative joins the intake screens at a splitter box, while the multi-pipeline alternative runs two parallel lines to the wet well without combining.

Routing two separate lines would require the excavation and burying of two parallel 30" pipes, along with two 30" sliding gates at the wet well for isolation. Two 10-ft tall trench boxes can be stacked to install the pipes and significantly reduce excavation costs. Two separate trenches of 5' width would be necessary to install both pipes.

A single-pipe alternative requires a larger pipe with an equivalent or greater cross-sectional area of two 30" pipes. The equivalent diameter of this pipe would be 42.4", which is not a standard pipe size. The closest available pipe diameter is 48". A trench of 8' wide would be necessary to bury the intake line; two trench boxes can also be stacked to achieve the target depth. To allow isolation of each screen for maintenance, two sliding gates would be required at a splitter box in the intake structure, and one additional sliding gate at the wet well for isolation. A summary of the design parameters is shown in Table 5-1.



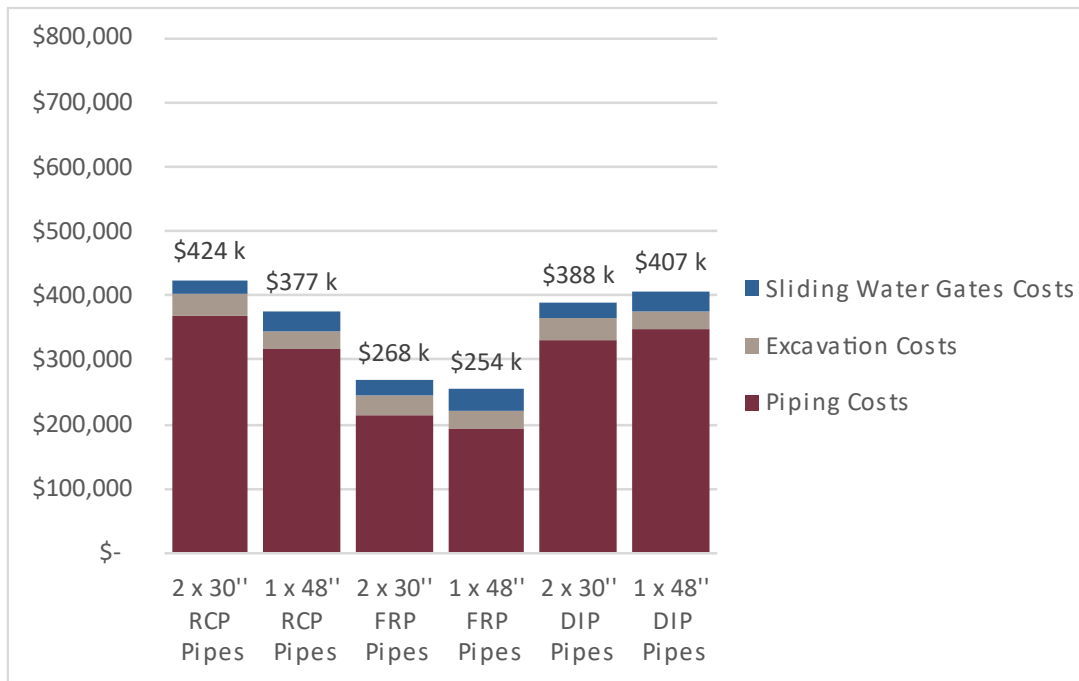
**Figure 5-1 Alternative Intake Screen Pipeline Layouts**

There are two potential alternatives for routing water from the intake structure to the wet well. The single pipeline alternative uses a splitter box with two smaller sliding gates, while the multi-pipeline alternative uses individual gates at the wet well.

**Table 5-1 Intake Pipeline Alternative Design Parameters**

Parameter	Multiple Pipes	Single Pipe
<b>Piping</b>		
Number of Pipes	2	1
Diameter of Pipe	30"	48"
Length of Pipe	550'	550'
<b>Excavation (Trench Boxes)</b>		
Excavation Width (Includes 0.5D Spacing)	5'	8'
Excavation Depth (At Wet Well)	22'	22'
Number of Trenches	2	1
<b>Sliding Gates</b>		
Number of Sliding Gates at Intake Structure	0	2
Number of Sliding Gates at Wet Well	2	1

Using the assumptions in Table 5-1, and up-to-date budgetary estimates for the costs of pipes, gates, and excavation, we calculated the total cost of each alternative. The cost is greatly influenced by the pipe material selection, so we repeated the analysis for reinforced concrete pipe (RCP), fiberglass reinforced pipe (FRP), and ductile iron pipe (DIP). The compared costs for each alternative and material are shown in Figure 5-2.



**Figure 5-2 Alternative Cost Comparison for Intake Screens to Wet Well Piping**

The cost of running two 30" lines is only moderately more expensive (\$14 to \$47 k) than a single line for RCP and FRP pipe, with the most significant factor being piping and excavation costs. In the case of DIP, it is more economical to install two pipes, with a cost savings of \$19 k



Based on the results shown in Figure 5-2, we recommend that two parallel lines are installed. The cost savings in the case of RCP or FRP are relatively minor and are outweighed by the benefits of having a redundant line in the case of pipe failure or scheduling routine maintenance activities. In the case of DIP, installing two pipes is the more economical choice. DIP is the preferred pipe material, but final pipe selection will be during final design.



## 6.0 Cost Estimates

### 6.1 Scope of Improvements

The scope of improvements for the Chattahoochee River Intake Pump Station includes the following project elements, shown in Table 6-1.

**Table 6-1 Project Elements**

Project Element
Site Civil
Intake Screen Structure and Screen System
Pump Station

### 6.2 Opinion of Probable Construction Cost

#### 6.2.1 Construction Cost Estimate Criteria

The level of accuracy in cost estimating varies depending on the level of detail defining the project scope. The estimate herein is classified by AACE 18R-97 as a Class 4 estimate with an expected accuracy of -30% to +50%. For the purpose of this report, the OPCC has been developed based on the facility and equipment layouts, process diagrams, and similar information compiled as part of the basis of design. Table 6-2 summarizes the main assumptions adopted for the development of the OPCC.

**Table 6-2 Preliminary Design OPCC Estimate Assumptions**

Consideration	Assumption
Contractor mobilization	3%
Contractor overhead and profit	18%
Unidentified item allowance for final design	5%
Design progression contingency	30%

#### 6.2.2 OPCC Development Guidelines

The preliminary design OPCC has been prepared based upon the following guidelines:

- The subtotal of each area includes costs assigned to contractor mobilization, contractor overhead and profit, and adopted estimating contingency for unidentified work.
- Excavation and backfill costs required for each facility area are assigned to that area.
- Electrical and instrumentation costs are assigned directly to each facility area's overall costs.

The development of the adopted unit prices incorporated costing knowledge obtained from a number of resources that include:





- Actual cost estimates and proposals provided by equipment manufacturers and suppliers
- Previous cost estimates prepared by the design team
- Contractor bid tabulations from recent project deliveries
- Cost estimating guides provided by nationally recognized cost estimating organizations.

#### 6.2.3 Construction Cost Estimate Findings

The September 2022 OPCC for the proposed improvements presents the estimated construction costs based on the present level of design development. Contingencies have been applied to the identified costs to account for unidentified work elements associated with the present level of design completion. The overall OPCC for the improvements under the scope of this project is presented in Table 6-3. The cost estimate breakdown summaries are given in Appendix B.

**Table 6-3 Overall OPCC for Chattahoochee River Intake Pump Station**

	Construction Element	Estimated Cost
Lump Sum Work	Site Civil	\$1,069,000
	Intake Structure	\$1,085,000
	Raw Water Pump Station	\$16,252,000
	<b>Sub-Total Estimated Lump Sum Work</b>	<b>\$18,406,000</b>
Cash Allowances for Products and Services	Work Change Directives	\$553,000
	Independent Testing Cash Allowance	\$185,000
	<b>Sub-Total Estimated Cash Allowances</b>	<b>\$738,000</b>
Escalation	<b>2 Years Escalation to Midpoint of Construction</b>	<b>\$3,018,000</b>
<b>Total Construction Cost</b>		<b>\$22,162,000</b>



## **7.0 Summary and Recommendations**

The recommended basis of design recommends a new Raw Water Pump Station and Intake on the Chattahoochee River that will have an initial capacity of 21 MGD and a buildout capacity of 30 MGD. It will include the following features:

- New intake screen structure sized for 30 MGD, the screen structure will include:
  - at least 2 passive intake screens and 2 connecting pipelines to the pump station wet well
  - an air burst system for cleaning the screen
  - screens will be constructed from alloys to prevent zebra mussel accumulation.
  - a method of isolation
  - a design that protects screens from large floating debris
- A new wet pit vertical turbine pump station, with initial capacity of 21 MGD and expandable to 30 MGD with pump replacement. The pump station will include the following features:
  - 4 vertical turbine pumps within an HI standard wet well design sized for up to 30 MGD.
  - Enclosed line shaft pumps with potable water lubrication system to prevent sediment and debris in the raw water from prematurely wearing bearings and seals.
  - Necessary instruments and controls for automatic flow-based control.
  - The building will house a traveling bridge crane for pump removal and maintenance, the airburst system, the pumps, and valves, and have a conditioned electrical room
  - The pumps are sized to stay within the low voltage horsepower range
  - The 30 MGD pumps may require medium voltage depending on head requirements in later stages of design
  - The station will be constructed outside of the floodplain with a finished floor elevation three feet above the floodplain
  - Adequate access and security of the site will be provided
  -

The recommended project construction costs are estimated to cost \$22 million.



# **Appendix A**

## **Route Study for the Raw Water Pipeline**

# Raw Water Pipeline Route Study

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**Coweta County Water and Sewerage Authority**



**September 2022**

**Prepared by:**

**Josh Evans**



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## **1.0 Executive Summary**

This Route Study summarizes our findings for the Chattahoochee Raw Water pipeline alignment investigations. We investigated three raw water pipeline alignments to supply a new source water for the B.T. Brown Reservoir. The alignments can be described as follows:

- Alternative 1 – Parallels the Georgia Power right-of-way
- Alternative 2 – Parallels the Atlanta Gas Light right-of-way, County road right-of-way, and the pumpback station easement
- Alternative 3 – Follows GDOT right-of-way and County road rights-of-ways

The following items accompany our investigation and contribute to the overall assessment of each alternative:

- A hydraulic analysis to calculate required pipe diameter.
- An approximate profile of the route to estimate operational requirements
- A route analysis to investigate existing easement constraints, potential utility conflicts, anticipated permitting, and coordination with other entities, and potential concerns.
- An environmental evaluation of the project extents to determine potential impacts and additional study requirements.
- An opinion of probable construction cost (OPCC) estimated for each alternative based on pipe length, joint restraint, laying conditions and required appurtenances.

Alternative 1 was chosen based on the criteria presented in the evaluation. From that, a 36-inch pipe was determined to be optimal for the design flow rates. Alternative 1 probable construction cost is estimated at \$44,282,205.

## **2.0 Introduction**

The Coweta County Water and Sewerage Authority (CCWSA) analyzed current water supply sources, recent droughts, and forecasted future needs through the year 2065 in their April 2017 Forecast of Water Needs Report (Metropolitan North Georgia Water Planning District 2017). Based on the forecast, CCWSA should increase its long-term water supply from 10 million gallons per day (MGD) to 20 MGD annual average daily (AAD) to meet future demand. In April 2020, CCWSA requested an amendment to the Water Resource Management Plan adopted by the Metropolitan North Georgia Water Planning District (District) in 2017 to include this additional need.

This proposed Long-Term Water Supply includes a proposed pump station that will withdraw raw water on the Chattahoochee River and transport the raw water to the BT Brown Reservoir where the filter plant will supply drinking water to CCWSA customers. This proposed pump station will pump the raw water 9.5 miles along a route to the 300-plus-acre B.T. Brown Reservoir, an impoundment on Alexander Creek, thus increasing the reservoir's safe yield.

CCWSA directed Garver to evaluate the engineering and costs for a raw water pipeline from the proposed pump station to the B.T. Brown Reservoir. In this report, Garver presents three alternative routes with an accompanying evaluation for each. Later, we compare the alternatives and determine a preferred alternative for review with CCWSA.

## **2.1 Information Collection**

Aerial photography, GIS data, and field reconnaissance was collected on the existing utilities and their associated structures. Garver contacted the impacted utilities including natural gas, water, electric, petroleum, and cable to better understand their systems' locations along the potential corridors.

## **3.0 Future Flows**

### **3.1 Model Development**

A hydraulic model of the pipeline was developed and evaluated using Bentley's WaterGEMS hydraulic modeling software. Hazen-Williams roughness coefficients of 120 and 140 were used throughout the hydraulic evaluation. The preferred alignment developed in ArcGIS Pro was imported into the model, and topographic data from the United States Geological Survey was used to develop a vertical profile. Flows of 21 MGD and 30 MGD were used for the initial phase and future buildout phase, respectively. Pipe velocities for 30-inch, 36-inch, and 42-inch mains are presented in Table 3-1.

**Table 3-1: Pipe Velocities**

<b>Pipe Size (in)</b>	<b>Pipe Velocity (ft/s)</b>	
	<b>21 MGD</b>	<b>30 MGD</b>
<b>30</b>	6.58	9.54
<b>36</b>	4.61	6.60
<b>42</b>	3.37	4.79

30-inch pipe diameter was determined to be too small based on the velocity of 9.5 ft/s and the associated head loss and impacts to pump size requirements at the design flow rate of 30 MGD. Based on the hydraulic model, pipe diameters of 36-inch and 42-inch are feasible options, but a 42-inch pipe is oversized, which may result in deposition of solids along the raw water line due to the relatively low velocities, particularly for initial flows. A 36-inch pipe was determined to be optimal for the design flow rates.

## **4.0 Environmental Evaluation**

As part of the preliminary design, Garver performed a high-level environmental assessment of the alignments. This was intended to determine which alignment was least impactful from an environmental permitting perspective.



#### 4.1 Floodplain

Portions of the study location are located in the 100-year floodplain (FEMA Firm Map 13077C0130D and 13077C0135D, dated February 6, 2013). These are shown in Figure 4-1. There are non-floodplain areas located along State Route 16, then along the Georgia Power easement from SR 16 to Sewell Mill Rd, and between Walt Carmichael Rd to Brimer Rd. FEMA floodplain requirements will be necessary for this project.

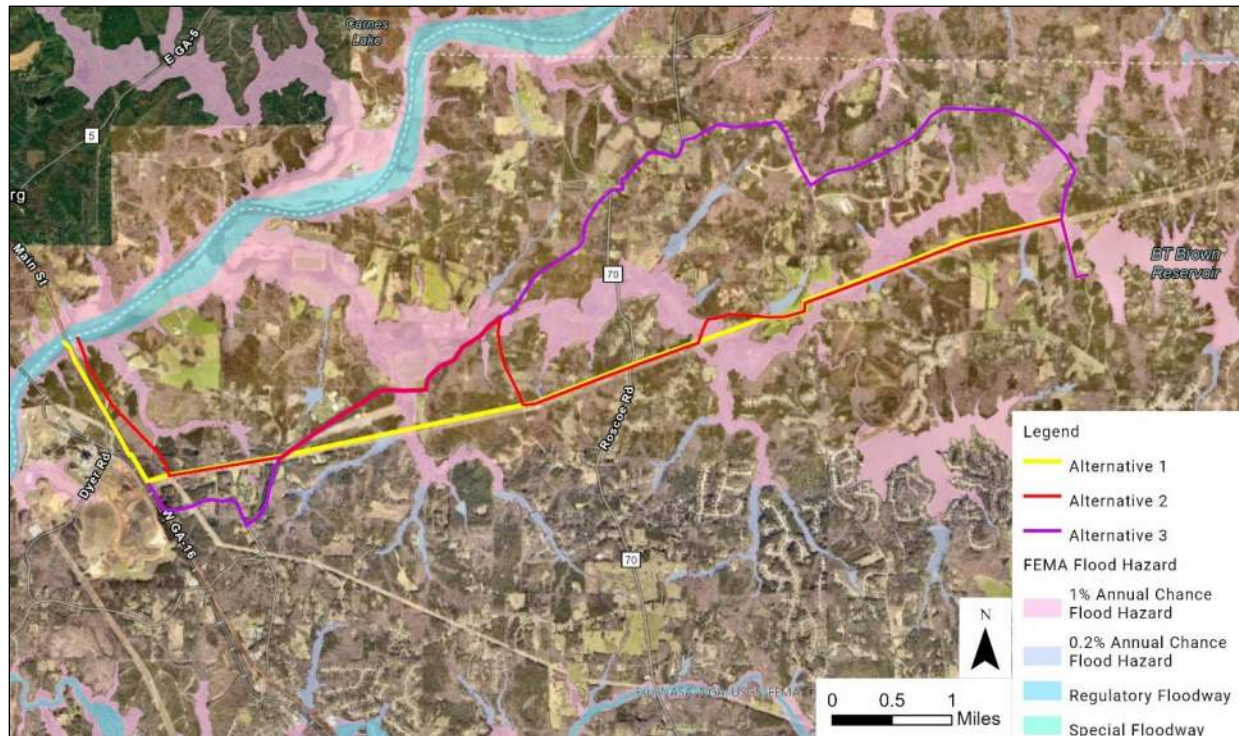


Figure 4-1: Floodplains

#### 4.2 Waters of the U.S (WOUS).

According to the U.S. Fish and Wildlife Services' (USFWS) National Wetland Inventory (NWI), there are two emergent wetlands at the north end of the project, one east of Jim Star Rd and the other west of Lake Ridge Rd. There is one forested / shrub wetland located south of the Panther Creek airport. There are 14 streams located within the study area according to NWI data. The WOUS effected by the alignments are shown in Figure 4-2. A Preliminary Jurisdictional Determination is recommended for this project to confirm any NWI findings and to determine potential impacts to WOUS.

If the impacts to WOUS do no result in a loss greater than 1/2-acre, the project is automatically covered by the U.S. Army Corps of Engineers (USACE) Nationwide Permit 58 – Utility Line Activities. If the impacts are greater than 1/10-acre, a Pre-Construction Notification (PCN) is required from the USACE

and potentially mitigation will be required. If impacts are over a half-acre, the project will require an Individual Permit and will require mitigation.

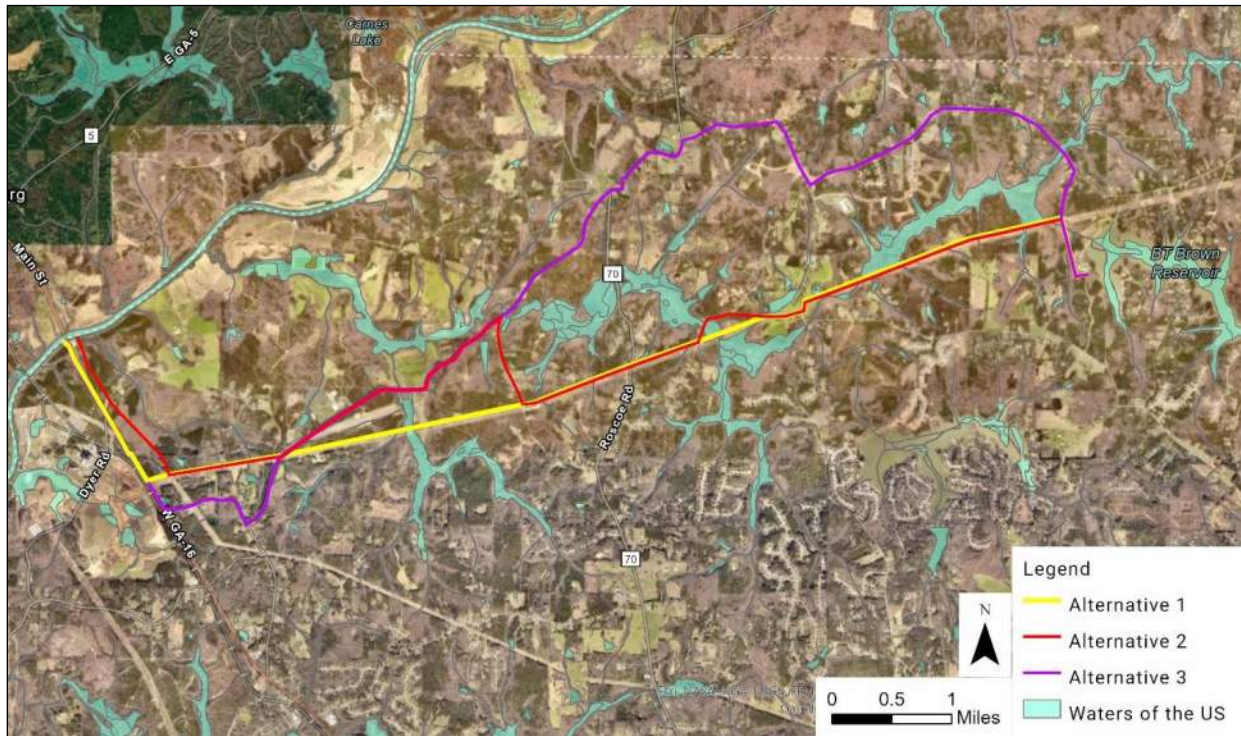


Figure 4-2: Waters of the US

#### 4.3 Hazardous Materials

Based on aerial imagery and a field walk, the Georgia Power plant Yates will be along the proposed pipeline alignments 1 and 2. This property was potentially used to store hazardous materials in the past. Although it appears that impacts from hazardous materials would be minimal, an order of a hazardous materials record database to determine any hazardous material locations should be completed.

#### 4.4 Threatened and Endangered Species

Georgia Fish and Wildlife reports there are several local threatened and endangered species with that potentially have habitat within the study area. Due to the construction being a temporary impact within the easement, it is not likely there would be significant impact. A habitat assessment field visit is recommended to support this during the design process. It is also recommended that Information for Planning and Consultation (IPaC) be requested from the U.S. Fish and Wildlife Services to ensure that there is no critical habitat for federally listed species in the study area.



## 5.0 Permitting and Coordination

The proposed raw water pipeline will require coordination and permit application with GDOT, Atlanta Gas Light, Kinder Morgan, Georgia Power, USACE, and EPD. The following subsections describe the preliminary understanding of the permit application requirements with these entities.

### 5.1 Georgia Department of Transportation (GDOT)

The proposed raw water pipeline alignment 1 and 2 will be crossing SR 16. It is anticipated that SR 16 will be crossed via trenchless methods to avoid excavation in the ROW. A utility encroachment permit application and a set of plans will need to be provided utilizing Georgia Utilities Permitting System (GUPS) to GDOT's district utilities engineer for the area.

### 5.2 Atlanta Gas Light and Kinder Morgan

The proposed raw water pipeline will cross the Atlanta Gas Light (AGL) high pressure natural gas pipeline four times and the Kinder Morgan petroleum pipeline once. The petroleum line crossing is located west of Rex Hyde Rd. The proposed raw water pipeline will be closely coordinated with these existing utilities if encroaching on their easement or when crossing the lines. Figure 5-1 shows the approximate locations of the existing gas and petroleum lines in relation to the proposed raw water pipeline.

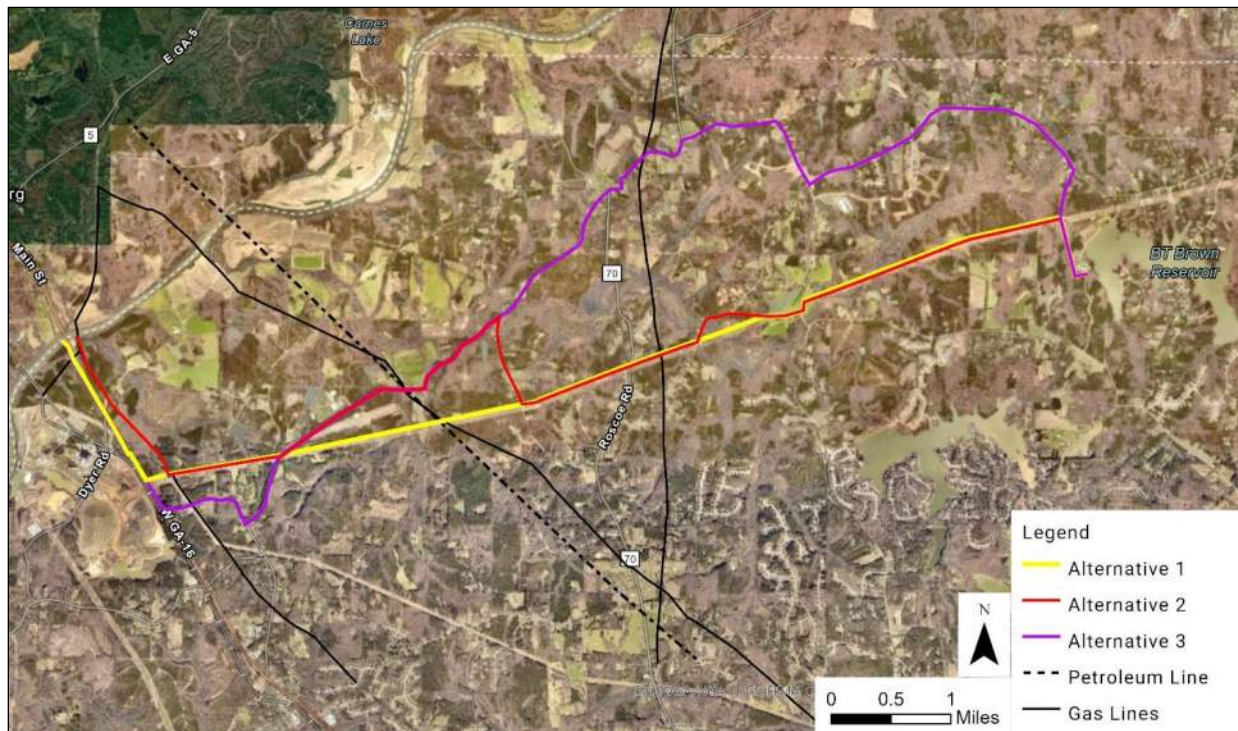


Figure 5-1: Utility Locations

### 5.3 United States Army Corps of Engineers

The proposed raw water pipeline will cross 14 identified stream crossings and three identified wetlands. Further analysis should be performed in the design process to determine the total impacts to jurisdictional areas, but it is anticipated that the impacts can be minimized to avoid a major permitting effort. If more than 1/10-acres of WOUS is impacted, a PCN will be required. Once the PCN is submitted, the USACE would review the project impacts to approve or request more information to complete the PCN. This review process can take up to 45 days.

### 5.4 Georgia Power

The proposed raw water pipeline will cross under the Georgia Power overhead power lines. To encroach on these easements, a Request for Easement permit will be made to Georgia Power Company by email. A site visit with the final plans must be scheduled with the Right of Way services division to meet and discuss their guidelines prior to any final decisions. This application process could take up to 3 months.

## 6.0 Alternatives Analysis

Garver provided CCWSA with three alternative alignments from the proposed pump station location at the Chattahoochee River to the B.T. Brown Reservoir during a route study workshop in July 2022. FrighGarver presented all three alternative alignments shown in Figure 6-1 and recommends Alternative 1 as the preferred route to CCWSA.

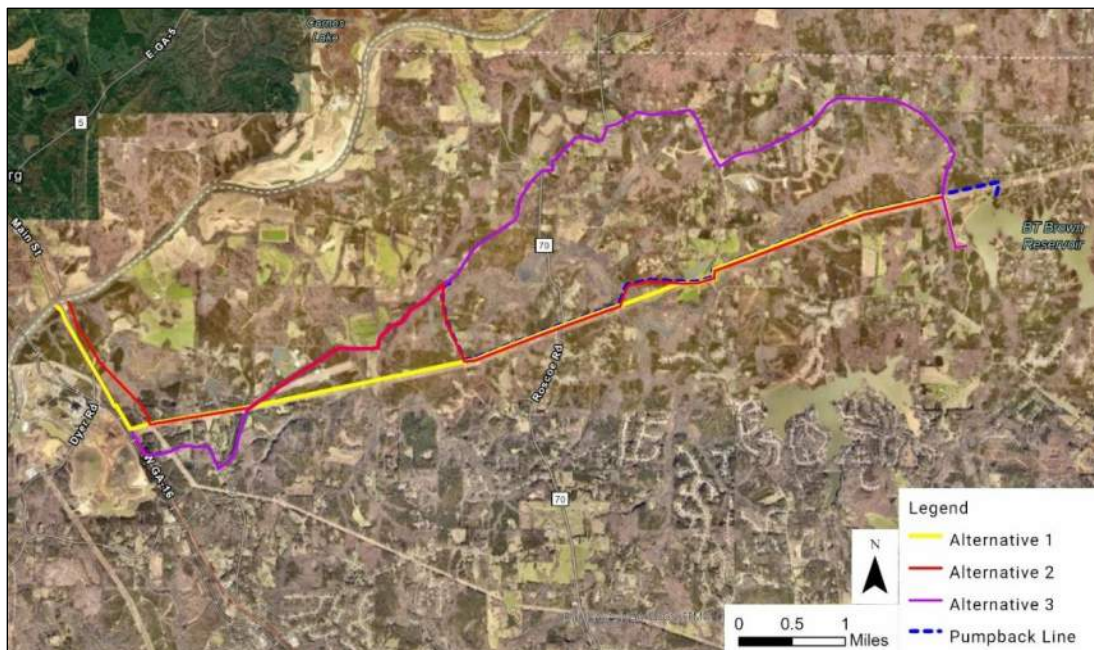


Figure 6-1: Alternative Alignments



## 6.1 Alternative 1 - Following the Georgia Power Right-Of-Way

### 6.1.1 Overview

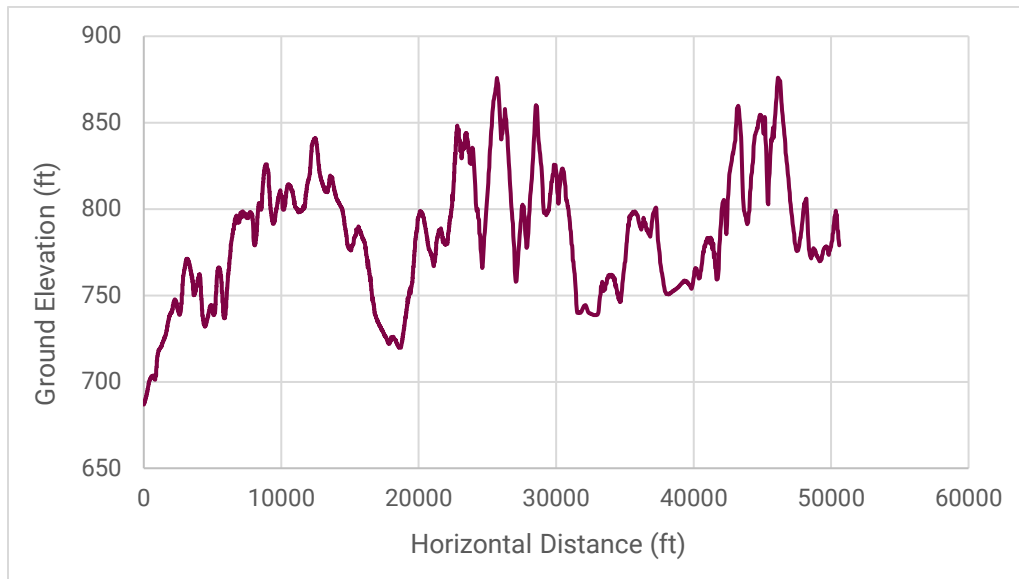


**Figure 6-2: Alternative 1 Alignment**

Alternative 1, shown in Figure 6-2, aligns the new 36-inch raw water pipeline parallel the west side of SR 16 in the ROW until it reaches the Georgia Power high voltage transmission easements. The pipeline would run along the north side of the Georgia Power transmission easement in a new 20-foot easement to be acquired from the individual property owners. The pipeline would run parallel along the north side of the Georgia Power easement until it crosses over Duncan Rd. At Duncan Rd the pipeline would then follow the Duncan Rd ROW along the south side until it dead ends into Jim Starr Rd. At this point the pipeline would cross under Jim Starr Rd and run along the east side of the ROW until it again reached the north side of the Georgia Power transmission easement. The pipeline would then continue along the north side of the Georgia Power transmission easement in a new 20-foot easement to be acquired from the individual property owners until it reached the B.T. Brown reservoir. At this point, it would run perpendicular across the transmission power line to connect the existing Lamar Smith Drive raw water line with the option of going into the B.T. Brown Reservoir or directly to the water treatment plant.

### 6.1.2 Alignment Profile

The profile for Alternative 1, shown in Figure 6-3, requires up to 44 air release valves and 31 blow-off valves due to elevation changes.



**Figure 6-3: Alternative 1 Profile**

### 6.1.3 Alignment Analysis

Advantages of the alignment:

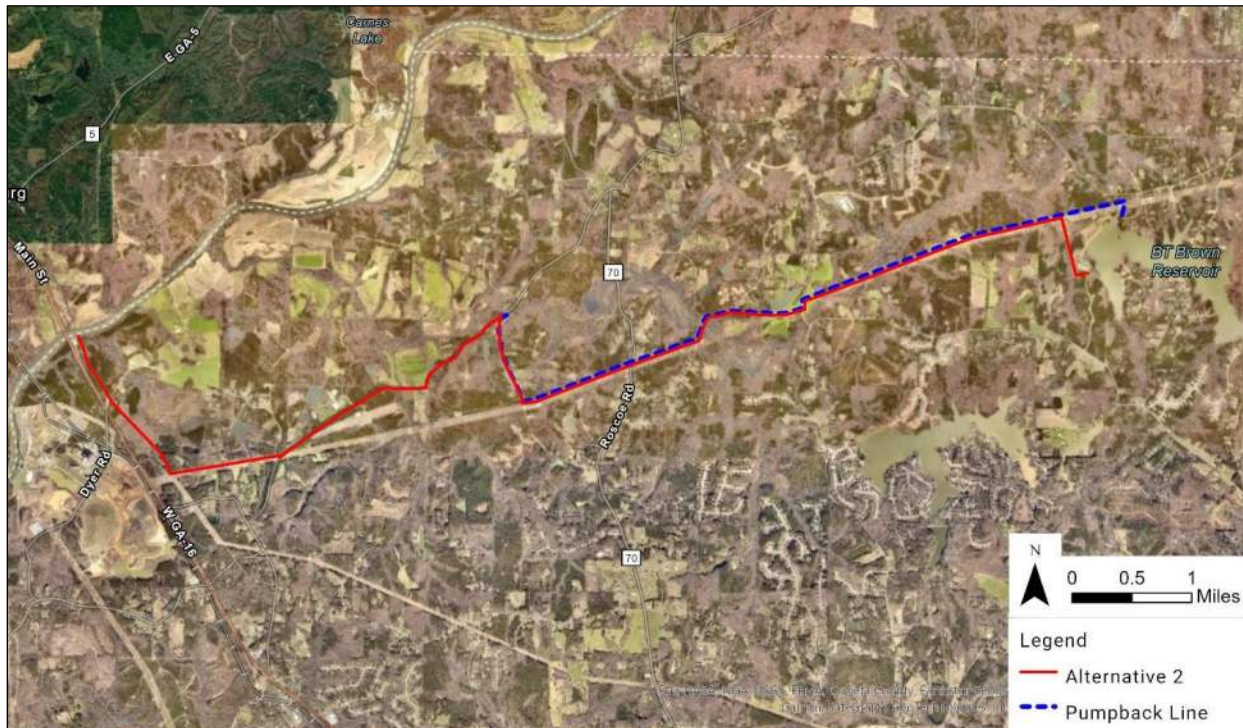
- Shortest length for the total alignment
- Lowest cost of piping overall due to the length
- Least number of easements required
- Fewest number of bends in the alignment to reduce head loss, thus the cost to operate is lower
- Less fittings, thus less restrained joint pipe
- Limited traffic impacts
- Least number of valves

Disadvantages of the alignment:

- GDOT approval will be required along SR 16
- Future access will be difficult due to limited access and fencing along the Georgia Power easement
- Construction easement required at Georgia Power incursion

## 6.2 Alternative 2 – Following the AGL line and Pumpback Station line

### 6.2.1 Overview



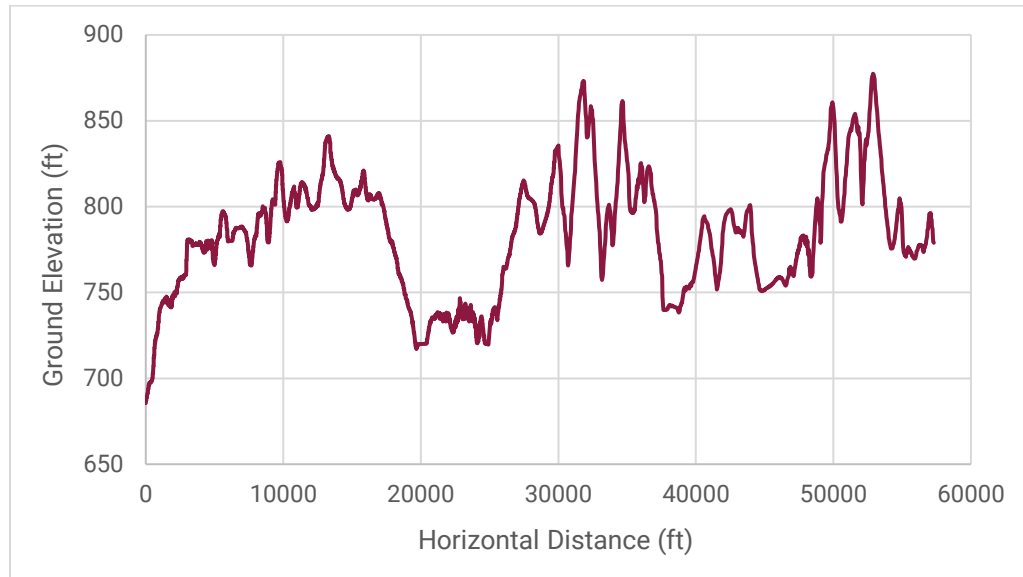
**Figure 6-4: Alternative 2 Alignment**

Alternative 2, shown in Figure 6-4, purposes running the new raw water pipeline parallel to the Atlanta Gas Light pipeline along the west side, in a new 20-foot easement to be acquired from the individual property owners, until it reaches the Georgia Power high voltage transmission easements. The pipeline would run along the north side of the Georgia Power transmission easement in a new 20-foot easement to be acquired from the individual property owners until it reaches Sewell Mill Rd. The pipeline would then follow Sewell Mill Rd along the south ROW until it reaches Walt Carmichael Rd. The pipeline would then follow the same alignment as the 18-inch pumpback station pipeline, along the west side of Walt Carmichael Rd ROW. When this road crosses the Georgia Power transmission easement, the raw water pipeline would again run parallel along the north side of this easement. When the pipeline reaches Brimer Rd, it would continue to follow the 18-inch pumpback pipe along the west side ROW of Brimer Rd. At Duncan Rd, the pipeline would then follow the Duncan Rd ROW along the south side until it dead ends into Jim Starr Rd. At this point, the pipeline would cross under Jim Starr Rd and run along the east side of the ROW until it again reached the north side of the Georgia Power transmission easement. The pipeline would then run along the north side of the Georgia Power transmission easement in a new 20-foot easement to be acquired from the individual property owners until it reaches the B.T. Brown reservoir. At this point, it will run perpendicular across the transmission power line to connect to the existing Lamar

Smith Drive raw water line with the option of going into the B.T. Brown Reservoir or directly to the water treatment plant.

### 6.2.2 Alignment Profile

The profile for Alternative 2, shown in Figure 6-5, requires up to 42 air release valves and 38 blow-off valves due to elevation changes.



**Figure 6-5: Alternative 2 Profile**

### 6.2.3 Alignment Analysis

Advantages of the alignment:

- No coordination with GDOT
- Existing 10-foot easement available
- Previous success with the property easement purchased along the pumpback pipe alignment

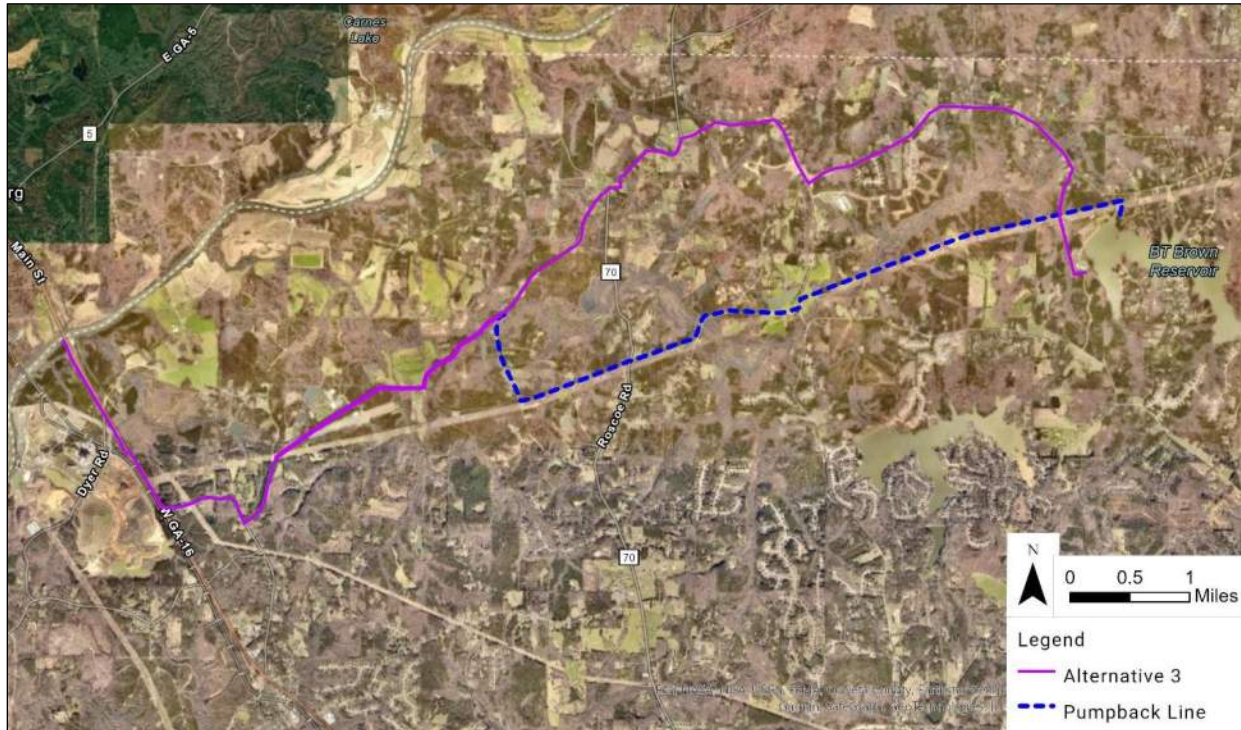
Disadvantages of the alignment:

- Most valves needed
- Cathodic protection required by proximity to the AGL high pressure gas pipeline
- Future access will be difficult due to limited access and fencing along the Georgia Power easement
- Construction easement required at Georgia Power incursion



### 6.3 Alternative 3 – Following the Right-of-Way

#### 6.3.1 Overview

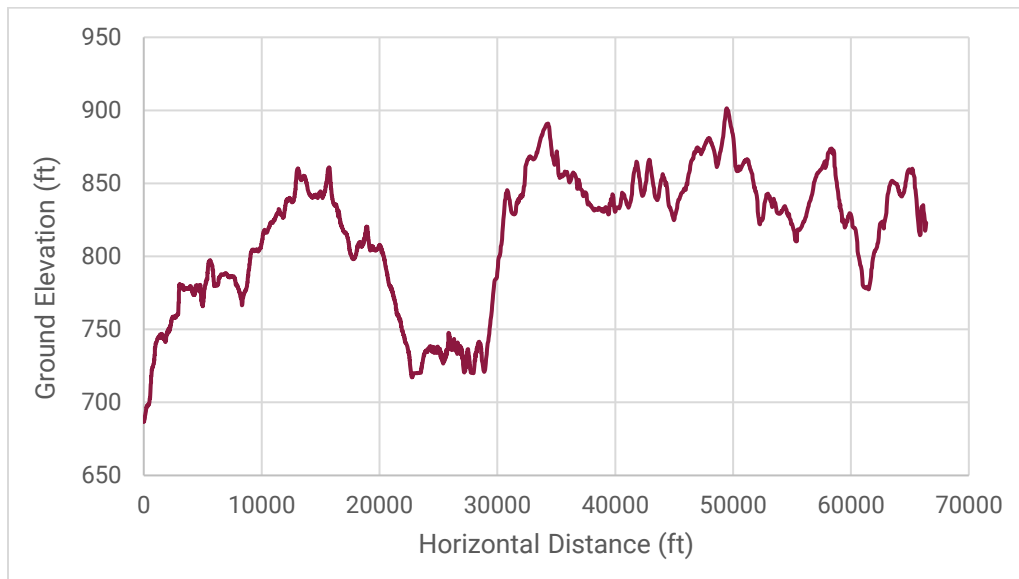


**Figure 6-6: Alternative 3 Alignment**

Alternative 3 only follows State and County ROWs. The 36-inch raw water pipeline would parallel the west side SR 16 in the ROW until it reaches Robinson Rd. The pipeline would then follow Robinson Rd, Old Carrolton Hwy, Sewell Mill Rd, Roscoe Rd, Jim Starr Rd and Tommy Lee Cook Rd ROWs until it reaches the B.T. Brown Reservoir. At this point, it would run perpendicular across the transmission power line to connect the existing Lamar Smith Drive raw water line with the option of going into the B.T. Brown Reservoir or directly to the water treatment plant.

#### 6.3.2 Alignment Profile

The profile for Alternative 3, shown in Figure 6-7, requires up to 45 air release valves and 33 blow-off valves due to elevation changes.



**Figure 6-7: Alternative 3 Profile**

### 6.3.3 Alignment Analysis

Advantages of the alignment:

- Access for future maintenance is accessible via GDOT or county roads
- It would reduce necessary easement width using ROW

Disadvantages of the alignment:

- Longest length for the total alignment
- Highest cost overall due to the length
- Travel for residents will be temporarily impacted
- Slower construction along the county road; longer construction time
- Most bends in the alignment to increase head loss, thus a higher cost to operate
- Greatest number of fittings requiring more restrained joints
- Potentially higher horsepower pumps need due to the length

## 7.0 Recommendation

As detailed in this route study, three alternative alignments were evaluated for the proposed raw water pipeline route. These three alignments are summarized below:

- Alternative 1: Install a new 36-inch pipeline that parallels GA Route 16 and the Georgia Power transmission easement.

- Alternative 2: Install a new 36-inch pipeline that parallels the Atlanta Gas Light pipeline and the existing 18-inch pumpback station pipeline.
- Alternative 3: Install a new 36-inch pipeline that in the ROW of GA Route 16 and county roads.

In Table 7-1, there is a direct comparison between the three alternative alignments. The length, cost estimate, and number of easements were gathered for each alignment. From the table, it can be determine that Alternative 1 is the shortest alignment in total length and this results in the lowest cost estimate for a 36-inch pipeline. Alternative 1 also has the fewest number of to be acquired easements needed due to it largely staying away from the more densely-populated residential areas. From this information and Garver's previous preferred alternative recommendation in the route study workshop, CCWSA determined that Alternative 1 preferred route for this project.

**Table 7-1: Alternative Alignment Comparisons**

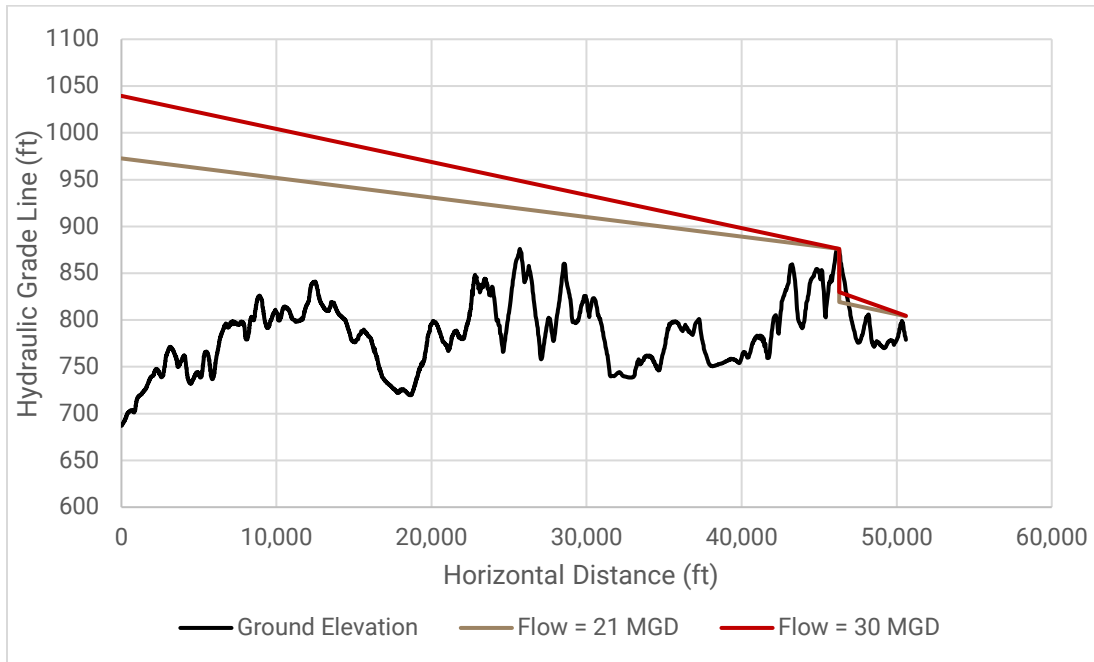
	<b>Length (ft)</b>	<b>30-inch Cost Estimate*</b>	<b>36-inch Cost Estimate*</b>	<b>42-inch Cost Estimate*</b>	<b>Number of Easements</b>
<b>Alternative 1</b>	51,600	\$24,750,000	\$29,700,000	\$34,650,000	55
<b>Alternative 2</b>	56,600	\$27,150,000	\$32,600,000	\$38,050,000	59
<b>Alternative 3</b>	66,300	\$31,850,000	\$38,200,000	\$44,550,000	117
*Cost estimate developed with a \$16.00/inch per linear foot factor					
*Easement cost is not included in cost estimate					

## **7.1 Alternative 1 - Following the Georgia Power transmission easement**

With CCWSA selecting Alternative 1 as the route to move forward with, Garver then developed other design components for this route, including hydraulics, air relief valves, and blow-off valves.

### **7.1.1 Hydraulic evaluation**

The hydraulic profile of the raw water line shown in Figure 7-1 presents the hydraulic grade line for the different design flow rates based on head loss through a 36-inch pipe with an assumed roughness coefficient (C) of 120. Additional high points throughout the line or changes in pipe size would impact the hydraulic profile.



**Figure 7-1: 36-inch Raw Water Main Hydraulic Profile (C = 120)**

#### 7.1.2 Maximum Pressure

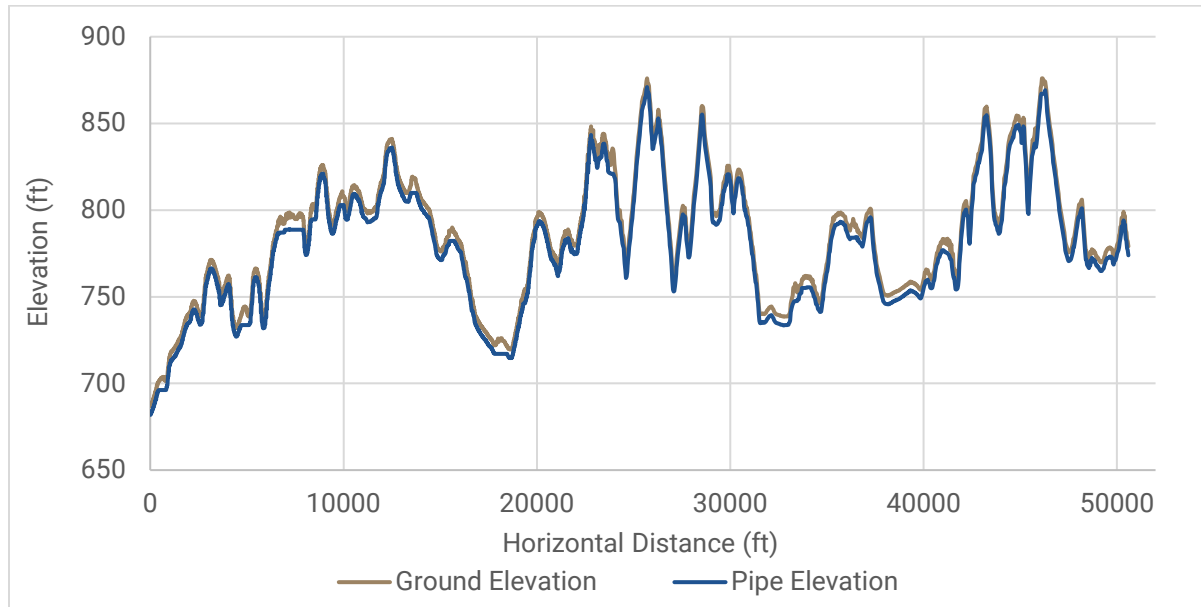
Maximum estimated pressures for 30-inch, 36-inch, and 42-inch mains are presented in Table 7-2. These pressures were determined based on hydraulic profiles developed with a Hazen-Williams roughness coefficient of 120, which is a typical value for the long-term condition of raw water lines.

**Table 7-2: Maximum Pressures (C=120)**

Pipe Size (in)	Maximum Pressure (psi)	
	21 MGD	30 MGD
<b>30</b>	175	268
<b>36</b>	122	151
<b>42</b>	99	116

#### 7.1.3 Pipe Profile

In Figure 7-2, the pipe profile for Alternative 1 is shown with the minimum of 5-foot of cover above the pipe in relation to the ground elevation. With this pipe elevation, it would create a need for 44 air release valves and 31 blow-off valves.



**Figure 7-2: Pipe Profile for Alternative 1**

#### 7.1.3.1 Air Release Valve

Air release valves are installed at the at local high points in a pipeline where air naturally collects. The greater the number of high points that are in a pipeline, the more air release valves will be needed to vent the accumulated air into the atmosphere and maintain hydraulic capacity of the system. Garver was able to develop an alternate vertical profile using a maximum cover of 14.5-feet, shown in Table 7-3. By doing this, it allows the pipeline to reduce the number of high points along the system. This change allows Garver the design capability of removing 9 air release valves. The number of air release valves needed for this system would be 35 if the maximum depth of 14.5 feet is designed.

**Table 7-3: Air Release Valves and Blow-off Valves**

	<b>Standard 5-foot Cover</b>	<b>Maximum 14.5-foot Cover</b>
<b>Air Release Valves Required</b>	44	35
<b>Blow-off Valves Required</b>	31	24

#### 7.1.3.2 Blow-Off Valves

Blow-off valves are installed in the lowest local points of a pipeline to be used to flush the pipeline during testing, maintenance, and repairs. These blow-off valves help to remove debris and sediment that collect in the system. In Table 7-3, Garver reduced the number of low points within the pipeline by allowing up to

14.5-feet of cover. This reduces the number of required blow-off valves from 31 to 24, which results in lower capital and O&M costs.

#### 7.1.4 Existing Easements

The route for Alternative 1 would include the option of utilizing the existing 10-foot easement that CCWSA already owns for the 18-inch pumpback station line. This easement could be utilized along the Georgia Power transmission easement from Walt Carmichael Rd to Brimer Rd and then from Duncan Rd to B.T. Brown Reservoir. The easement would need to be expanded to 20-foot in total to allow for both the existing 18-inch pipe and the proposed 36-inch raw water pipeline.

#### 7.1.5 OPCC

### Coweta County Water Sewerage Authority

#### ENGINEER'S ESTIMATE OF PROBABLE COST -SCHEDULE 1

ITEM NO.	DESCRIPTION	UNIT	ESTIMATED QUANTITY	Engineer's Estimate of Probable Cost	
				UNIT PRICE	AMOUNT
1	Furnish & Install 36-in. Pipe	LF	51,854	\$576.00	\$29,867,904
2	8" Blow Off Valve	EA	31	\$3,383.00	\$104,873
3	4" Air Release Valve	EA	35	\$2,516.00	\$88,060
4	36" Butterfly Valve	EA	7	\$24,767.00	\$173,369
5	Manhole	EA	66	\$8,000.00	\$528,000
6	36" Trenchless Crossing in 48" Casing	LF	595	\$1,199.00	\$713,405
7	Trenching, Backfilling, and Compaction	LF	51,259	\$5.00	\$256,295
8	SWPPP	LF	51,854	\$5.00	\$259,270
9	Mobilization/Demobilization	LS	1	\$450,000.00	\$450,000
10	Miscellaneous	LS	5%		\$1,622,059
11	Contingency on Construction Subtotals	LS	30%		\$10,218,970
12	Escalation of Cost to Midpoint Construction (3 years at 5%)	LS	5%		\$6,979,983

**Total Engineer's Estimate - Schedule 1** \$44,282,205

**Total Engineer's Estimate with Escalation - Schedule 1** \$51,262,188

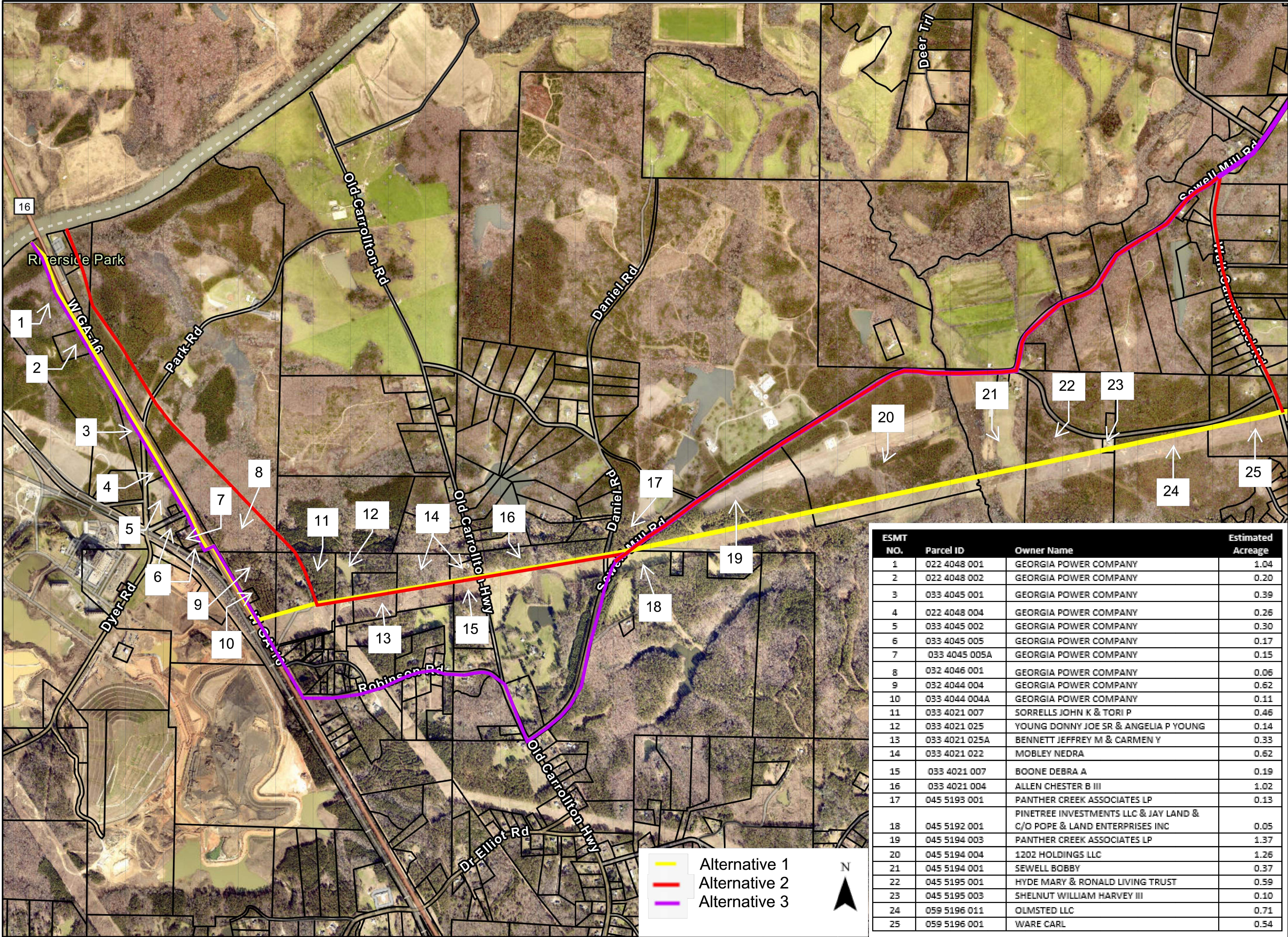
*Schedule 1 doesn't include the cost for easement acquisitions and temporary construction easements.*

# APPENDIX A


## Alternative 1 Route Alignment and Parcel Information



File: L:\2022\22W38085 - Coweta County Chattahoochee Water\Deliverables\Technical Memo\CRWPS-01-G000.dwg Last Save: 9/15/2022 9:43 AM Last saved by: JLEvans  
Last plotted by: Evans, Josh L Plot Style: AEOnono.ctb Plot Scale: 1/2"=50'-0" Plot Date: 9/21/2022 1:42 PM Plotter used: None



ESMT NO.	Parcel ID	Owner Name	Estimated Acreage
1	022 4048 001	GEORGIA POWER COMPANY	1.04
2	022 4048 002	GEORGIA POWER COMPANY	0.20
3	033 4045 001	GEORGIA POWER COMPANY	0.39
4	022 4048 004	GEORGIA POWER COMPANY	0.26
5	033 4045 002	GEORGIA POWER COMPANY	0.30
6	033 4045 005	GEORGIA POWER COMPANY	0.17
7	033 4045 005A	GEORGIA POWER COMPANY	0.15
8	032 4046 001	GEORGIA POWER COMPANY	0.06
9	032 4044 004	GEORGIA POWER COMPANY	0.62
10	033 4044 004A	GEORGIA POWER COMPANY	0.11
11	033 4021 007	SORRELLS JOHN K & TORI P	0.46
12	033 4021 025	YOUNG DONNY JOE SR & ANGELIA P YOUNG	0.14
13	033 4021 025A	BENNETT JEFFREY M & CARMEN Y	0.33
14	033 4021 022	MOBLEY NEDRA	0.62
15	033 4021 007	BOONE DEBRA A	0.19
16	033 4021 004	ALLEN CHESTER B III	1.02
17	045 5193 001	PANTHER CREEK ASSOCIATES LP	0.13
18	045 5192 001	PINETREE INVESTMENTS LLC & JAY LAND & C/O POPE & LAND ENTERPRISES INC	0.05
19	045 5194 003	PANTHER CREEK ASSOCIATES LP	1.37
20	045 5194 004	1202 HOLDINGS LLC	1.26
21	045 5194 001	SEWELL BOBBY	0.37
22	045 5195 001	HYDE MARY & RONALD LIVING TRUST	0.59
23	045 5195 003	SHELNUT WILLIAM HARVEY III	0.10
24	059 5196 011	OLMSTED LLC	0.71
25	059 5196 001	WARE CARL	0.54




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REGISTRATION NO.  
PEF007804

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BY	
DESCRIPTION	
DATE	
REV.	



COWETA COUNTY WATER & SEWERAGE AUTHORITY  
NEWNAN, GA

COWETA COUNTY  
CHATTAHOOCHEE RIVER  
INTAKE, PUMP STATION  
AND RAW WATER PIPELINE

JOB NO.: 22W38085  
DATE: SEPT. 2022  
DESIGNED BY:  
DRAWN BY:  
CHECKED BY:

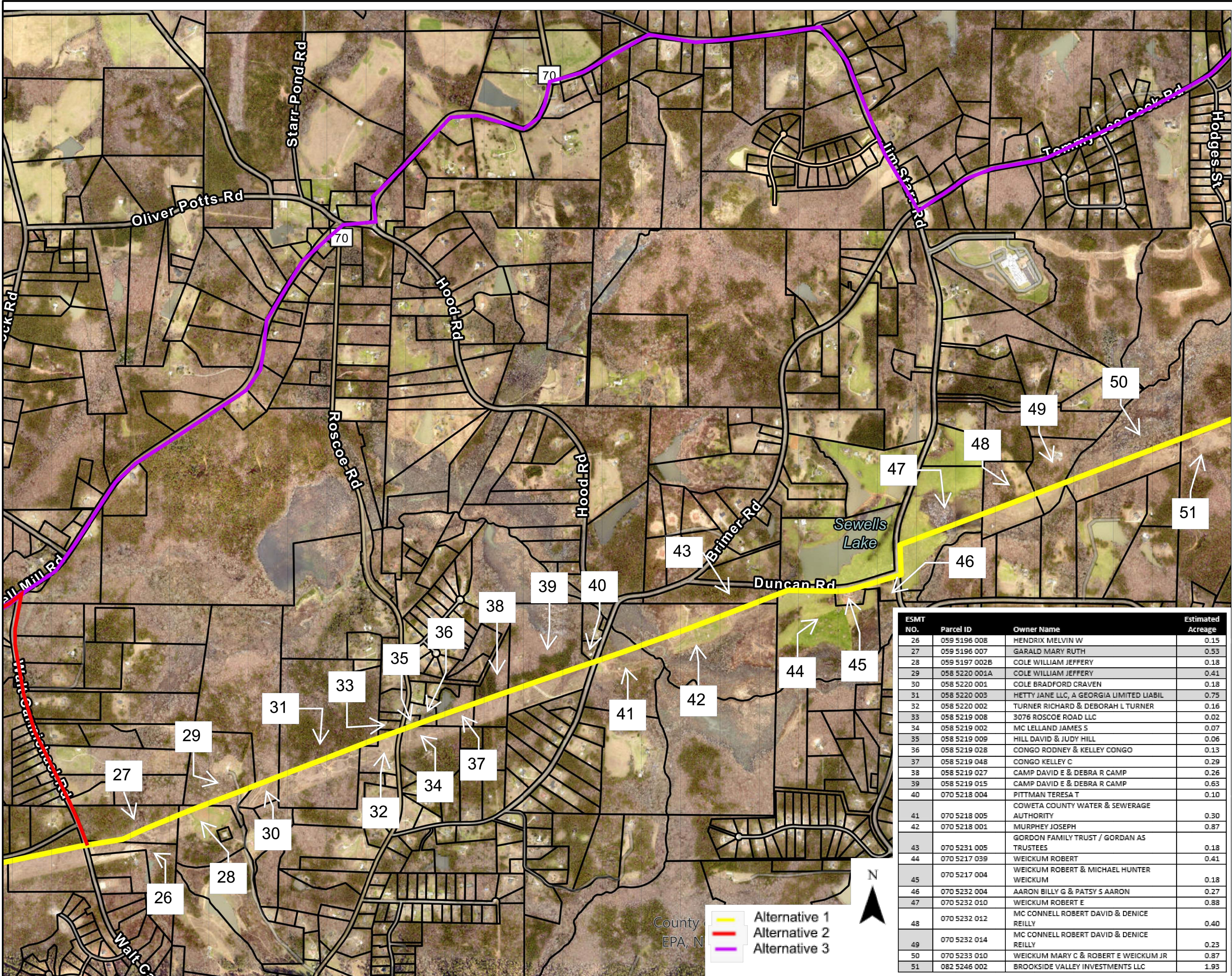
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DRAWING NUMBER

SHEET NUMBER



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Last plotted by: Evans, Josh L Plot Style: AECmon.ctb Plot Scale: 1:2,500 Plot Date: 9/21/2022 1:43 PM Plotter used: None



ESMT NO.	Parcel ID	Owner Name	Estimated Acreage
26	059 5196 008	HENDRIX MELVIN W	0.15
27	059 5196 007	GARALD MARY RUTH	0.53
28	059 5197 002B	COLE WILLIAM JEFFERY	0.18
29	058 5220 001A	COLE WILLIAM JEFFERY	0.41
30	058 5220 001	COLE BRADFORD CRAVEN	0.18
31	058 5220 003	HETTY JANE LLC, A GEORGIA LIMITED LIABIL	0.75
32	058 5220 002	TURNER RICHARD & DEBORAH L TURNER	0.16
33	058 5219 008	3076 ROSCOE ROAD LLC	0.02
34	058 5219 002	MC LELLAND JAMES S	0.07
35	058 5219 009	HILL DAVID & JUDY HILL	0.06
36	058 5219 028	CONGO RODNEY & KELLEY CONGO	0.13
37	058 5219 048	CONGO KELLEY C	0.29
38	058 5219 027	CAMP DAVID E & DEBRA R CAMP	0.26
39	058 5219 015	CAMP DAVID E & DEBRA R CAMP	0.63
40	070 5218 004	PITTMAN TERESA T	0.10
41	070 5218 005	COWETA COUNTY WATER & SEWERAGE AUTHORITY	0.30
42	070 5218 001	MURPHEY JOSEPH	0.87
43	070 5231 005	GORDON FAMILY TRUST / GORDAN AS TRUSTEES	0.18
44	070 5217 039	WEICKUM ROBERT	0.41
45	070 5217 004	WEICKUM ROBERT & MICHAEL HUNTER WEICKUM	0.18
46	070 5232 004	AARON BILLY G & PATSY S AARON	0.27
47	070 5232 010	WEICKUM ROBERT E	0.88
48	070 5232 012	MC CONNELL ROBERT DAVID & DENICE REILLY	0.40
49	070 5232 014	MC CONNELL ROBERT DAVID & DENICE REILLY	0.23
50	070 5233 010	WEICKUM MARY C & ROBERT E WEICKUM JR	0.87
51	082 5246 002	BROOKSIDE VALLEY INVESTMENTS LLC	1.93

- Alternative 1
- Alternative 2
- Alternative 3

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BY

DESCRIPTION

DATE

REV.

COWETA COUNTY WATER & SEWERAGE AUTHORITY  
NEWNAN, GA

COWETA COUNTY CHATTAHOOCHEE RIVER INTAKE, PUMP STATION AND RAW WATER PIPELINE

JOB NO.: 22W38085

DATE: SEPT. 2022

DESIGNED BY:

DRAWN BY:

CHECKED BY:

BAR IS ONE INCH ON ORIGINAL DRAWING

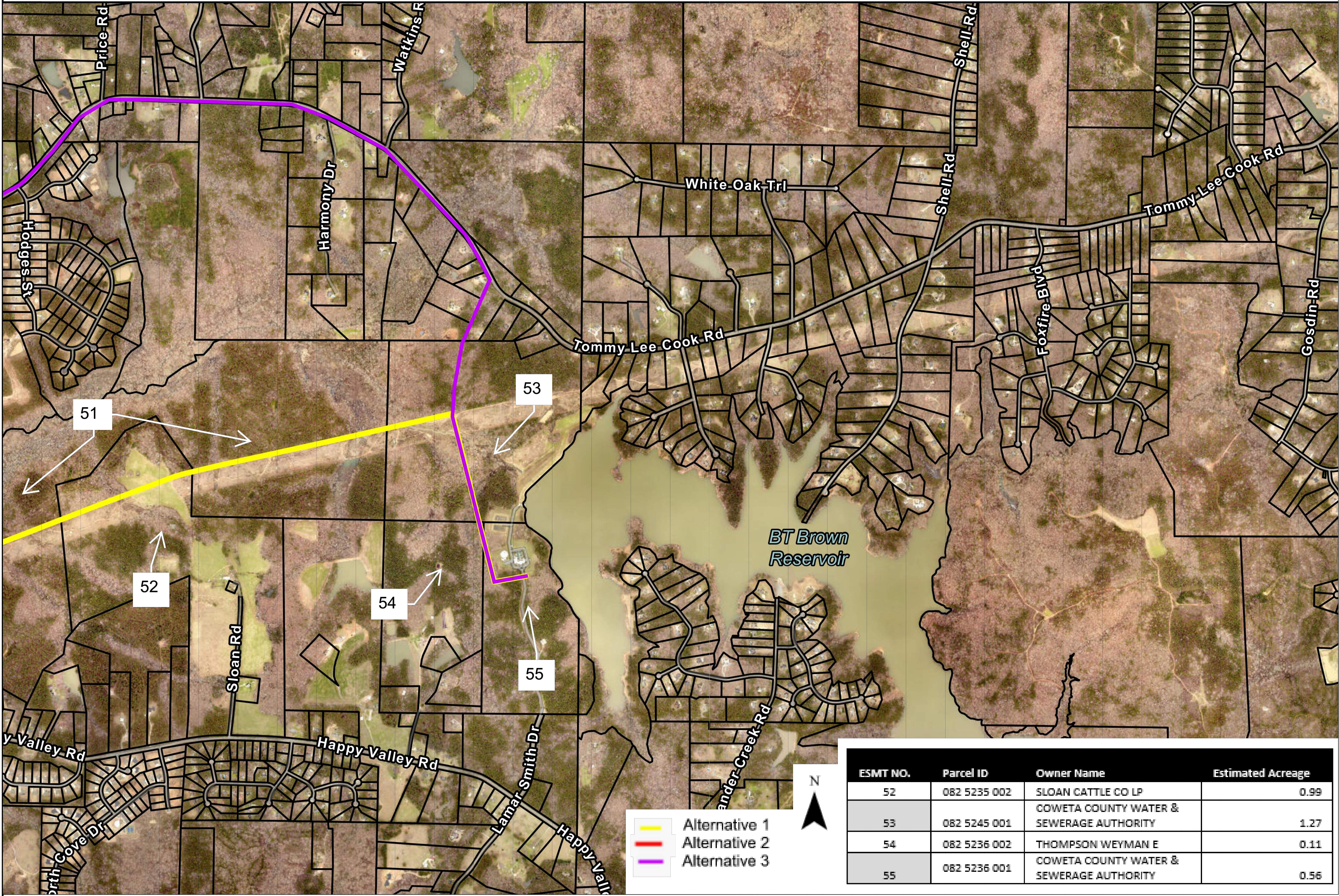
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SHEET NUMBER




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Last plotted by: Evans, Josh L Plot Style: AEC.ctb Plot Date: 9/21/2022 1:43 PM Plotter used: None



- Alternative 1
- Alternative 2
- Alternative 3

ESMT NO.	Parcel ID	Owner Name	Estimated Acreage
52	082 5235 002	SLOAN CATTLE CO LP	0.99
53	082 5245 001	COWETA COUNTY WATER & SEWERAGE AUTHORITY	1.27
54	082 5236 002	THOMPSON WEYMAN E	0.11
55	082 5236 001	COWETA COUNTY WATER & SEWERAGE AUTHORITY	0.56




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REV.	DATE	DESCRIPTION	BY



COWETA COUNTY WATER & SEWERAGE AUTHORITY  
NEWNAN, GA

COWETA COUNTY  
CHATTAHOOCHEE RIVER  
INTAKE, PUMP STATION  
AND RAW WATER PIPELINE

JOB NO.: 22W38085  
DATE: SEPT. 2022  
DESIGNED BY:  
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# APPENDIX B

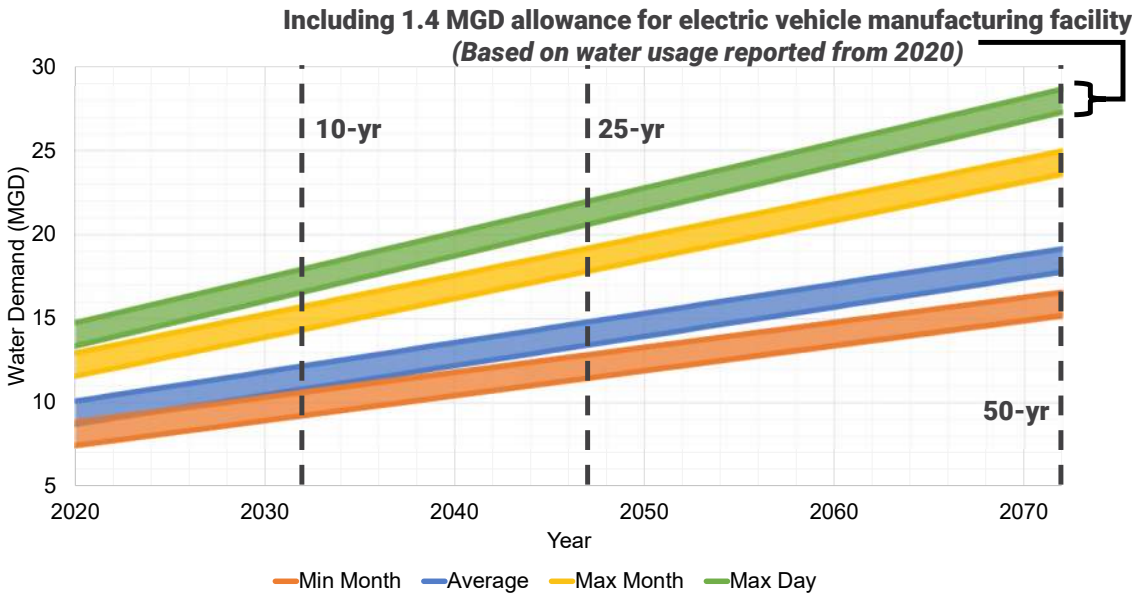
## Route Study Workshop – July 14, 2022

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# Raw Water Pipeline Route Study

July 14<sup>th</sup>, 2022

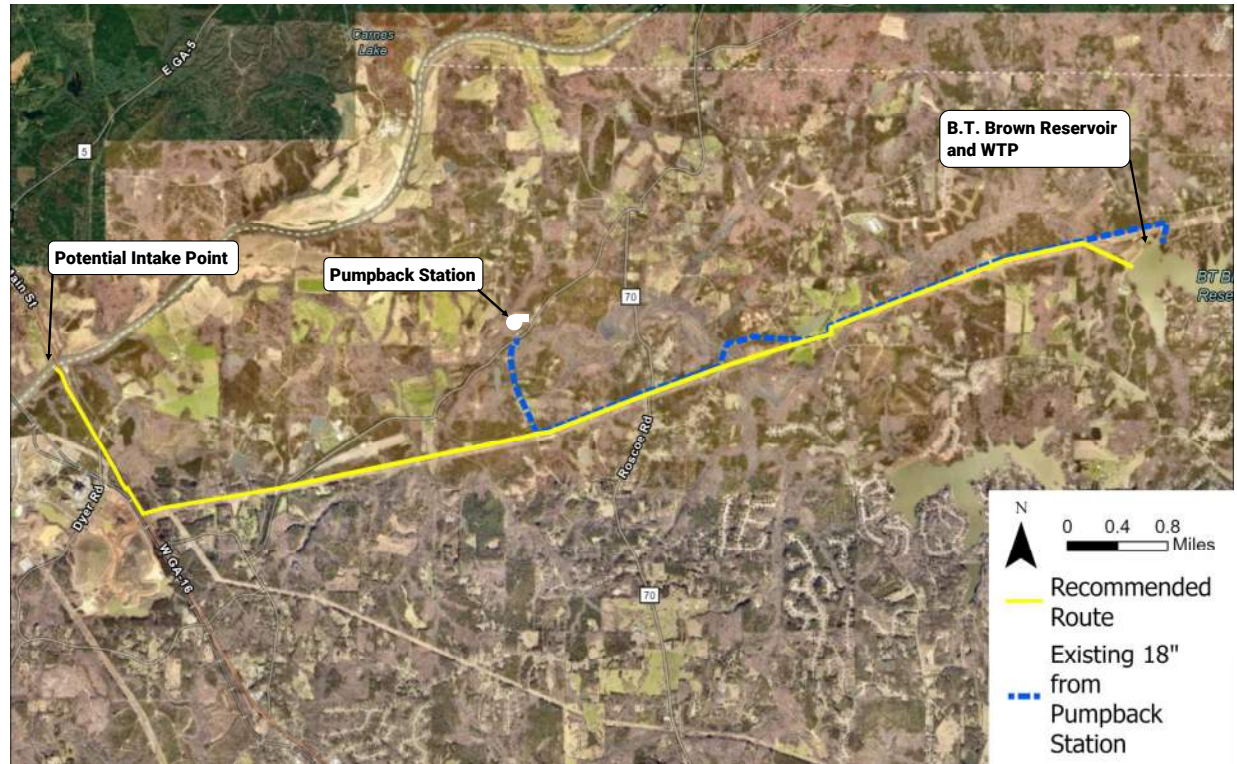
# Water Demand Summary



Demand Condition	10-yr (2032)	25-yr (2047)	50-yr (2072)
Min Month	10.6	12.8	16.6
Average	12.2	14.8	19.2
Max Month	15.7	19.2	25.0
Max Day	18.0	22.0	28.7

# System Overview

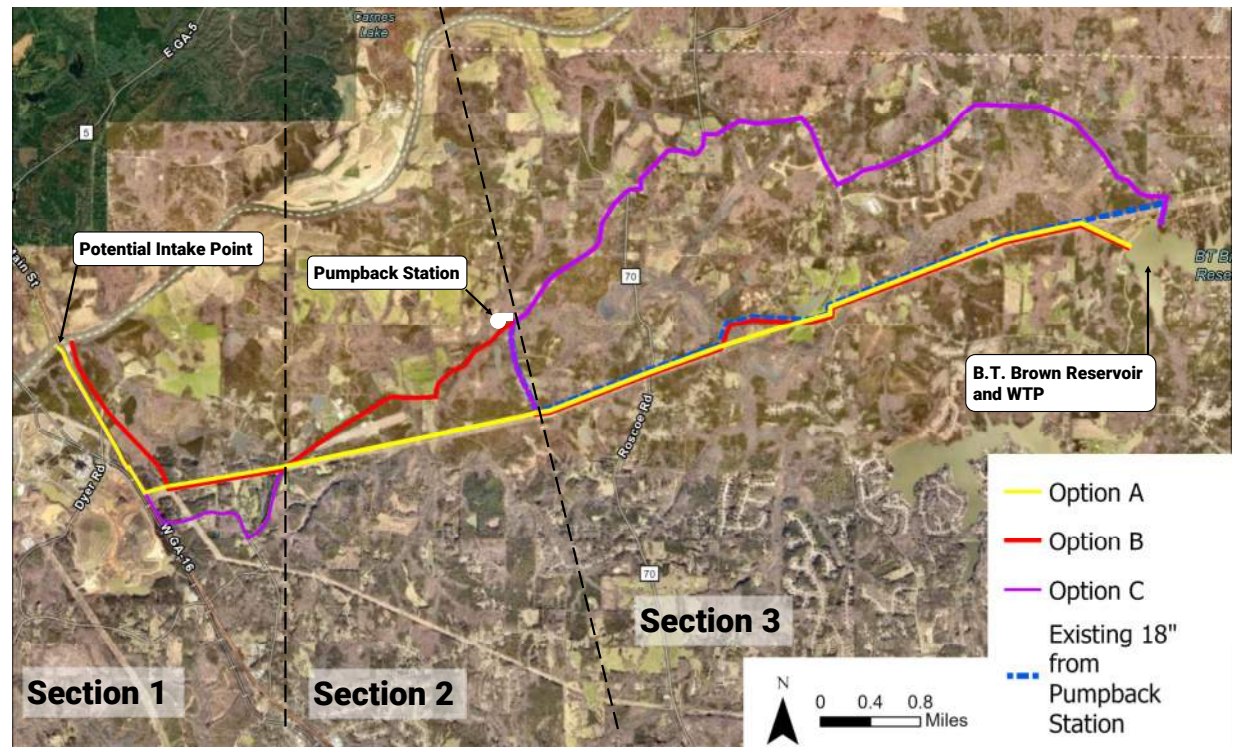
- 18" raw water line from pumpback station to B.T. Brown Reservoir
- Recommended route from potential Chattahoochee River intake to B.T. Brown Reservoir





# Route Options

- 3 sections with 3 options each
- **Option A** is Garver's recommended route
  - Correspondence with Georgia Power on Section 1
  - Shortest, most direct route

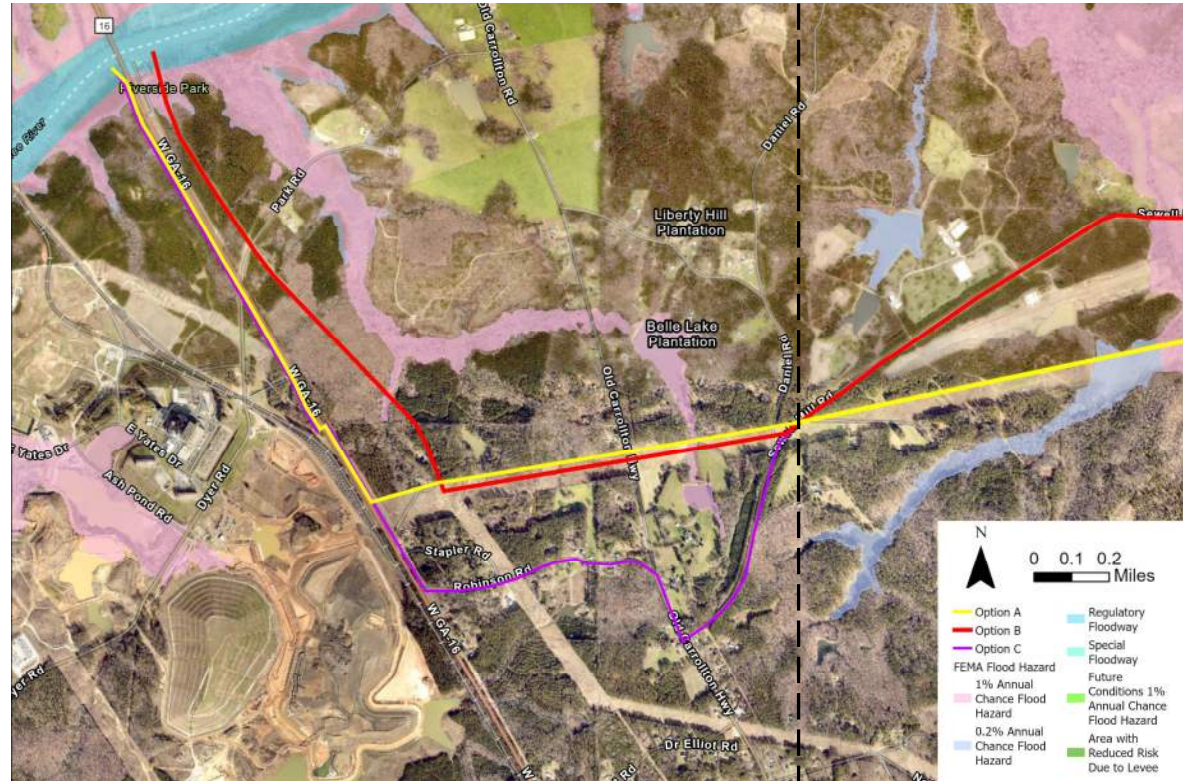


- 
- This aerial map illustrates three proposed road alignment options (Option A, Option B, and Option C) for a project in the Liberty Hill area. The map shows the following features:
- Option A (Yellow):** A route starting from the top left, passing through Liberty Hill Plantation, and ending near Belle Lake Plantation.
  - Option B (Red):** A route starting from the top left, passing through Liberty Hill Plantation, and ending near Belle Lake Plantation.
  - Option C (Purple):** A route starting from the bottom left, passing through Liberty Hill Plantation, and ending near Belle Lake Plantation.
  - Geographic Labels:** Liberty Hill Plantation, Belle Lake Plantation, Wickett Rd, Old Carriage Rd, Old Carriage Ln, and Belle Lake Rd.
  - Scale and Orientation:** A scale bar indicates 0 to 0.2 miles, and a north arrow is present.



# Section 1 Floodplains

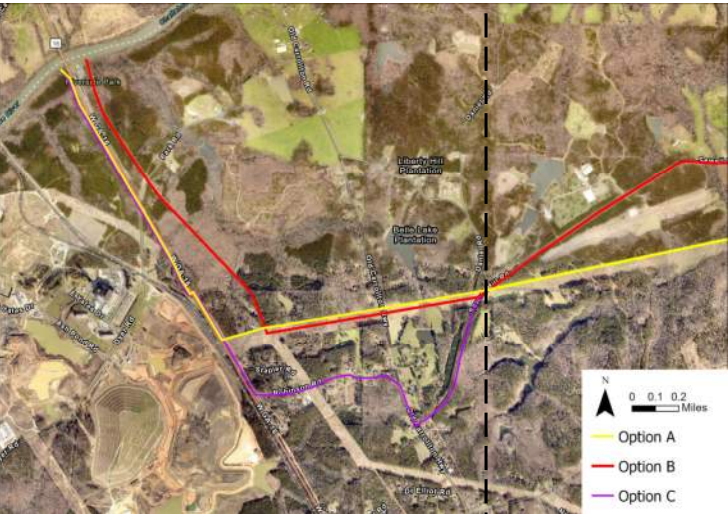
- Minimal floodplain in all three options



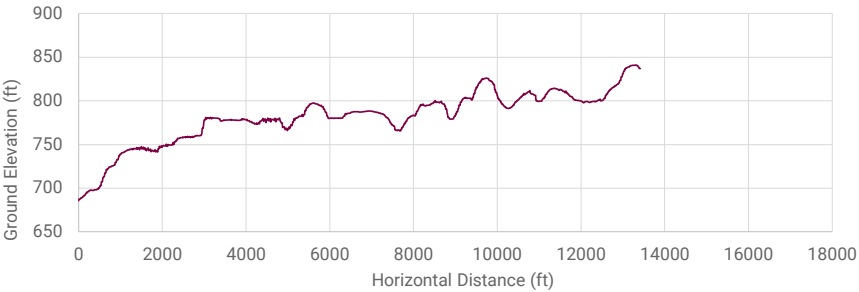
This aerial map illustrates the proposed road alignments for the Liberty Hill area. The map features three main options: Option A (yellow line), Option B (red line), and Option C (purple line). Option A runs from the top left, through the center, and towards the bottom right. Option B follows a similar path but is more direct in some sections. Option C is a more winding route, primarily located in the lower-left and lower-center portions of the map. The map also identifies wetlands and streams in light blue, and various local roads including Old Carrollton Rd, Old Carrollton Hwy, and several private drives. Two white circles highlight specific points of interest or potential concerns along the alignments. A scale bar and north arrow are provided in the bottom right corner for reference.



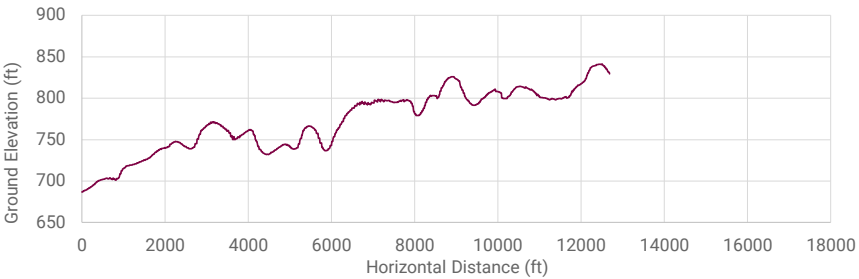
# Section 1 Profiles



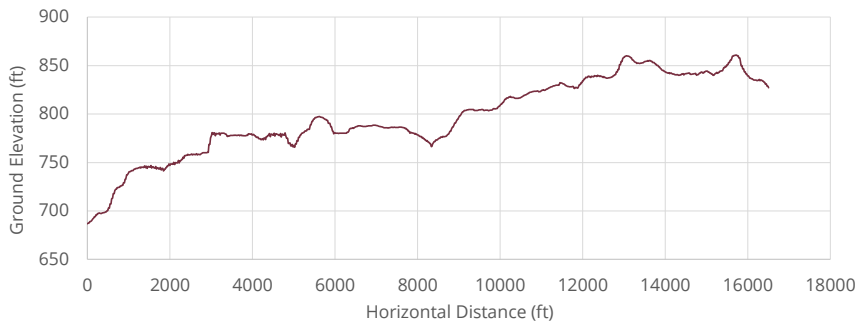
Option A



Option B



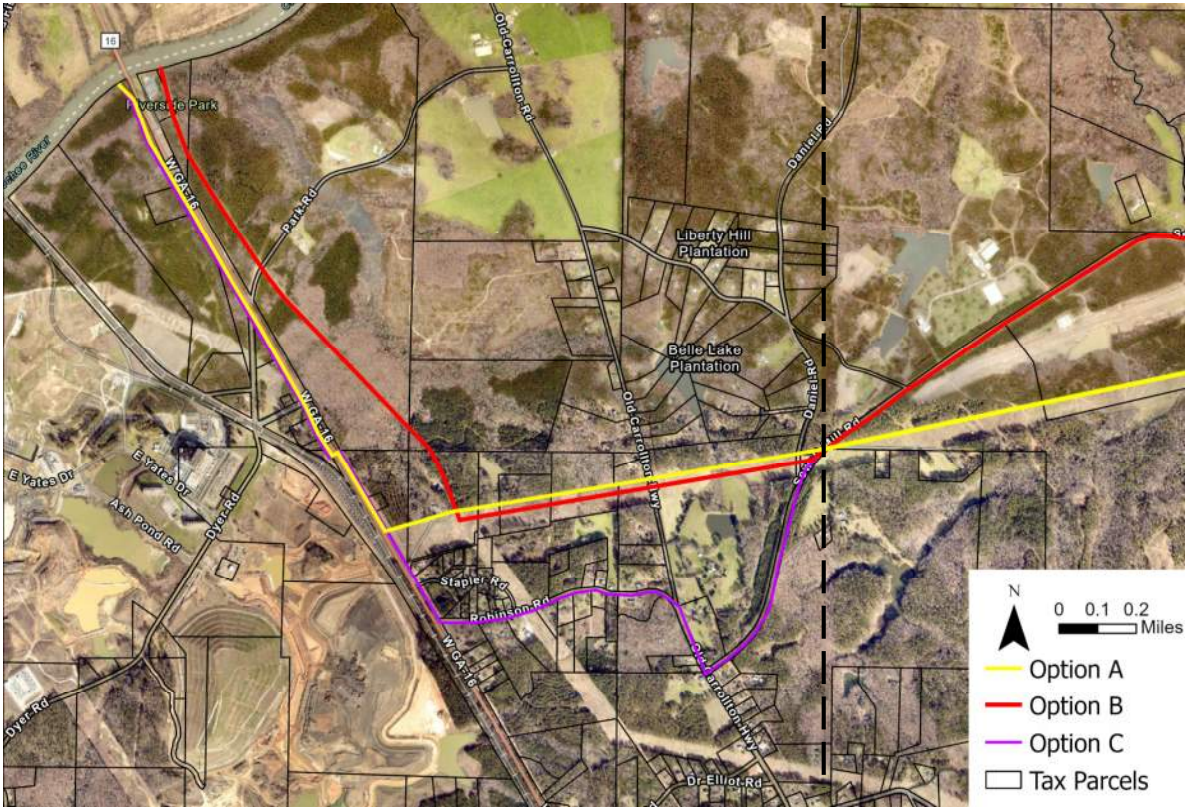
Option C



# Section 1 Cost and Easements

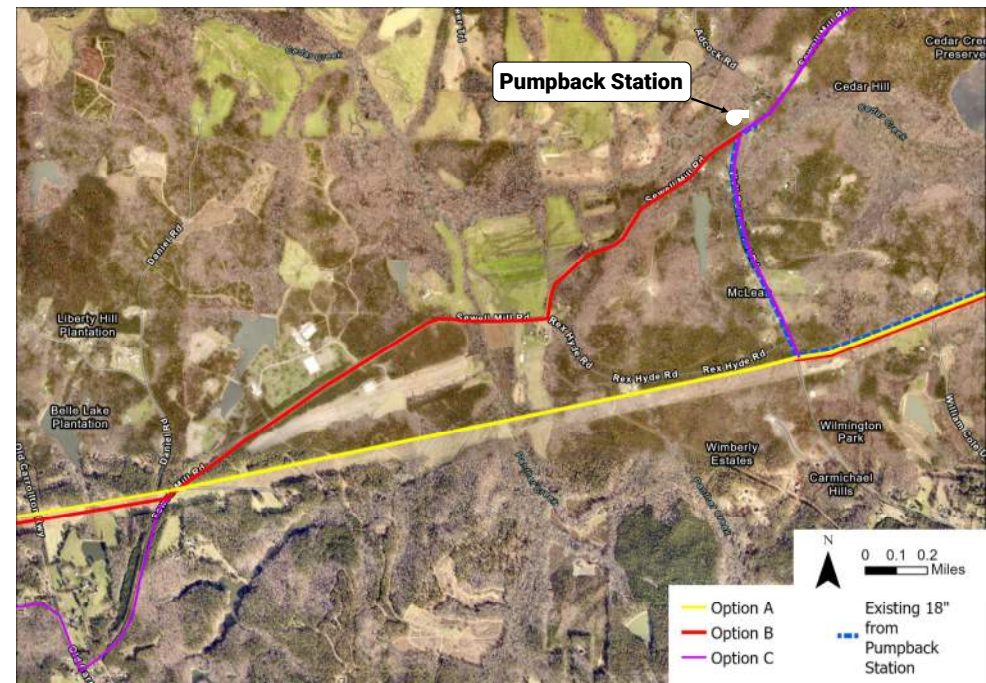
	Length (ft)	30" Cost Estimate*	36" Cost Estimate*	42" Cost Estimate*	Number of Easements
Option A	13,416	\$6,439,680	\$7,727,616	\$9,015,552	9
Option B	12,683	\$6,087,840	\$7,305,408	\$8,522,976	11
Option C	16,508	\$7,923,840	\$9,508,608	\$11,093,376	22

\*Cost estimate developed with a \$16.00/inch per linear foot factor  
\*Easement cost is not included in cost estimate



## Section 2

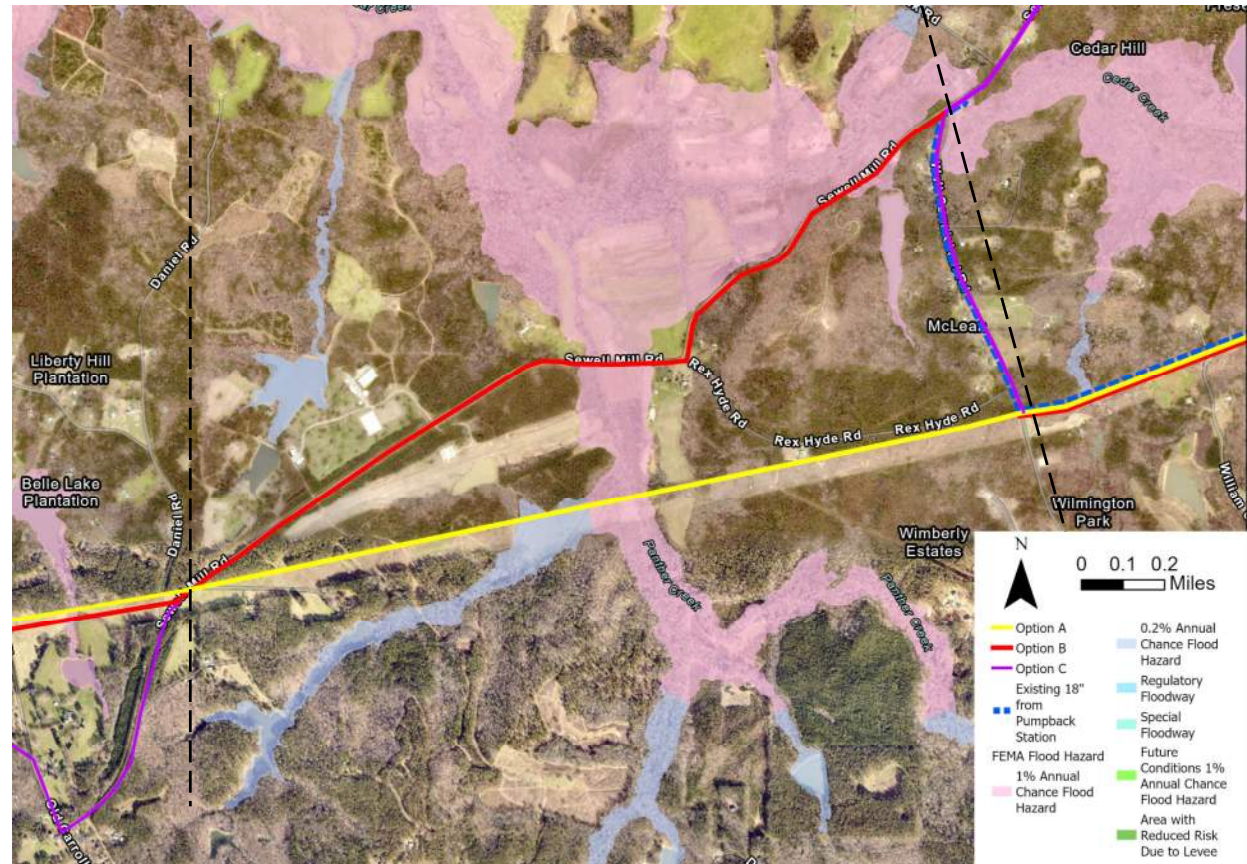
- Option A follows Georgia Power right-of-way
- Option B follows Sewell Mill Road
  - Option B' parallels the existing 18" from the pumpback station along Walt Carmichael Road





## Section 2 Floodplains

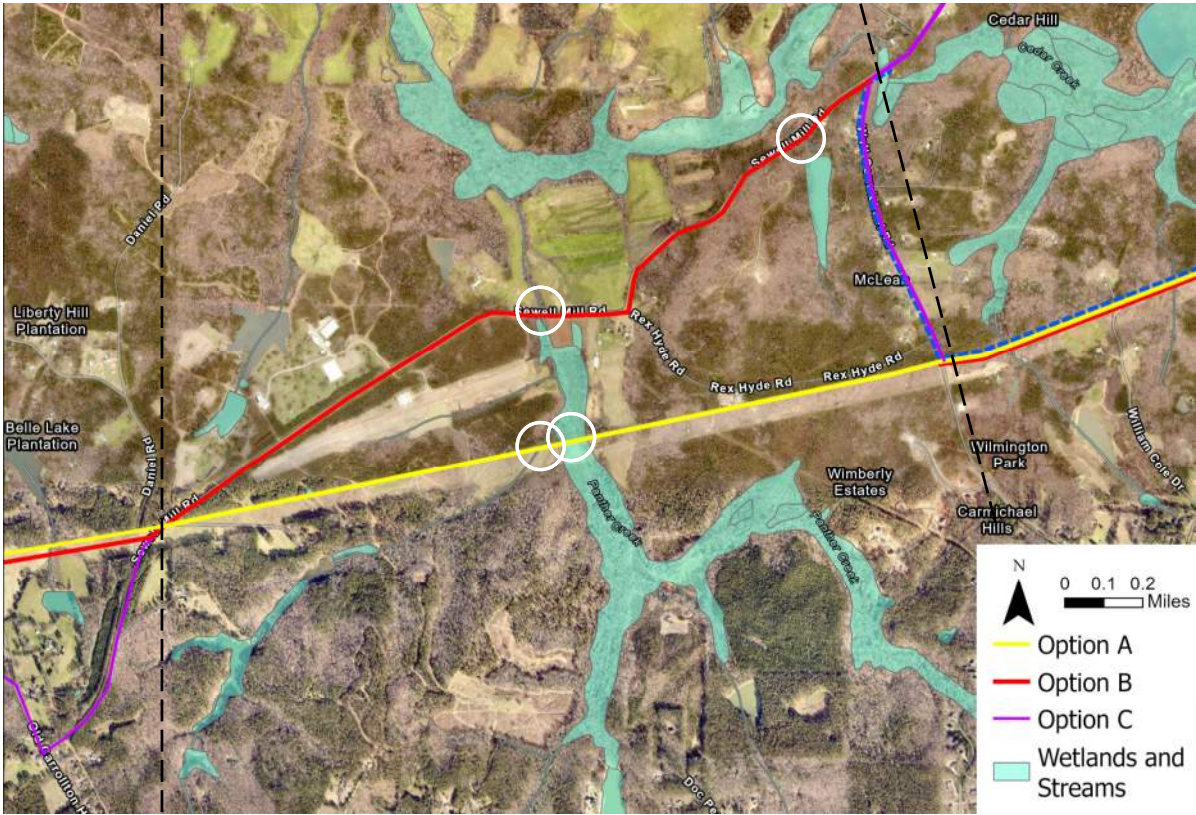
- **Option B** crosses more 100-year floodplain



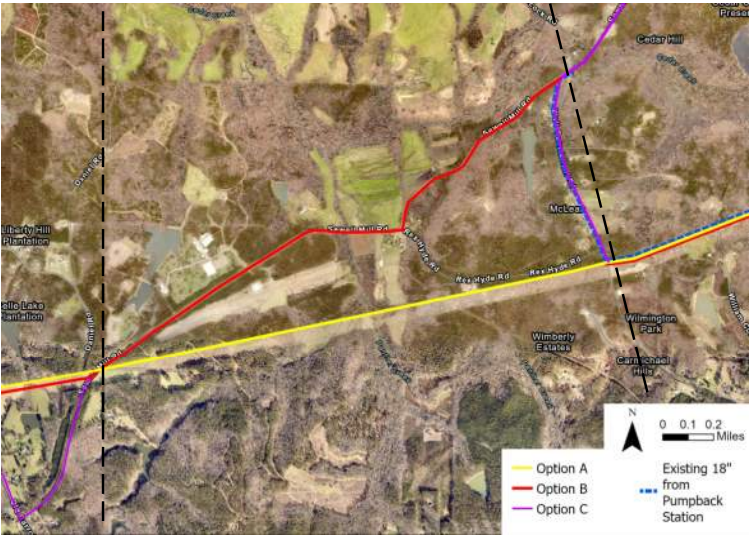


# Section 2 Wetlands and Streams

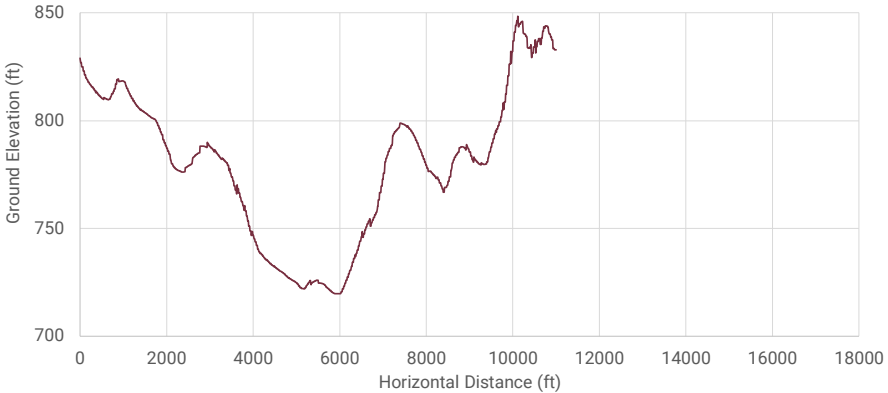
	Stream Crossings	Wetland Crossings
Option A	2	1
Option B	2	1
Option B'	0	0



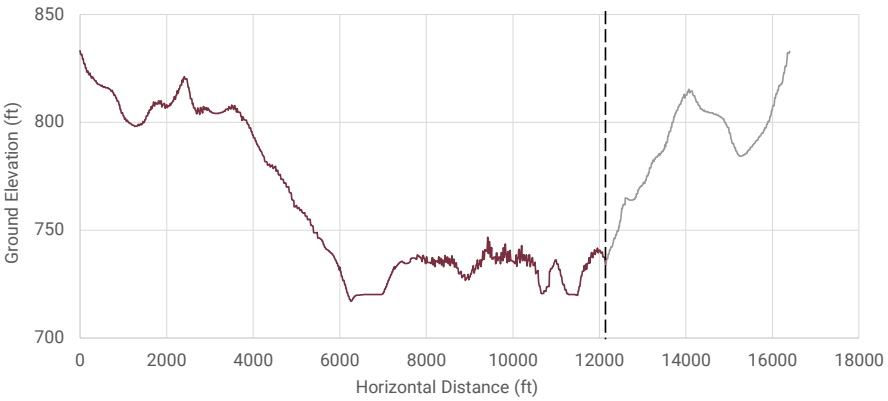
# Section 2 Profiles



Option A



Option B



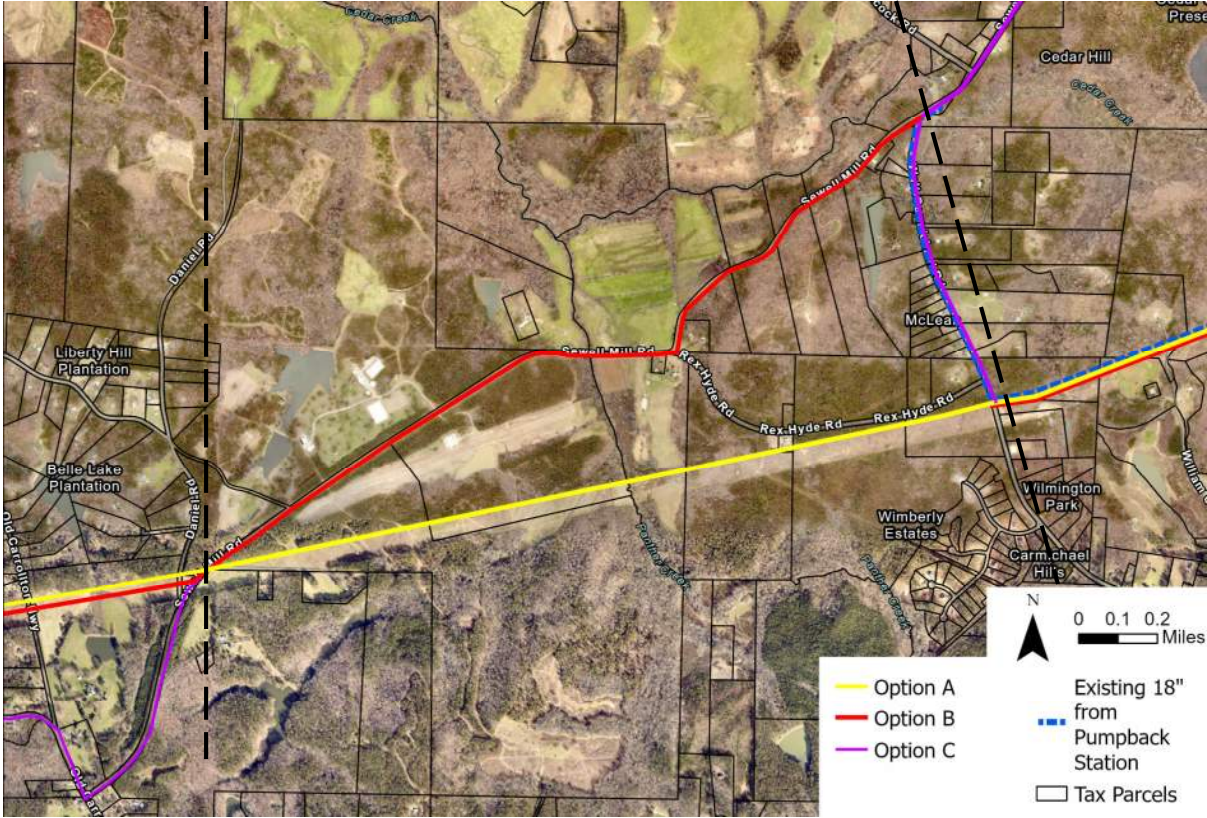
Option B'



# Section 2 Cost and Easements

	Length (ft)	30" Cost Estimate*	36" Cost Estimate*	42" Cost Estimate*	Number of Easements
Option A	11,178	\$5,365,440	\$6,438,528	\$7,511,616	7
Option B	12,146	\$5,830,080	\$6,996,096	\$8,162,112	11
Option B'	4,248	\$2,039,040	\$2,446,848	\$2,854,656	17

\*Cost estimate developed with a \$16.00/inch per linear foot factor  
\*Easement cost is not included in cost estimate

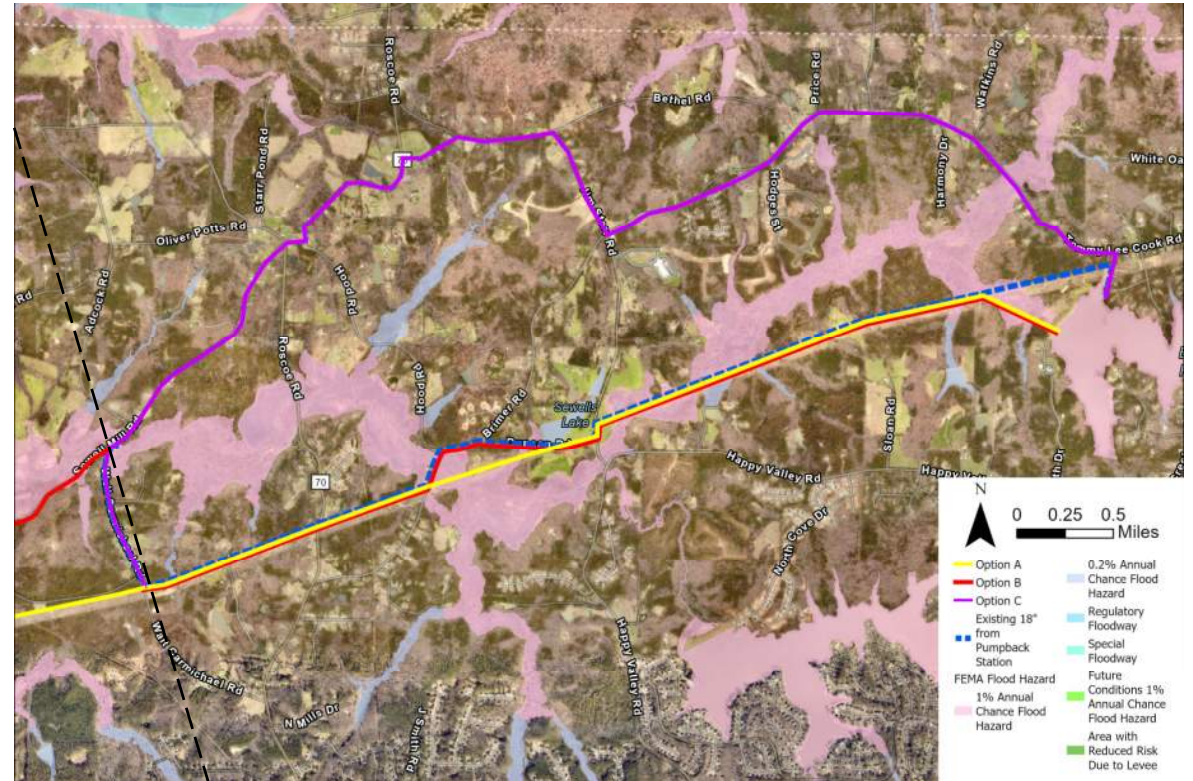


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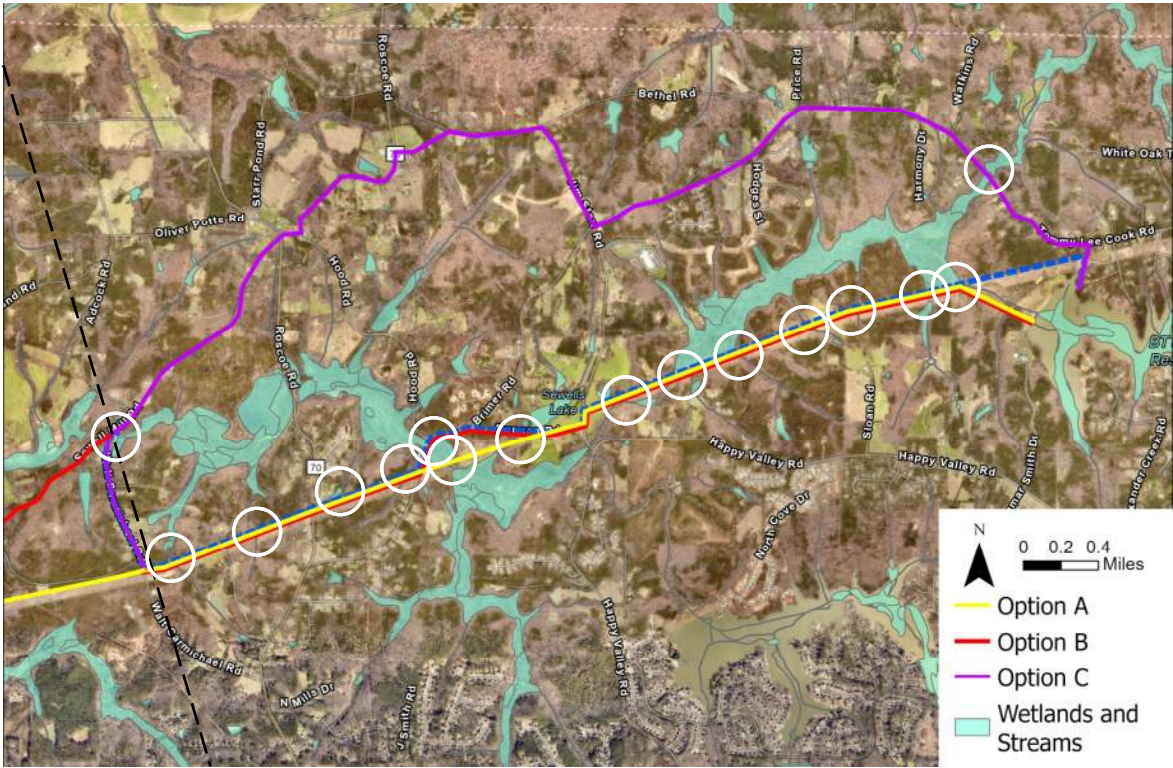
## Section 3 Floodplains

- Options **A** and **B** cross more 100-year floodplain



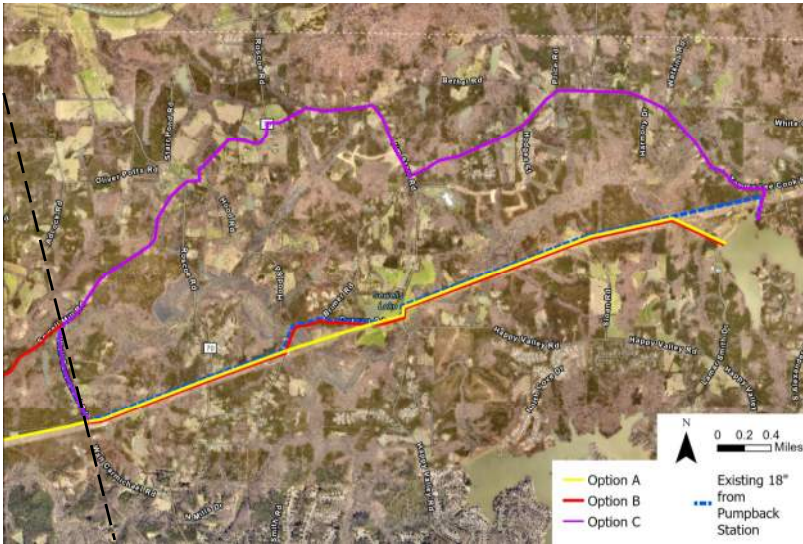
# Section 3 Wetlands and Streams

	Stream Crossings	Wetland Crossings
Option A	11	2
Option B	11	3
Option C	2	2

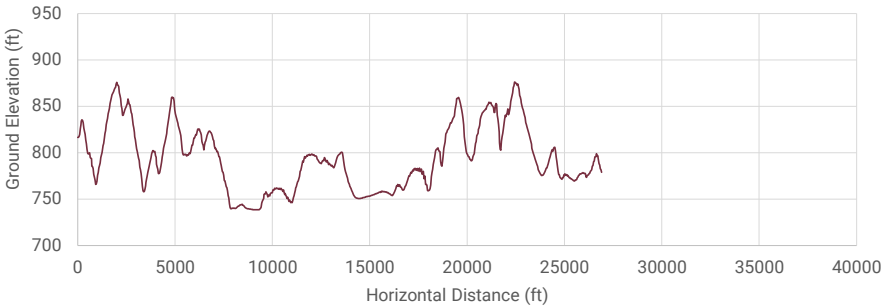




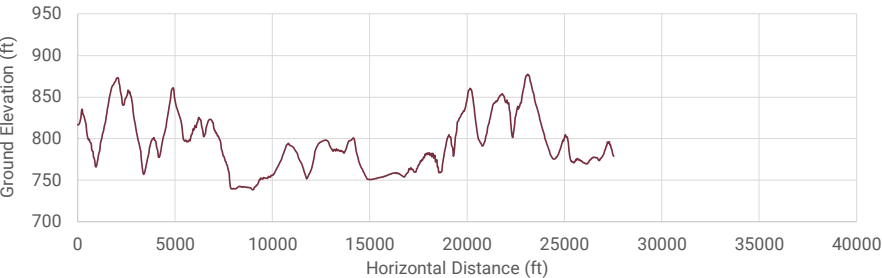
# Section 3 Profiles



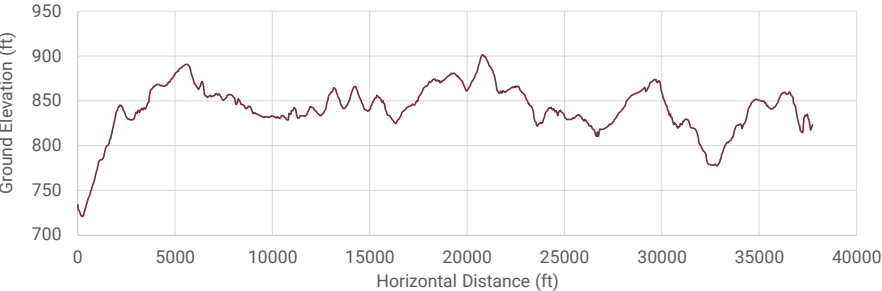
Option A



Option B



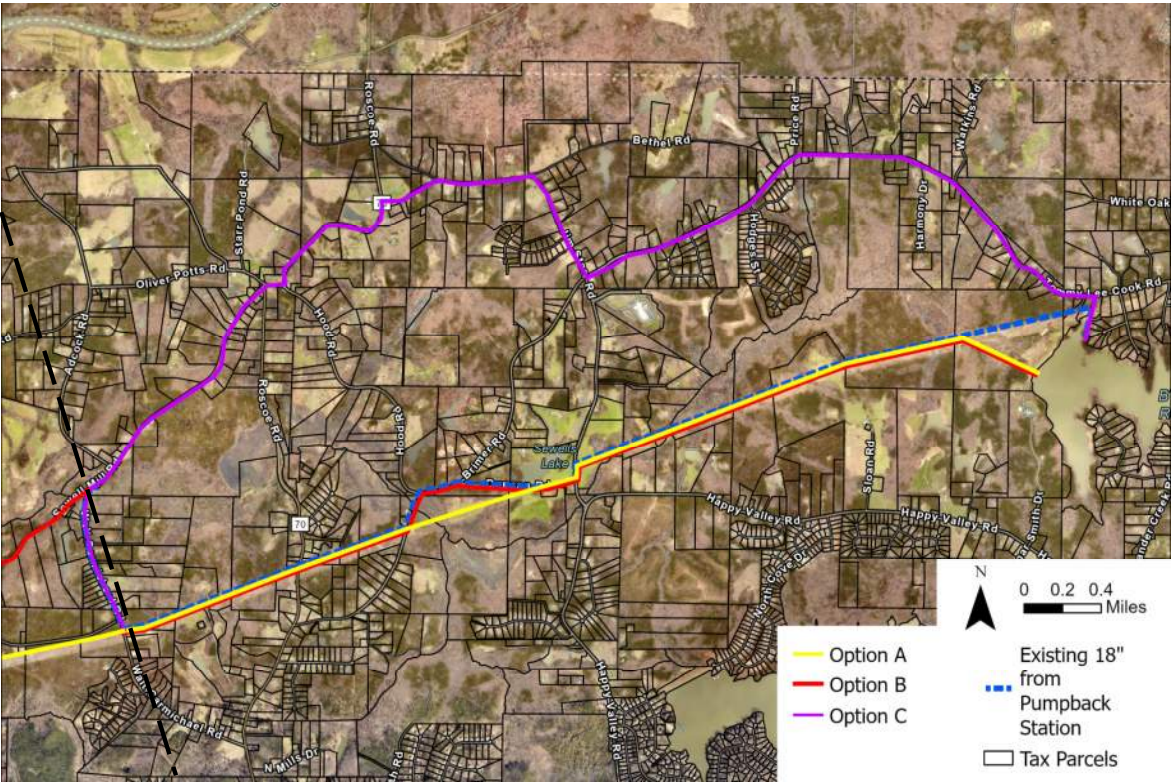
Option C



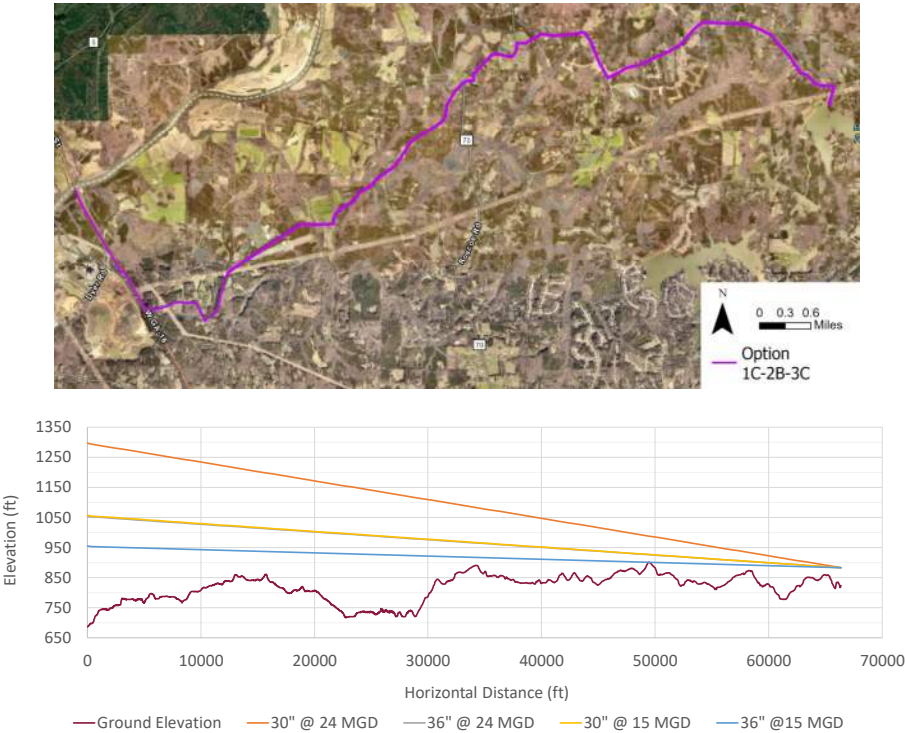
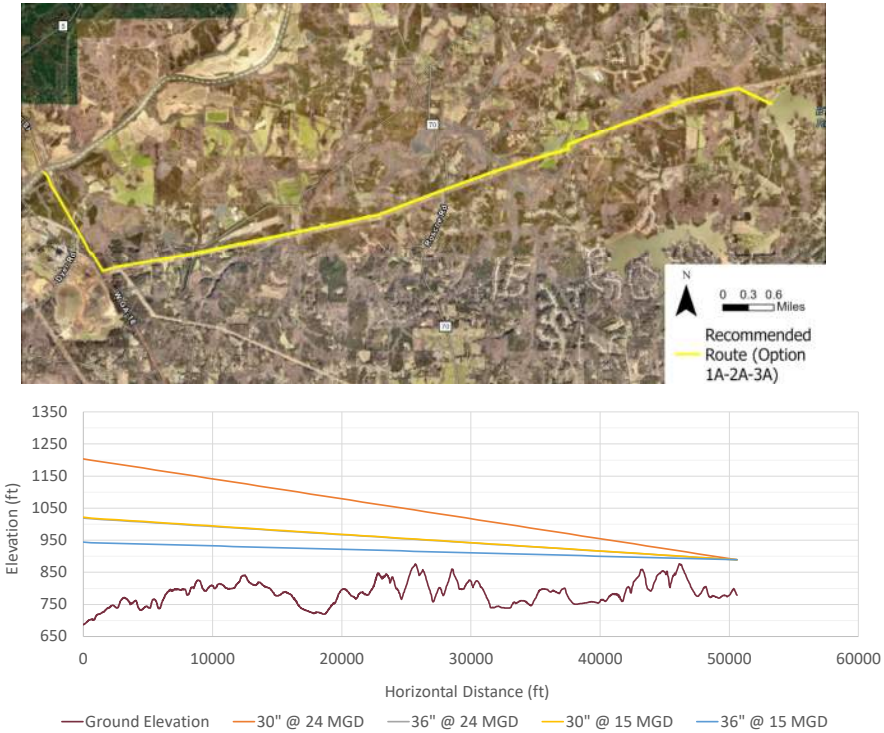
# Section 3 Cost and Easements

	Length (ft)	30" Cost Estimate*	36" Cost Estimate*	42" Cost Estimate*	Number of Easements
Option A	26,908	\$12,915,840	\$15,499,008	\$18,082,176	23
Option B	27,525	\$13,212,000	\$15,854,400	\$18,496,800	20
Option C	37,655	\$18,074,400	\$21,689,280	\$25,304,160	84

\*Cost estimate developed with a \$16.00/inch per linear foot factor  
\*Easement cost is not included in cost estimate

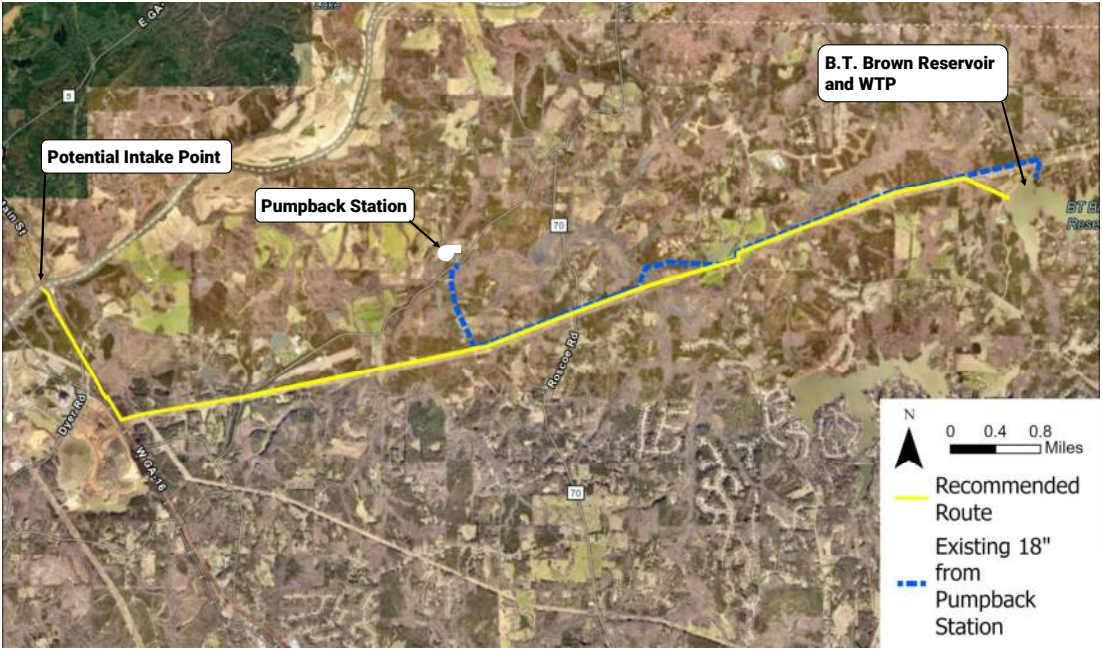


# Route Hydraulic Profiles





# Summary and Recommendations



Recommended Route Summary						
Length (ft)	30" Cost Estimate*	36" Cost Estimate*	42" Cost Estimate*	Number of Easements	Stream Crossings	Wetland Crossings
51,502	\$24,720,960	\$29,665,152	\$34,609,344	39	3	14
*Cost estimate developed with a \$16.00/inch per linear foot factor						
*Easement cost is not included in cost estimate						

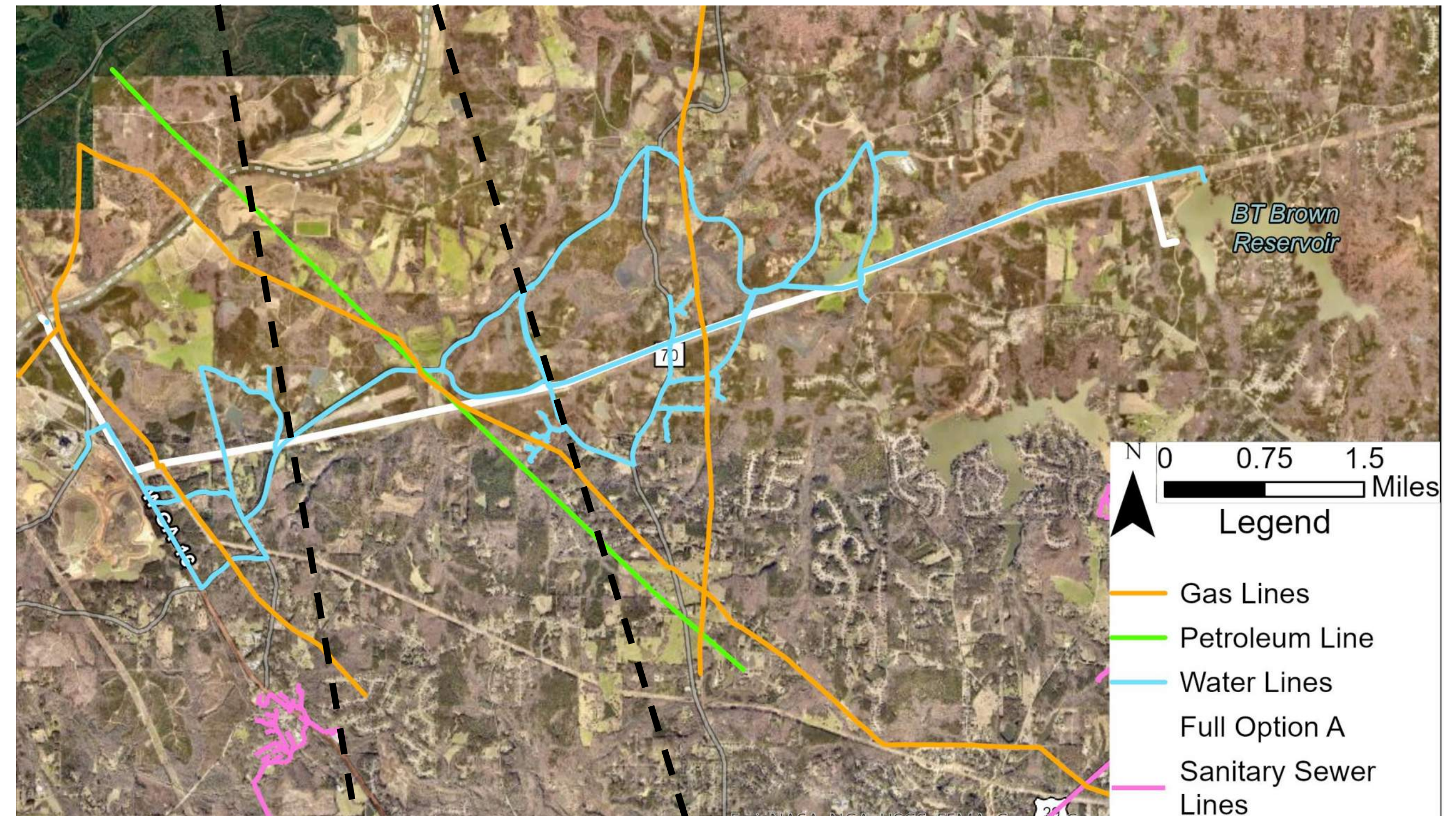
# APPENDIX C

## Route Study Update – August 16, 2022



# Utility Conflicts

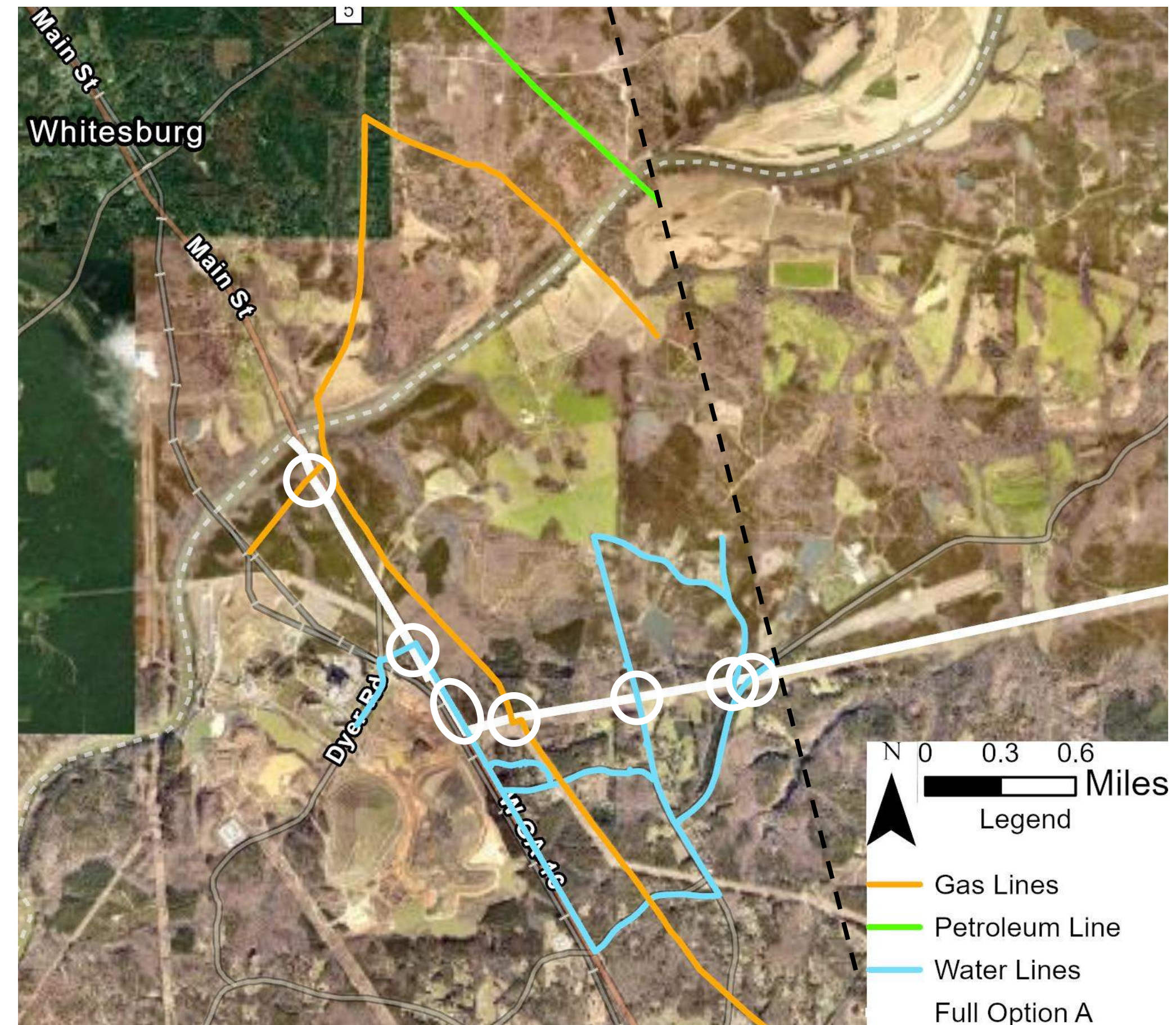
- Gas Lines
- Water Lines
- Petroleum Line (Kinder Morgan)
- Fiber, Cable and DSL Internet





# Section 1

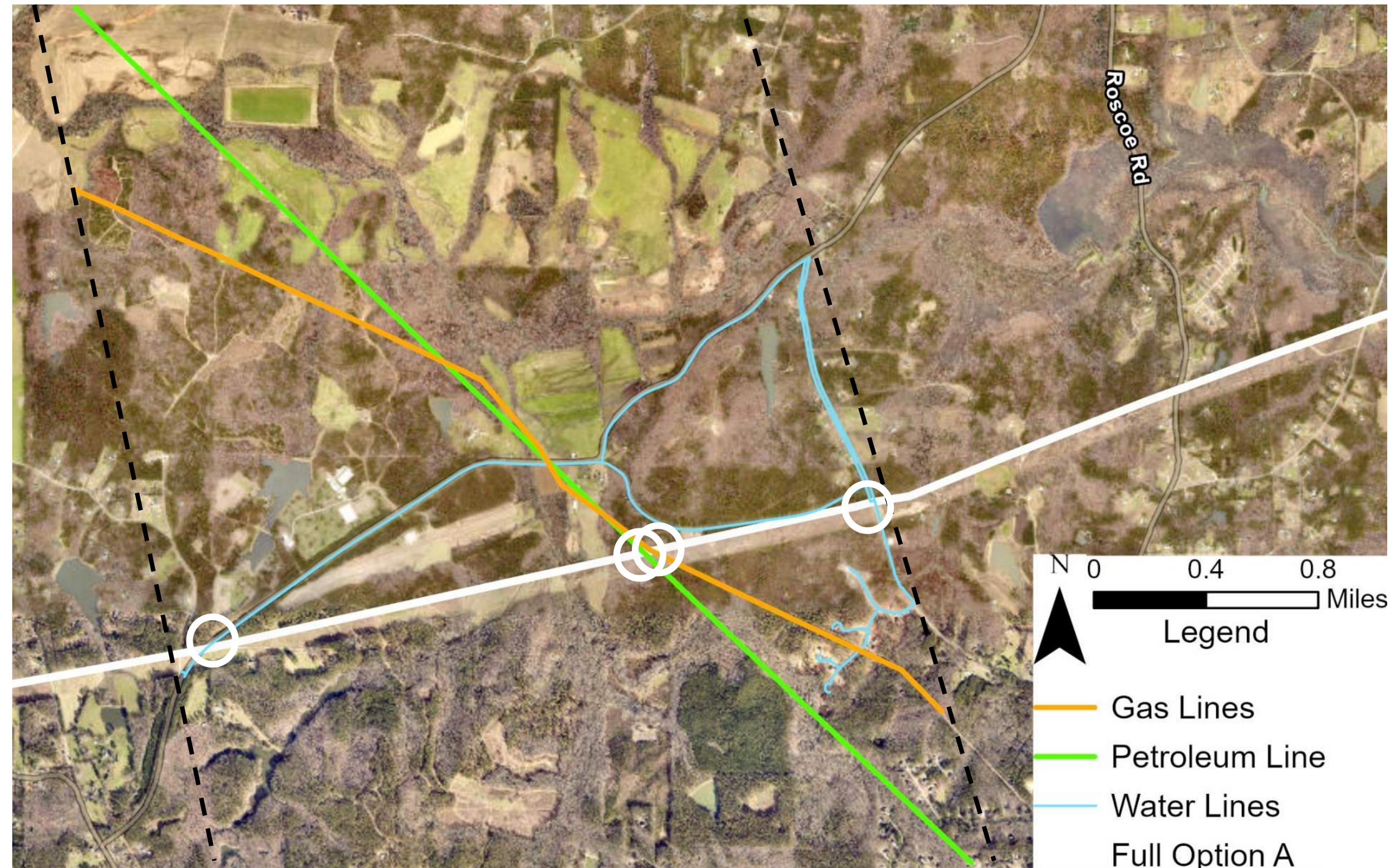
- 4 Water Line Crossings
- Runs parallel to the water line along Route 16
- 2 Gas Line Crossings





## Section 2

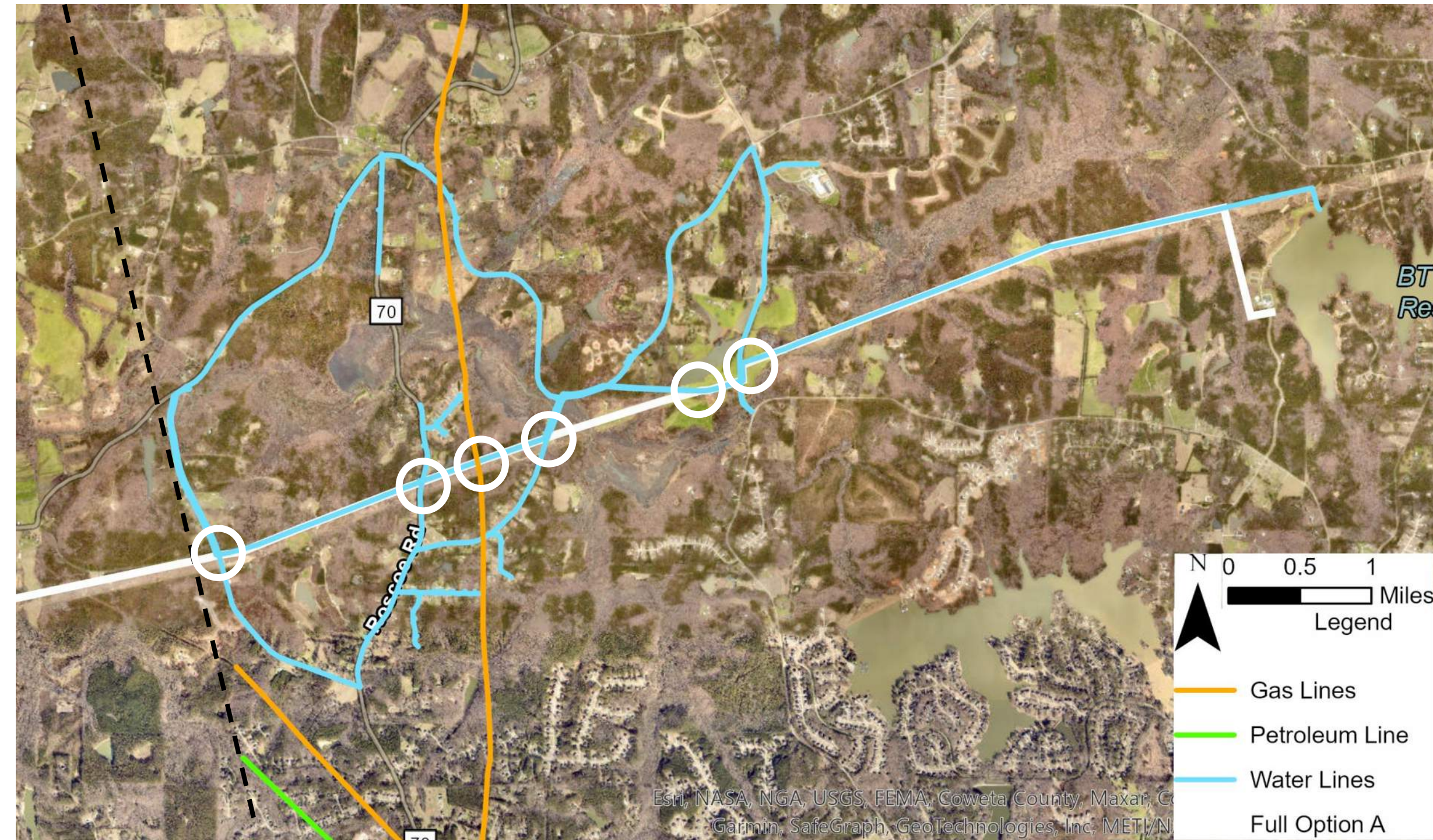
- 2 Water Line crossings
  - Runs parallel to the water line
- 1 Gas Line crossing
- 1 Petroleum Line Crossing





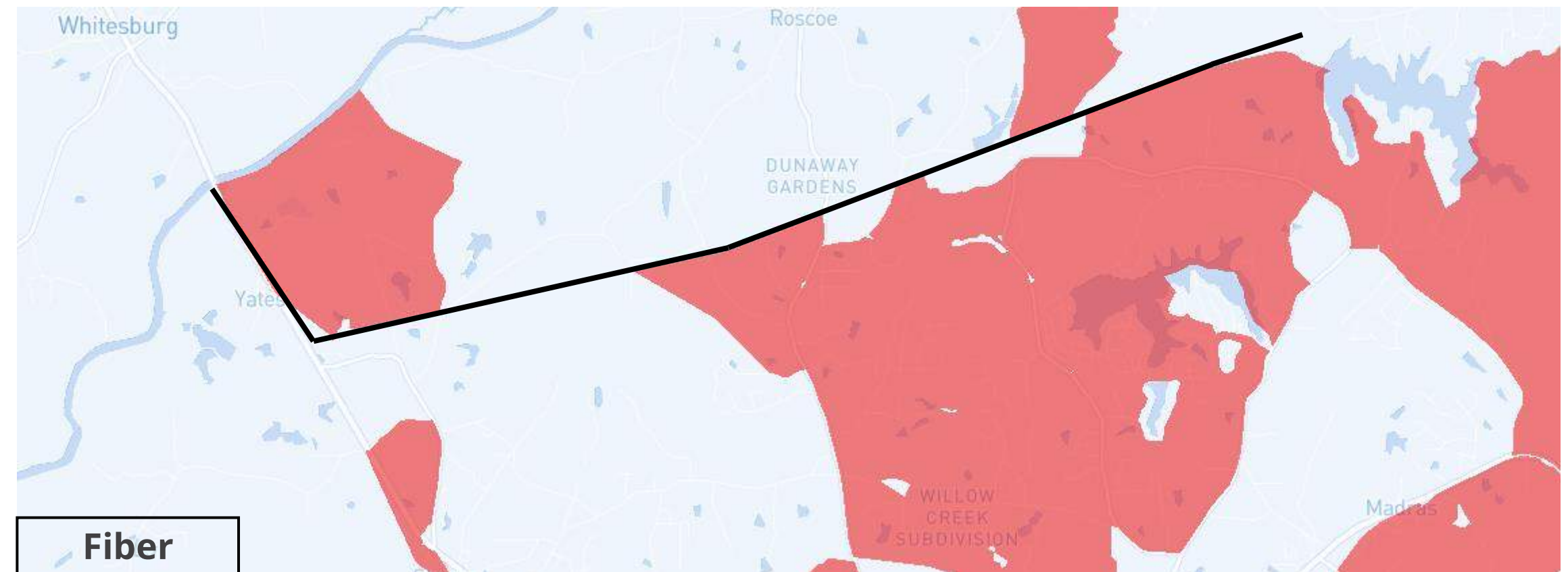
# Section 3

- 5 Water Line Crossings
- Runs parallel to the water line for most of the section
- 1 Gas Line Crossing



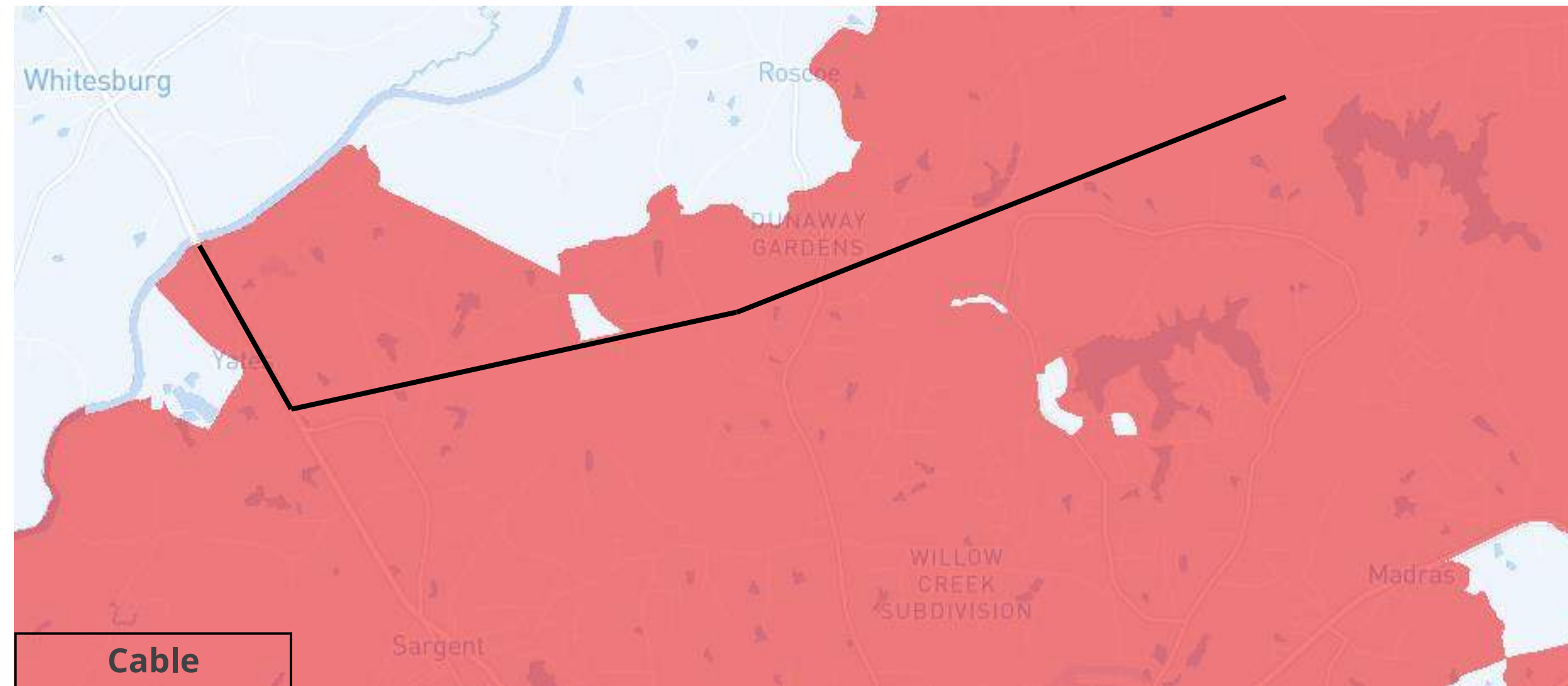


# AT&T Coverage



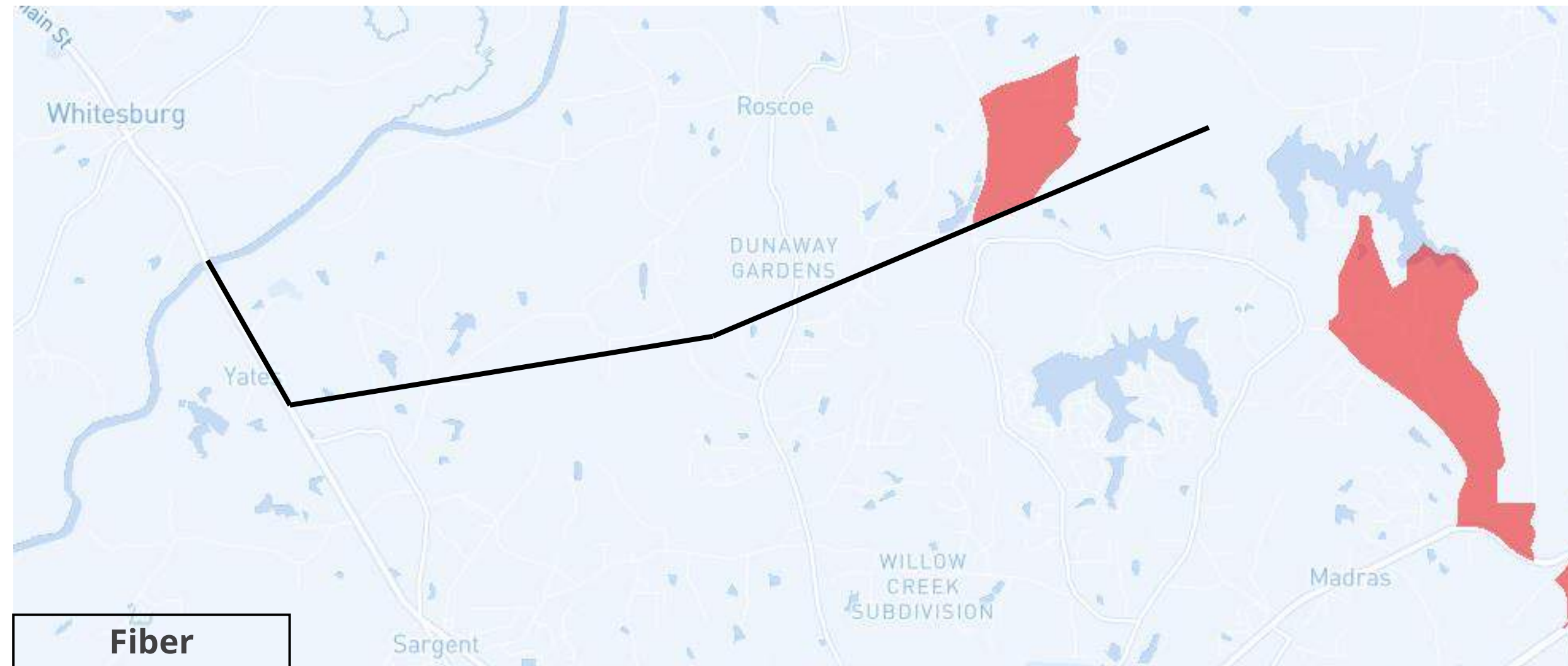
# Spectrum Coverage

---



# WOW! Coverage

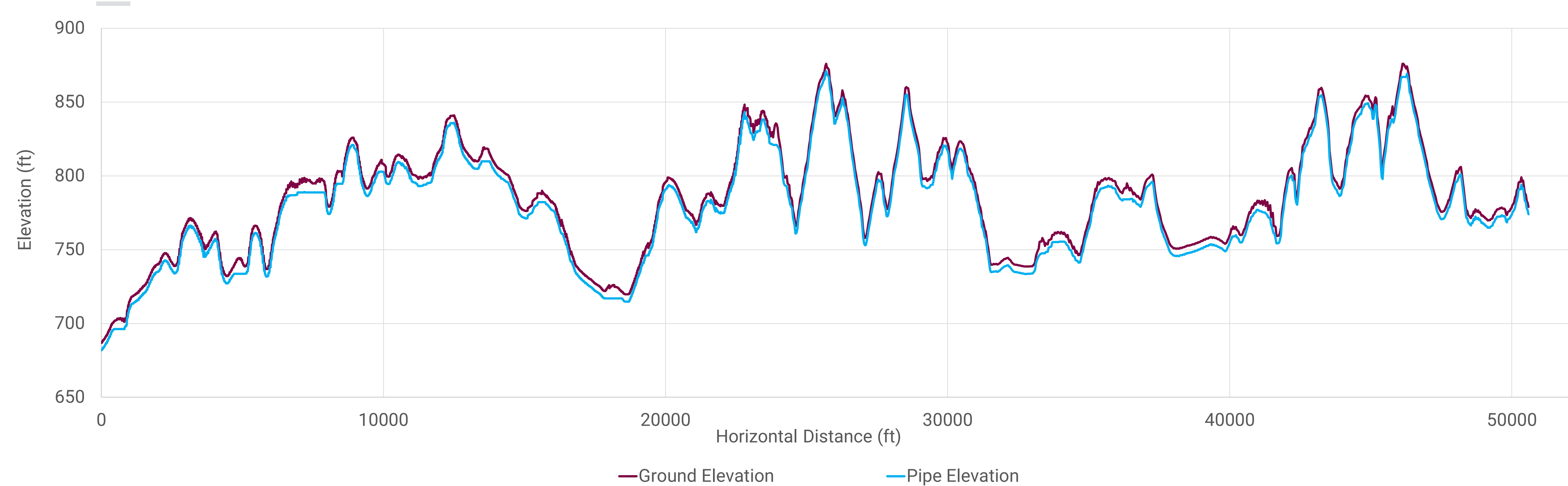
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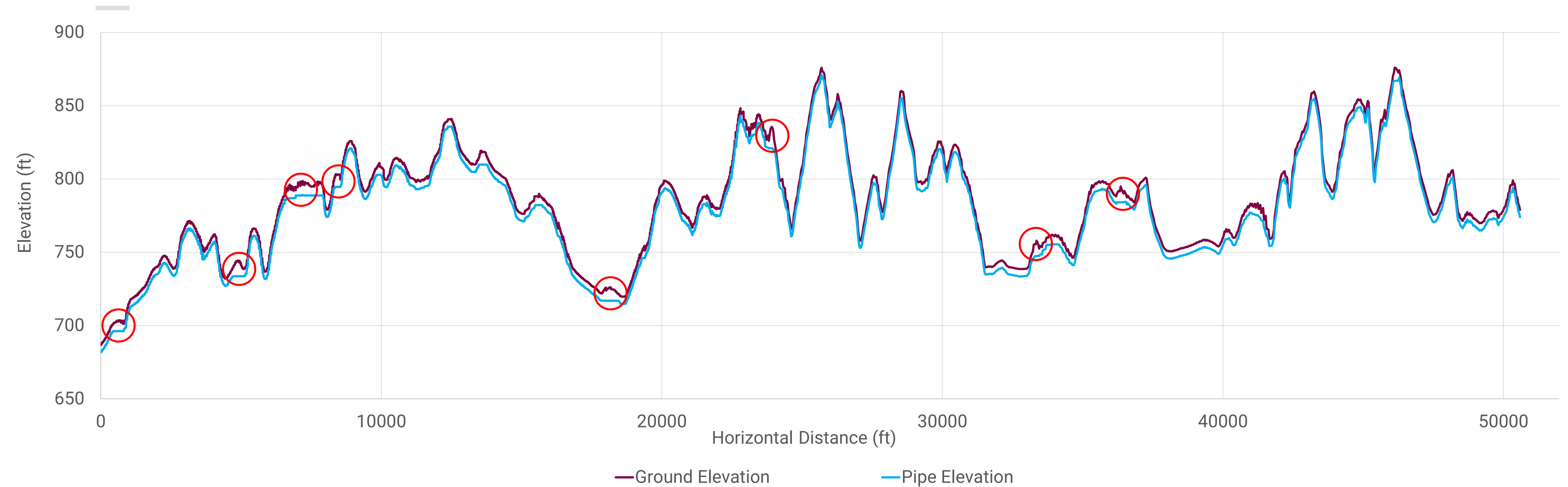
# Pipe Profile

Depth of Cover	
Minimum (feet)	Maximum (feet)
5.0	14.5



# Air Release Valves Minimized

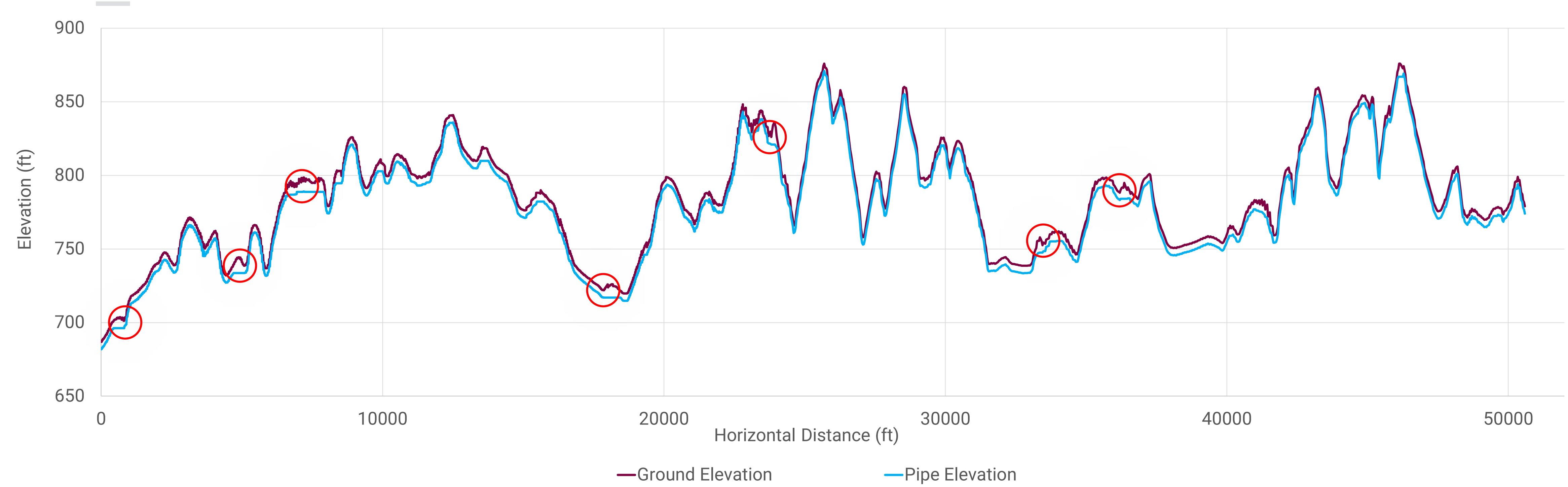
Air Release Valves Required	
Standard 5' Cover	With Engineering Design
44	35



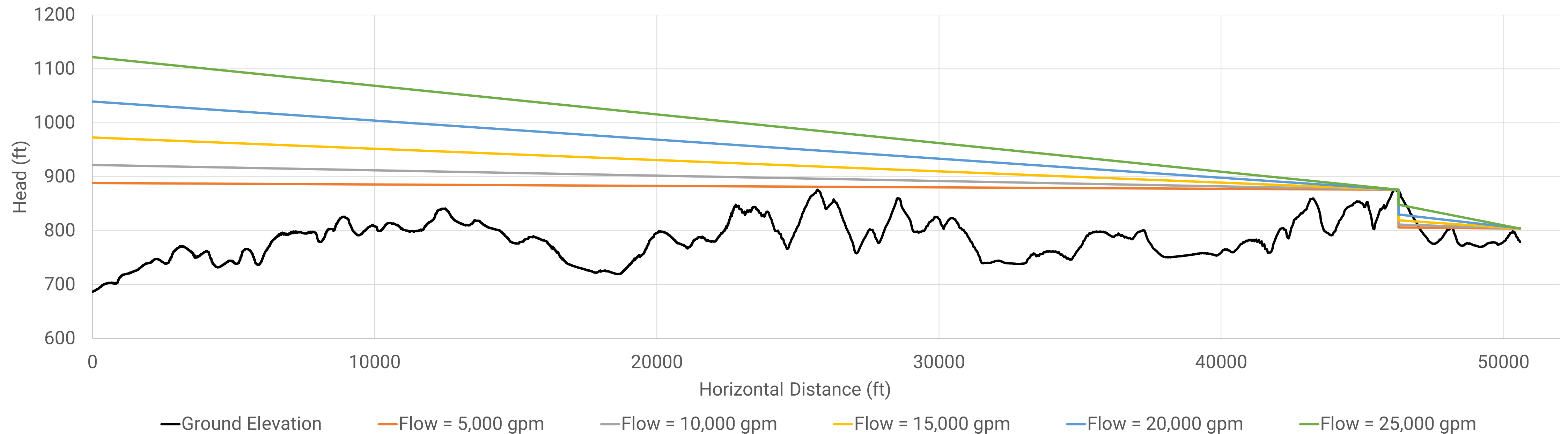


# Blow Off Valves Minimized

Blow Off Valves Required	
Standard 5' Cover	With Engineering Design
31	24

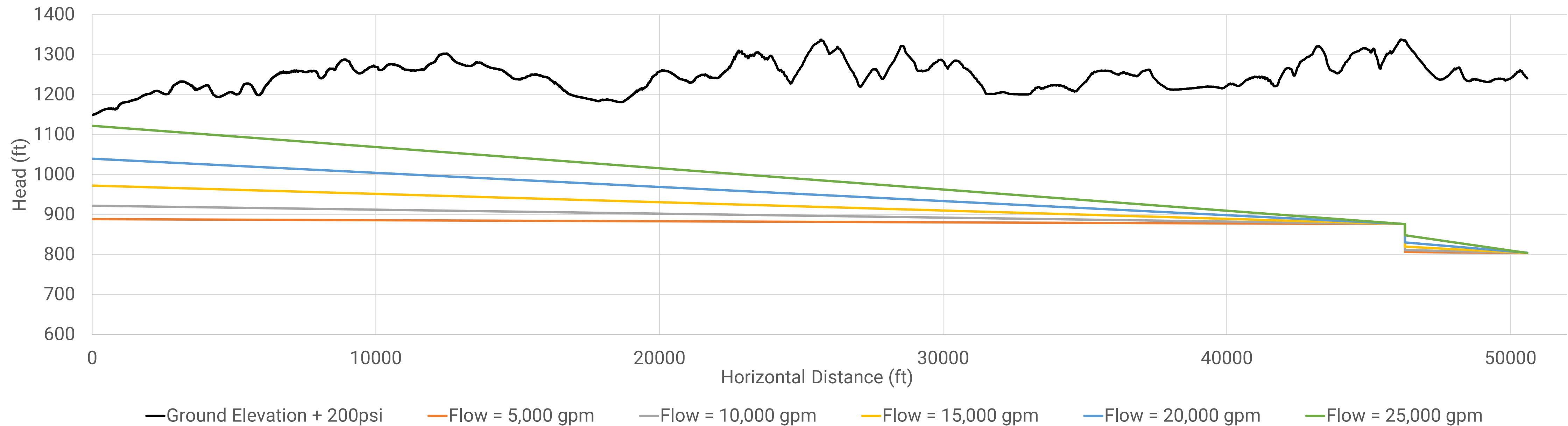


# 36" Pipe Hydraulic Profiles





# Maximum Pressure < 200 PSI



# Next Steps

---

- Site walk of proposed route
  - Wednesday, August 16<sup>th</sup>
- Route study technical memo
  - Monday, September 12<sup>th</sup>





## **Appendix B**

# **Opinion of Probable Construction Cost Itemized Breakdown**

# Chattahoochee Raw Water Pump Station

Coweta County Water and Sewerage Authority

22W38085

## 1. Lump Sum Work (Facilities or Areas)

\$ Total

[05](#) Pump Station and Intake

\$18,406,000

Subtotal Estimated Lump Sum Work

\$18,406,000

## 2. Line Items

\$ Total

Subtotal for Line Items

\$0

## 3. Cash Allowances for Products & Services

\$ Total

Work Change Directives

3%

\$553,000

Independent Testing Cash Allowance

1%

\$185,000

Subtotal for Cash Allowance Items

\$738,000

## 4. Project Escalation

\$ Total

Escalation of Cost to Midpoint Construction

3

Years @

5.%

\$3,018,000

## 5. Deductive Alternates

\$ Total

Subtotal Estimated Deductive Alternatives

\$0

Escalation of Cost to Midpoint Construction

3

Years @

5.%

\$0

**Complete Project**

\$19,144,000

**Complete Project with Escalation**

\$22,162,000



**Chattahoochee Raw Water Pump Station**  
**Coweta County Water and Sewerage Authority**  
**22W38085**

**Facility Number: 5**  
**Facility Description: Pump Station and Intake**

Description	Qty	Unit	Unit Cost	Labor	Total Cost
<b>Division 1 - General Requirements</b>					
Dewatering	1	LS	\$ 50,000.00	\$ 15,000.00	\$ 65,000
Coffer Dam	1	LS	\$ 50,000.00	\$ 15,000.00	\$ 65,000
Miscellaneous	5%				\$ 6,500
Subtotal Division 1					\$ 136,500
<b>Division 3 - Concrete</b>					
<b>Pump Station - Wet Well</b>					
Base Slab	238	CY	\$ 800.00	\$ 240.00	\$ 248,012
Wall	720	CY	\$ 1,600.00	\$ 480.00	\$ 1,497,363
Pump Bay Dividing Walls	71	CY	\$ 1,600.00	\$ 480.00	\$ 147,542
Elevated Slab	81	CY	\$ 2,400.00	\$ 720.00	\$ 253,600
<b>Pump Station - Pump Building</b>					
Base Slab	113	CY	\$ 800.00	\$ 240.00	\$ 117,789
Wall	19	CY	\$ 1,600.00	\$ 480.00	\$ 39,985
					\$ -
<b>Pump Station - Electrical Building</b>					
Base Slab	72	CY	\$ 800.00	\$ 240.00	\$ 75,260
Wall	84	CY	\$ 1,600.00	\$ 480.00	\$ 173,783
<b>Intake Structure</b>					
Base Slab	21	CY	\$ 800.00	\$ 240.00	\$ 22,017
Wall	80	CY	\$ 1,600.00	\$ 480.00	\$ 165,640
Miscellaneous	5%				\$ 137,050
Subtotal Division 3					\$ 2,878,041
<b>Division 4 - Masonry</b>					
CMU	4406	SF	\$ 35.00	\$ 10.50	\$ 200,488
Brick Veneer	4406	SF	\$ 40.00	\$ 12.00	\$ 229,129
Miscellaneous	5%				\$ 21,481
Subtotal Division 4					\$ 451,097
<b>Division 5 - Metals</b>					
Grating (Intake Bar Screens)	403	SF	\$ 50.00	\$ 15.00	\$ 26,183
Roof Joists	765	LF	\$ 400.00	\$ 120.00	\$ 397,800
Miscellaneous	5%				\$ 21,199
Subtotal Division 5					\$ 445,182
<b>Division 6 - Wood &amp; Plastics</b>					
Miscellaneous	5%				\$ -
Subtotal Division 6					\$ -
<b>Division 7 - Thermal/Moisture Protection</b>					
Membrane Roofing	3570	SF	\$ 8.00	\$ 2.40	\$ 37,124
Insulation	3570	SF	\$ 2.00	\$ 0.60	\$ 9,281
Miscellaneous	5%				\$ 2,320
Subtotal Division 7					\$ 48,726
<b>Division 8 - Openings</b>					
Double Door	3	EA	\$ 800.00	\$ 240.00	\$ 3,120
Single Door	1	EA	\$ 500.00	\$ 150.00	\$ 650
14' x 14' Rollup Door	1	EA	\$ 2,200.00	\$ 660.00	\$ 2,860
Aluminum hatch	1	EA	\$ 3,000.00	\$ 900.00	\$ 3,900
Miscellaneous	5%				\$ 527
Subtotal Division 8					\$ 11,057
<b>Division 9 - Finishes</b>					

Building Finishes	4406 SF	\$	2.00		\$	8,812	
Pipe Coating	2% of Pipe Cost			\$	4,616.32	\$	28,577
Miscellaneous	5%					\$	1,869
	Subtotal Division 9					\$	39,258
Division 10 - Specialties							
Building Allowance	3570 SF	\$	200.00		\$	713,928	
Miscellaneous	5%				\$	35,696	
	Subtotal Division 10					\$	749,624
Division 22 - Plumbing							
Building Plumbing Allowance	1 LS	\$	30,000.00		\$	30,000	
Pump Seal Water Connection (8" PVC)	2500 LF	\$	100.00	\$	30.00	\$	325,000
Miscellaneous	5%				\$	17,750	
	Subtotal Division 22					\$	372,750
Division 23 - HVAC							
Building HVAC Allowance	1 LS	\$	150,000.00		\$	150,000	
Miscellaneous	5%				\$	7,500	
	Subtotal Division 23					\$	157,500
Division 26 - Electrical							
400 HP VFD	4 EA	\$	100,000.00	\$	30,000.00	\$	520,000
Main 2500A Switchboard	1 EA	\$	250,000.00	\$	75,000.00	\$	325,000
Motor Control Center	1 EA	\$	20,000.00	\$	6,000.00	\$	26,000
Building Electrical (Lights, Recepts, Grounding, Etc.)	2500 SF	\$	10.00	\$	10.00	\$	50,000
Pump 1 Conduit & Wire	50 LF	\$	170.00	\$	102.00	\$	13,600
Pump 2 Conduit & Wire	50 LF	\$	170.00	\$	102.00	\$	13,600
Pump 3 Conduit & Wire	50 LF	\$	170.00	\$	102.00	\$	13,600
Pump 4 Conduit & Wire	50 LF	\$	170.00	\$	102.00	\$	13,600
Misc. Conduit and Wire	1 LS	\$	15,000.00	\$	15,000.00	\$	30,000
Service Conduit & Wire	75 LF	\$	510.00	\$	306.00	\$	61,200
						\$	-
Control Panel	1 EA	\$	75,000.00	\$	22,500.00	\$	97,500
SCADA Integration	1 LS			\$	100,000.00	\$	100,000
Georgia Power Utility Service Allowance	1 LS	\$	300,000.00			\$	300,000
Miscellaneous	5%				\$	78,205	
	Subtotal Division 26					\$	1,642,305
Division 31 - Earthwork							
Excavation	20000 CY	\$	20.00	\$	6.00	\$	520,000
Shoring	6000 SF	\$	80.00	\$	24.00	\$	624,000
Backfill	17500 CY	\$	30.00	\$	9.00	\$	682,500
Granular Fill	1000 CY	\$	60.00	\$	18.00	\$	78,000
Miscellaneous	5%				\$	95,225	
	Subtotal Division 31					\$	1,999,725
Division 32 - Exterior Improvements							
Gravel Drive	15323 SF	\$	1.80	\$	0.54	\$	35,856
Fencing	503 LF	\$	15.00	\$	4.50	\$	9,802
25.5' Fence Gates	2 EA	\$	1,500.00	\$	450.00	\$	3,900
Miscellaneous	5%				\$	2,478	
	Subtotal Division 32					\$	52,036
Division 40 - Process Integration							
Yard Piping							
36" DIP Connection to Transmission Main	34 LF	\$	379.64	\$	113.89	\$	16,928
30" Intake Pipe (DIP)	550 LF	\$	290.00	\$	87.00	\$	207,350
Station Piping							
14" Pipe (DIP)	45 LF	\$	1,215.00	\$	364.50	\$	70,888
14" Check Valve	4 EA	\$	4,000.00	\$	1,200.00	\$	20,800
14" Pump Control Valve	4 EA	\$	7,000.00	\$	2,100.00	\$	36,400
14" Gate Valve	4 EA	\$	3,000.00	\$	900.00	\$	15,600
36" X 20" Tee	4 EA	\$	10,000.00	\$	3,000.00	\$	52,000



14" x 20" Reducer	4 EA	\$	3,500.00	\$	1,050.00	\$	18,200
Air Release Valve	4 EA	\$	325.00	\$	97.50	\$	1,690
Air Pipe	4 EA	\$	150.00	\$	45.00	\$	780
Intake Stop Logs	1 EA	\$	93,000.00	\$	9,300.00	\$	102,300
30" x 30" x 35" Slide Gates	2 EA	\$	11,000.00	\$	3,300.00	\$	28,600
Miscellaneous	5%					\$	28,577
	Subtotal Division 40					\$	600,113
Division 43 - Process Gas & Liquid Handling							
VT Pumps	4 EA	\$	300,000.00	\$	90,000.00	\$	1,560,000
						\$	-
						\$	-
						\$	-
Miscellaneous	5%					\$	78,000
	Subtotal Division 43					\$	1,638,000
Division 46 - Water and Wastewater Equipment							
Intake Screens	2 EA	\$	101,000.00	\$	30,300.00	\$	262,600
Air Burst System	1 EA	\$	105,000.00	\$	31,500.00	\$	136,500
Field Service	1 EA	\$	-	\$	6,000.00	\$	6,000
						\$	-
Miscellaneous	5%					\$	20,255
	Subtotal Division 46					\$	425,355
Subtotal This Facility							
						\$	11,648,000
Contingency					30%	\$	3,495,000
Mobilization					3%	\$	455,000
Contractor's Overhead and Profit					18%	\$	2,808,000
Total Estimated Facility Costs						\$	18,406,000



# **Appendix C**

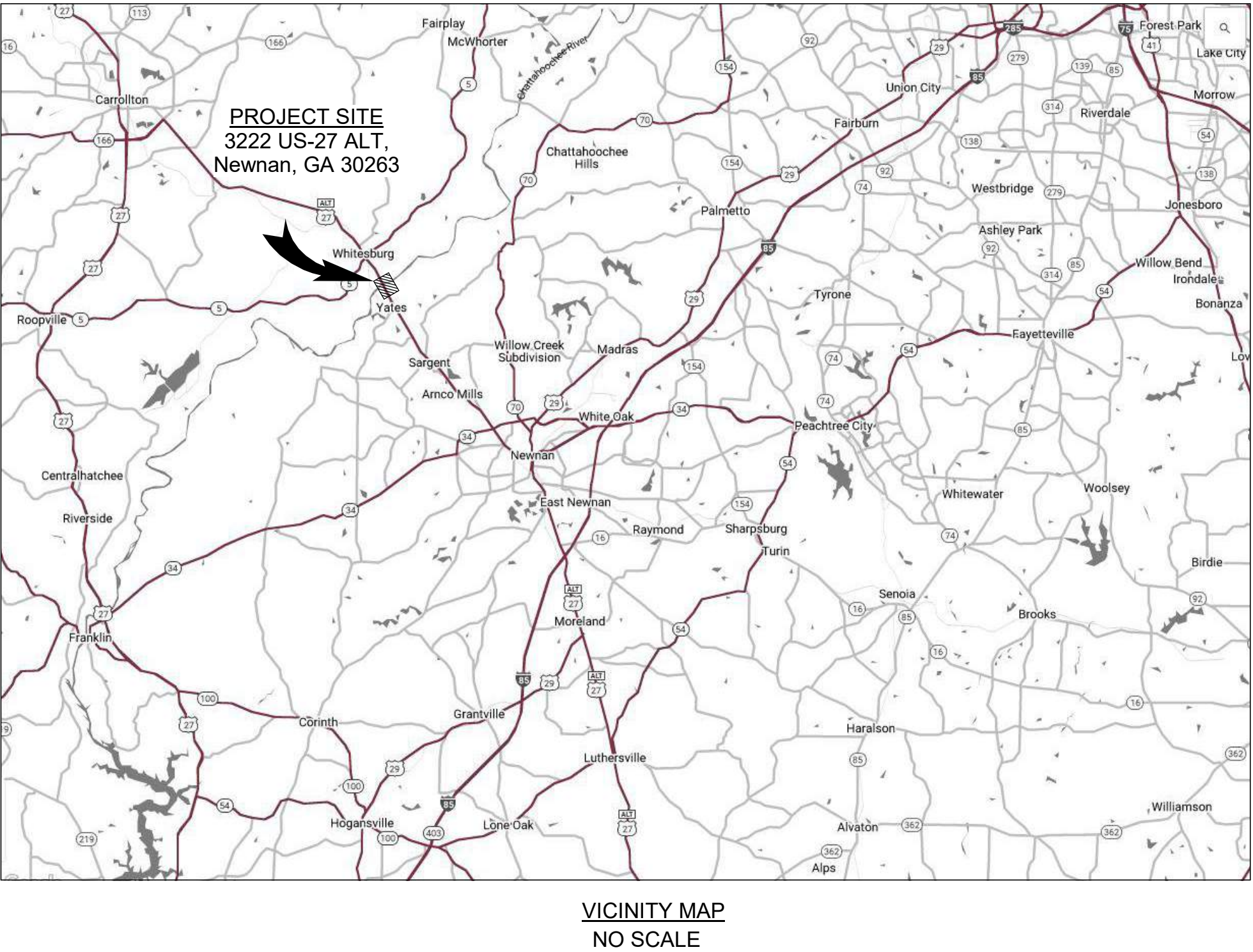
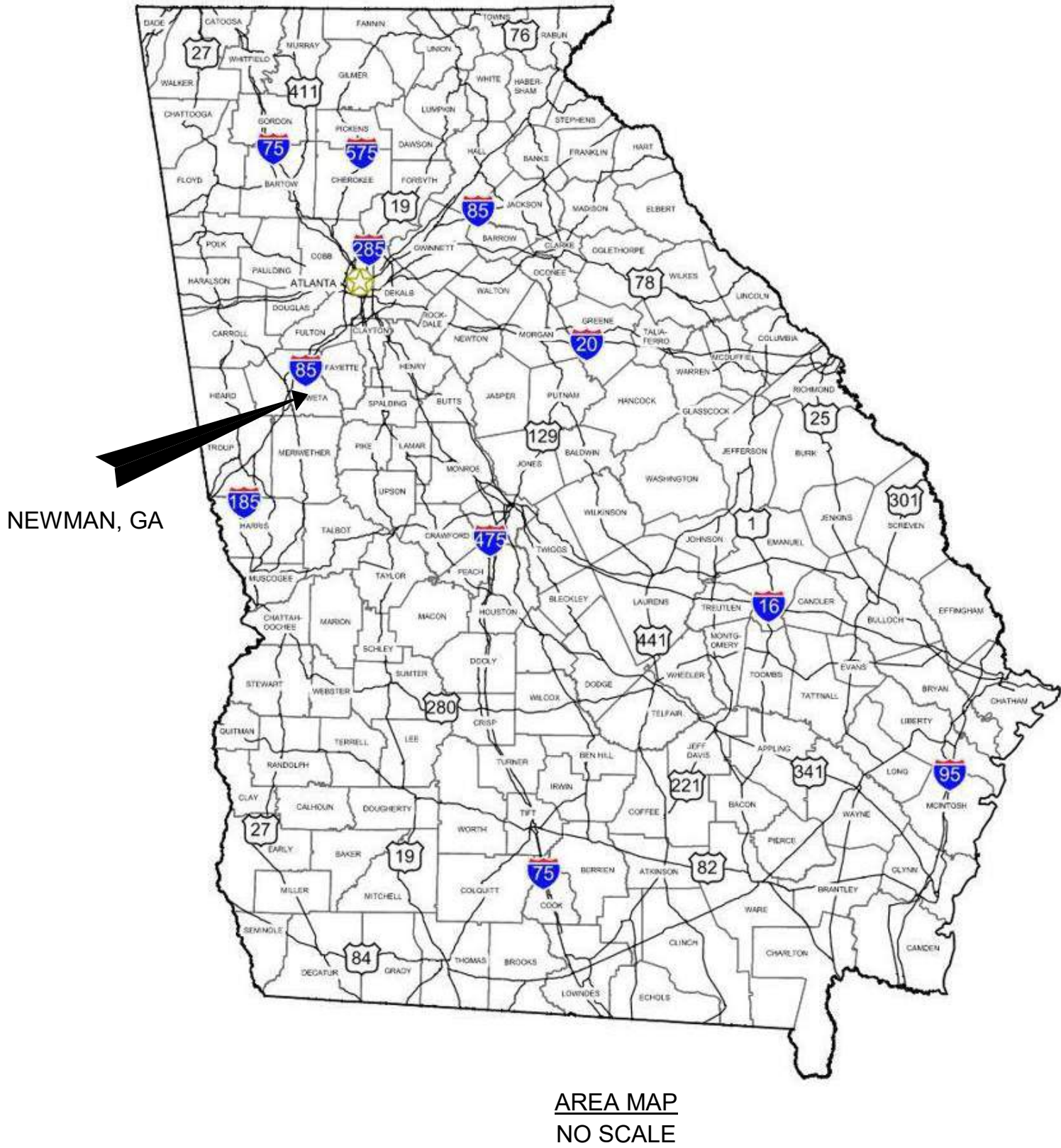
## **10% Design Drawing Set**



# COWETA COUNTY CHATTAHOOCHEE RIVER INTAKE, PUMP STATION AND RAW WATER PIPELINE

## COWETA COUNTY WATER & SEWERAGE AUTHORITY

10% DESIGN - NOT FOR CONSTRUCTION



GARVER PROJECT NO. 22W38085  
SEPTEMBER 2022



200 Galleria Drive SE; Suite 520,  
Atlanta GA 30339  
Phone: 678-264-8904



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REV	DATE	DESCRIPTION	BY

COWETA COUNTY WATER &  
SEWERAGE AUTHORITY  
NEWNAN, GA  
COWETA COUNTY  
CHATTAHOOCHEE RIVER  
INTAKE, PUMP STATION  
AND RAW WATER PIPELINE

COVER SHEET

JOB NO.: 22W38085  
DATE: SEPT. 2022  
DESIGNED BY: GWK  
DRAWN BY: EGB  
CHECKED BY: JAP

BAR IS ONE INCH ON  
ORIGINAL DRAWING  
IF NOT ONE INCH ON THIS SHEET,  
ADJUST SCALES ACCORDINGLY.

DRAWING NUMBER  
**01-G001**  
SHEET  
NUMBER **01**



Revit File: BM 360/22W38085 - Coweta County Water MP\Coweta\_General.rvt  
Plot Date: 9/22/2022 2:58:44 PM

01 - GENERAL		
SHEET NO.	DWG. NO.	DESCRIPTION
01	01-G001	COVER SHEET
02	01-G002	INDEX OF DRAWINGS
03	01-G003	GENERAL CONVENTIONS AND ABBREVIATIONS
04	01-G004	CIVIL NOTES AND LEGEND 1
05	01-G005	CIVIL NOTES AND LEGEND 2
06	01-G006	PROCESS & INSTRUMENTATION DIAGRAM NOTES, LEGENDS, AND ABBREVIATIONS
07	01-G007	STRUCTURAL NOTES, LEGENDS, AND ABBREVIATIONS
08	01-G009	PROCESS MECHANICAL NOTES, LEGENDS, AND ABBREVIATIONS
09	01-G010	BUILDING MECHANICAL NOTES, LEGENDS, AND ABBREVIATIONS
10	01-G011	ELECTRICAL NOTES, LEGENDS, AND ABBREVIATIONS

05 - SITE CIVIL		
SHEET NO.	DWG. NO.	DESCRIPTION
11	05-C100	EXISTING SITE PLAN - OVERVIEW
12	05-C101	PROPOSED SITE PLAN - OVERVIEW
13	05-C300	PROPOSED SITE PLAN - ENLARGED

08 - PROCESS AND INSTRUMENTATION DIAGRAMS		
SHEET NO.	DWG. NO.	DESCRIPTION
14	08-I101	INTAKE SCREENS P&ID
15	08-I102	PUMP STATION P&ID

10 - CHATTAHOOCHEE RAW WATER PUMP STATION		
SHEET NO.	DWG. NO.	DESCRIPTION
16	10-S101	INTAKE PUMP STATION OVERALL PLAN
17	10-S201	INTAKE PUMP STATION BUILDING ELEVATIONS 1
18	10-S202	INTAKE PUMP STATION BUILDING ELEVATIONS 2
19	10-P101	INTAKE PUMP STATION OVERALL PLAN - 30 MGD
20	10-P102	INTAKE PUMP STATION OVERALL PLAN - 21 MGD
21	10-P103	WETWELL PLAN
22	10-P104	INTAKE PUMP STATION PUMP ROOM PLAN
23	10-P105	INTAKE PUMP STATION SURGE TANK PLAN
24	10-P106	INTAKE SCREENING PLAN
25	10-P301	INTAKE PUMP STATION SECTIONS 1
26	10-P302	INTAKE PUMP STATION SECTIONS 2
27	10-P304	INTAKE SCREENING SECTIONS AND DETAILS 1
28	10-P305	INTAKE SCREENING SECTIONS AND DETAILS 2
29	10-P901	INTAKE PUMP STATION ISOMETRIC 1
30	10-P902	INTAKE PUMP STATION ISOMETRIC 2
31	10-P903	INTAKE SCREENING ISOMETRIC
32	10-E501	ONELINE DIAGRAM - 21 MGD

DRAWING NUMBER EXAMPLE		DRAWING NUMBER LEGEND				
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		C – CIVIL	S – STRUCTURAL	100 – SITE PLANS	100 – PLAN VIEWS	400 – LARGE SCALE VIEWS
		I – INSTRUMENTATION & CONTROL	P – PROCESS MECHANICAL	200 – GRADING & PAVING	200 – ELEVATIONS	500 – DETAILS
		X – DEMOLITION	M – BUILDING MECHANICAL	300 – PIPING & PROFILES	300 – SECTIONS	600 – DIAGRAM OR SCHED
		F – FIRE & LIFE SAFETY	E – ELECTRICAL			900 – ISOMETRICS
			T – TELECOMMUNICATIONS			



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BY	DESCRIPTION	DATE	REV

COWETA COUNTY WATER &  
SEWERAGE AUTHORITY  
NEWNAN, GA

COWETA COUNTY  
CHATTAHOOCHEE RIVER  
INTAKE, PUMP STATION  
AND RAW WATER PIPELINE

INDEX OF DRAWINGS

JOB NO.: 22W38085  
DATE: SEPT. 2022  
DESIGNED BY: GWK  
DRAWN BY: EGB  
CHECKED BY: JAP

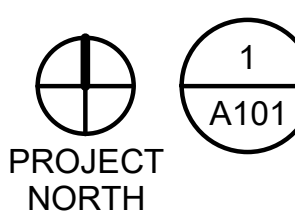
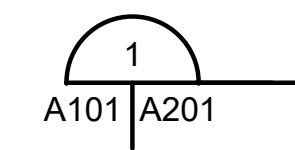
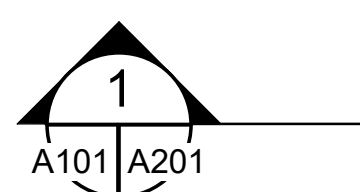
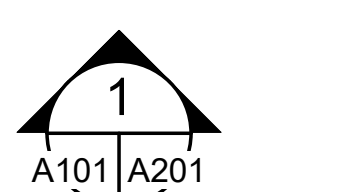
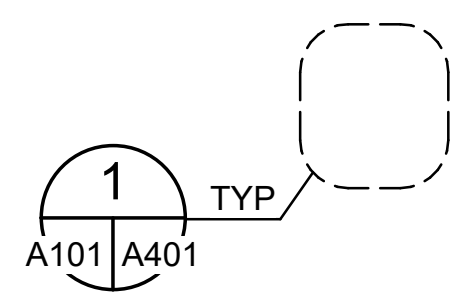
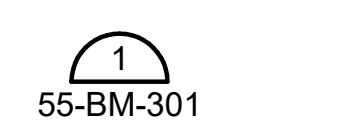
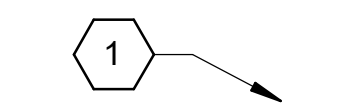
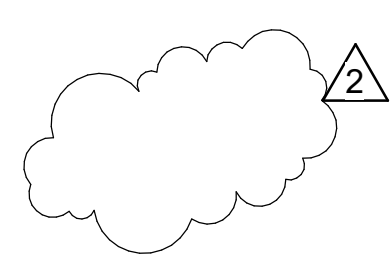
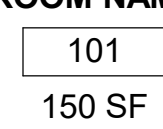
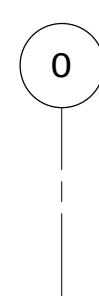

BAR IS ONE INCH ON  
ORIGINAL DRAWING 1"  
0  
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DRAWING NUMBER

**01-G002**


SHEET  
NUMBER **02**



GENERAL CONVENTIONS		
SYMBOL		DESCRIPTION
	<b>TITLE</b> SCALE: 1/8" = 1'-0"	TITLE DENOTES A PLAN VIEW LAYOUT (THIS EXAMPLE IS PLAN VIEW No. 1 ON SHEET A101)
	<b>TITLE</b> SCALE: 1/8" = 1'-0"	TITLE DENOTES AN ELEVATION, SECTION, OR DETAIL VIEW LAYOUT (THIS EXAMPLE IS DETAIL 1 ON SHEET A201 WHICH HAS A BACK REFERENCE ON SHEET A101)
		CALLOUT DENOTES A CUT SECTION TYPICAL REFERENCE (THIS EXAMPLE OCCURS ON SHEET A101 REFERENCING SECTION No. 1 ON SHEET A201)
		CALLOUT DENOTES AN ELEVATION REFERENCE (THIS EXAMPLE OCCURS ON SHEET A101 REFERENCING ELEVATION No. 1 ON SHEET A201)
		CALLOUT DENOTES AN ENLARGED AREA TYPICAL REFERENCE (THIS EXAMPLE OCCURS ON SHEET A101 REFERENCING DETAIL No. 1 ON SHEET A401)
		CALLOUT DENOTES A STANDARD DETAIL REFERENCE
		CALLOUT DENOTES A KEYED NOTE REFERENCE
		CLOUDED REGION INDICATES A REVISED AREA
<b>ROOM NAME</b> 		SYMBOL INDICATES A ROOM / AREA DESIGNATION WITH ROOM NUMBER AND SQUARE FOOTAGE
		SYMBOL INDICATES A STRUCTURAL GRIDLINE OR DATUM
		SYMBOL INDICATES A DATUM IN A SECTION OR ELEVATION

ABBREVIATIONS	
ABBREV	DESCRIPTION
ABV	ABOVE
AFF	ABOV FINISH FLOOR
ANSI	AMERICAN NATIONAL STANDARDS INSTITUE
ASTM	AMERICAN SOCIETY OF TESTING AND MATERIALS
AUX	AUXILIARY
AWWA	AMERICAN WATER WORKS ASSOCIATION
BKR	BREAKER
BOP	BOTTOM OF PIPE
BOS	BOTTOM OF STRUCTURE
CJ	CONSTRUCTION JOINT
CKT	CIRCUIT
CL	CENTERLINE
CMU	CONCRETE MASONRY UNIT
COGEN	COMBINED HEAT AND POWER GENERATION
COL	COLUMN
CONT	CONTINUOUS
DIA	DIAMETER
EA	EXHAUST AIR, EXPANSION ANCHOR, EACH
EL, ELEV	ELEVATION
ELEC	ELECTRICAL
ENCL	ENCLOSURE
FA	FIRE ALARM
FFE	FINISHED FLOOR ELEVATION
FL	FLOW LINE
FLR	FLOOR
FRP	FIBERGLASS REINFORCED PLASTIC
FT	FEET, FOOT
GA	GAUGE, GAGE
GALV	GALVANIZED
GFI, GFCI	GROUND FAULT CIRCUIT INTERRUPTER
GRND	GROUND
H, HT	HEIGHT
HOA	HAND-OFF-AUTOMATIC
HORIZ	HORIZONTAL
HP	HORSEPOWER, HEAT PUMP
HYD	HYDRANT
ID	INSIDE DIAMETER
IE	INVERT ELEVATION
kVA	KILOVOLT-AMPERES
KW	KILOWATTS
LBS, #	POUNDS
LF	LINEAR FEET
MAX	MAXIMUM
N/A	NOT AVAILABLE
NFPA	NATIONAL FIRE PROTECTION ASSOCIATION
NIC	NOT IN CONTRACT
NTS	NOT TO SCALE
OC	ON CENTER
OFCI	OWNER FURNISHED
OH	OVERHEAD
OSHA	OCCUPATIONAL SAFETY & HEALTH ADMINISTRATION
PD	PROCESS DRAIN
PIV	POST INDICATOR VALVE
PNL	PANEL
PRV	PRESSURE RELIEVE VALVE
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH

ABBREVIATIONS	
ABBREV	DESCRIPTION
PSIA	POUNDS PER SQUARE INCH ABSOLUTE
PSIG	POUNDS PER SQUARE INCH GAUGE
PVC	POLYVINYL CHLORIDE
RE:	REFERENCE, REFER
REINF	REINFORCEMENT
REQD	REQUIRED
RM	ROOM
SEC	SECTION
SF	SQUAURE FEET
SHT	SHEET
SPEC	SPECIFICATIONS
SQ	SQUARE
STA	STATION
STD	STANDARD
SURF	SURFACE
SUSP	SUSPEND, SUSPENDED
T&B	TOP AND BOTTOM
THRU	THROUGH
TYP	TYPICAL
U/F	UNDER FLOOR
U/G	UNDER GROUND
U/S	UNDER SLAB
UL	UNDERWRITERS LABORATORIES, INC.
UNO	UNLESS NOTED OTHERWISE
V	VOLT, VALVE
VA	VOLT-AMPERE
VERT	VERTICAL
W	WATT, WIRE, WIDTH, WINDOW, WATER
W/	WITH
W/O	WITHOUT
WS	WATERSTOP
WT	WATERTIGHT, WEIGHT
XMFR	TRANSFORMER




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BY				
DESCRIPTION				
DATE				
REV				

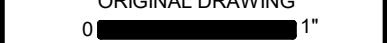


COWETA COUNTY WATER & SEWERAGE AUTHORITY  
NEWNAN, GA

COWETA COUNTY  
CHATTahooCHEE RIVER  
INTAKE; PUMP STATION  
AND RAW WATER PIPELINE

GENERAL CONVENTIONS AND ABBREVIATIONS

JOB NO.: 22W38085  
DATE: SEPT. 2022  
DESIGNED BY: GWK  
DRAWN BY: EGB  
CHECKED BY: JAP

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0"  1"  
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DRAWING NUMBER  
**01-G003**

SHEET NUMBER **03**

File: L:\2022\22W38085 - Coweta County Chattahoochee Water\Drawings\CRWPS-01-G004-G005-GN.dwg, Last Save: 9/12/2022 4:48 PM, Last saved by: BD Purnell  
Last plotted by: Purnell, Brianna D., Plot Style: AECmono.ctb, Plot Date: 9/12/2022 4:51 PM, Plotter Used: None

GENERAL CIVIL NOTES

1. SAFETY SHALL BE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE ENGINEER SHALL NOT BE RESPONSIBLE FOR SAFETY, MEANS, OR METHODS OF THE CONTRACTOR.
2. THE CONTRACTOR SHALL BE RESPONSIBLE FOR CONTACTING ALL APPROPRIATE AGENCIES BEFORE WORK COMMENCES TO VERIFY THE TYPE, LOCATION, PROTECTION REQUIREMENTS, DEPTH OF ALL EXISTING UTILITIES, DRAINAGE FACILITIES, AND OTHER OBSTRUCTIONS. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL COSTS ASSOCIATED WITH REPAIRING AND/OR REPLACING ANY SUCH ITEMS DAMAGED DURING CONSTRUCTION.
3. CAUTION: UNDERGROUND UTILITIES SHOWN ARE TAKEN FROM EXISTING RECORDS AND ARE SHOWN FOR THE CONVENIENCE OF THE CONTRACTOR ONLY. THE CONTRACTOR SHALL CONTACT ALL UTILITY OWNERS AND CONFIRM LOCATIONS OF UTILITIES AT LEAST 48 HOURS BEFORE BEGINNING CONSTRUCTION. THE CONTRACTOR SHALL ACCURATELY LOCATE AND UNCOVER ALL EXISTING UTILITIES BEFORE BEGINNING CONSTRUCTION. ANY DAMAGE RESULTING FROM THE CONTRACTOR'S OPERATIONS SHALL BE REPAIRED AT THE CONTRACTOR'S EXPENSE. WHERE CROSSING OF EXISTING UTILITIES OCCUR, PROVIDE 12" MINIMUM CLEARANCE EXCEPT WATER MAINS SHALL BE 24". CROSS UNDER ALL WATER MAINS WHERE NOT POSSIBLE TO PROVIDE 18" CLEARANCE.
4. SEWER AND WATER SERVICE SHALL BE MAINTAINED DURING ENTIRE CONSTRUCTION PERIOD OR TEMPORARY FACILITIES PROVIDED.
5. CONTRACTOR IS RESPONSIBLE FOR ALL DEWATERING ACTIVITIES AND ASSOCIATED PERMITS REQUIRED FOR ALL EXCAVATIONS REQUIRED TO COMPLETE THE PROJECT.
6. APPROXIMATE LOCATIONS OF OVERHEAD POWER LINES MAY OR MAY NOT BE SHOWN ON PLANS. CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR VERIFYING ALL LOCATIONS IN THE FIELD AND PLAN WORK IN THESE AREAS ACCORDINGLY.
7. CONTRACTOR SHALL BE RESPONSIBLE FOR SITE DRAINAGE AND COMPLIANCE WITH ALL GOVERNMENTAL STORM WATER REGULATIONS AND PERMITS (SWPPP) AS REQUIRED. CONTRACTOR SHALL OBTAIN NOI FROM APPROPRIATE STATE BODY PRIOR TO ANY CONSTRUCTION. CONTRACTOR SHALL BE RESPONSIBLE FOR ANY PERMITS REQUIRED FOR WORK WITHIN STREAMS.
8. IT SHALL BE THE CONTRACTOR'S SOLE RESPONSIBILITY TO PROVIDE TRAFFIC CONTROL AND SIGNAGE FOR THE DURATION OF PROJECT AS REQUIRED BY THE NATIONAL MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES - PART VI, AND/OR ALL OTHER APPLICABLE GUIDELINES OF GDOT, COUNTY, CITY OR ANY OTHER AUTHORITIES HAVING JURISDICTION OVER THE PROJECT AREAS. ALL ROAD CLOSURE MUST BE APPROVED BY THE CITY OF NEWMAN TRAFFIC ENGINEER PRIOR TO ANY PUBLIC ROAD CLOSURES.
9. CONTRACTOR SHALL MAINTAIN TRAFFIC FLOW TO RESIDENCES AND BUSINESSES WITH MINIMUM DISRUPTION OF ACCESS.
10. ALL STREETS AND DRIVEWAYS SHALL BE OPEN CUT UNLESS NOTED OTHERWISE.
11. ALL EXCAVATION OR BACKFILL OUTSIDE TRAFFIC WAYS SHALL BE COMPACTED TO MIN 95% STANDARD PROCTOR DENSITY TO PREVENT SETTLEMENT.

PAVING AND GRADING NOTES

1. ALL PAVING MATERIALS AND CONSTRUCTION SHALL MEET THE GDOT STANDARD SPECIFICATIONS UNLESS OTHERWISE NOTED.
2. ANY PAVEMENT DAMAGED DURING CONSTRUCTION SHALL BE REPAIRED TO EQUAL OR BETTER CONDITION AT THE CONTRACTORS EXPENSE.
3. ANY DISTURBED AREAS NOT SPECIFICALLY DESIGNATED TO BE GRADED SHALL BE RESTORED TO EQUAL OR BETTER CONDITION AND SHALL BE GRADED TO DRAIN AS APPROVED BY THE ENGINEER.
4. FINAL PAVEMENT SURFACES SHALL NOT BE PLACED UNTIL ALL MAJOR CONSTRUCTION ACTIVES HAVE CONCLUDED.
5. ANY CHANGES TO FINAL GRADE ELEVATIONS AS SHOWN ON THE PLANS SHALL BE APPROVED BY THE ENGINEER.
6. ALL ASPHALT AND CONCRETE PAVING REMOVED AND REPLACED SHALL BE NEAT SAW CUT.
7. ALL OPEN CUT TRAFFIC WAYS (ROADS, PARKING LOTS, DRIVES, ETC.) AND ALL AREAS LYING WITHIN PRISM OF TRAFFIC WAYS, SHALL HAVE CRUSHED STONE BACKFILL COMPACTED WITH VIBRATORY COMPACTOR MAXIMUM 6" LIFTS AND COMPACTED TO MINIMUM 100%-98% MODIFIED PROCTOR DENSITY TO PREVENT SETTLEMENT FOR ITS ENTIRE TRENCH HEIGHT AND WIDTH. COMPACTED "PUG-MIX" SHALL BE USED AND MAINTAINED IN TOP 12" OF TRENCH HEIGHT AS REQUIRED TO PREVENT AGGREGATE LOSS DUE TO TRAFFIC.

YARD PIPING NOTES

1. MINIMUM COVER OVER PIPING SHALL BE 3'-0", MEASURED FROM FINISHED GRADE.
2. PROVIDE MINIMUM PIPE COVER, AS SPECIFIED. IN GENERAL LAY PIPE TO UNIFORM GRADES BETWEEN THE ELEVATIONS SHOWN, UNLESS OTHERWISE APPROVED. IN SOME CASES, EXISTING CONDITIONS PROHIBIT UNIFORM GRADES BETWEEN THE ELEVATIONS SHOWN, AND FIELD ADJUSTMENTS TO UNIFORM GRADES ARE REQUIRED AS APPROVED BY ENGINEER.
3. SIZE OF FITTINGS SHOWN ON PLANS SHALL CORRESPOND TO ADJACENT STRAIGHT RUN OF PIPE, UNLESS OTHERWISE INDICATED. TYPE OF JOINT AND FITTING MATERIAL SHALL BE AS SPECIFIED FOR ADJACENT STRAIGHT RUN OF PIPE.
4. ALL JOINTS SHALL BE WATERTIGHT.
5. THRUST AT FITTINGS SHALL BE RESISTED BY RESTRAINED JOINTS AS SPECIFIED AND AS REQUIRED TO RESIST THRUST, UNLESS OTHERWISE APPROVED BY ENGINEER. SEE THRUST RESTRAINT DETAILS  $\left(\frac{D40}{2339-012}\right)$ .
6. CONTRACTOR SHALL LOCATE AND UNCOVER ALL CONNECTIONS TO EXISTING LINES, AND ANY POSSIBLE CONFLICTS WITH PROPOSED FACILITIES AND VERIFY LOCATION, ELEVATION, PIPE MATERIAL, AND PIPE O.D. PRIOR TO ANY CONSTRUCTION.
7. CONTRACTOR SHALL MAINTAIN AND PROTECT ALL EXISTING BURIED PIPING AND UTILITIES. THE CONTRACTOR IS RESPONSIBLE FOR REPAIRING ANY DAMAGED UNDERGROUND FACILITIES.
8. ALL SMALL DIAMETER PIPING SHALL BE INSTALLED AS SHOWN ON DRAWINGS WITH ALL FITTINGS AND VALVES AS REQUIRED TO PROVIDE A FUNCTIONAL PIPELINE AS SPECIFIED.
9. ALL BURIED VALVES SHALL BE INSTALLED WITH VALVE BOX AS SPECIFIED.
10. ALL PIPELINE SHUTDOWNS SHALL BE COORDINATED WITH THE OPERATORS. A WRITTEN WORK PLAN SHALL BE SUBMITTED AND APPROVED BY THE ENGINEER AND CITY 24 HOURS PRIOR TO ANY SHUTDOWNS.
11. ROCK SHALL BE UNDERCUT A MINIMUM OF 4" AND PIPE BEDDED IN STONE. NO SEPARATE PAY ITEM EXISTS FOR ROCK EXCAVATION. ALL EXCAVATION SHALL BE CONSIDERED TO BE UN-CLASSIFIED EXCAVATION AND SUBSIDIARY TO OTHER BID ITEMS.
12. CONTRACTOR SHALL BE RESPONSIBLE FOR THE PROPER DISPOSAL OF THE EXISTING PIPE, EXISTING MANHOLES, AND ANY EXCESS MATERIALS RESULTING FROM THE WORK.
13. WHERE BYPASS PUMPING IS REQUIRED DURING THE PROJECT, PUMPING SHALL BE HELD TO A MINIMUM. ROUND-THE-CLOCK BYPASS PUMPING IS NOT ALLOWED. AT END OF EACH DAYLIGHT CONSTRUCTION PERIOD, EXISTING WATER WILL BE TEMPORARILY ROUTED TO NEW OR EXISTING PIPES WITH FITTINGS, PIPE, HOSE, OR OTHER APPURTENANCES AS REQUIRED AND DITCH LINES SHALL BE BACKFILLED TO EXISTING GRADE. COST OF THIS WORK SHALL BE INCLUDED IN PIPE INSTALLATION UNLESS LISTED AS A SEPARATE BID ITEM.
14. CONTRACTOR SHALL PREVENT STORM WATER AND DEBRIS FROM ENTERING PIPES AND MANHOLES AT ALL TIMES. ALL PIPES AND MANHOLES SHALL BE SECURELY PLUGGED AT THE END OF EACH DAY.

ABBREVIATIONS				ABBREVIATIONS			
ABBREV	DESCRIPTION	ABBREV	DESCRIPTION	ABBREV	DESCRIPTION	ABBREV	DESCRIPTION
ABDN	ABANDON	MGD	MILLION GALLONS PER DAY	W/O	WITHOUT		
AC	ACRE	MH	MANHOLE	WL	WATER LINE		
AFF	ABOVE FINISHED FLOOR	MIN	MINIMUM	WSL	WATER SERVICE LINE		
ALUM	ALUMINUM SULFATE	MISC	MISCELLANEOUS	WS	WATERSTOP		
APPROX	APPROXIMATE	MJ	MECHANICAL JOINT	WTM	WATER TRANSMISSION MAIN		
ASPH	ASPHALT	N	NORTH	WWF	WELDED WIRE FABRIC BY		
ASSY	ASSEMBLY	NE	NORTHEAST				
BC	BACK OF CURB	NG	NATURAL GAS				
BLDG	BUILDING	NW	NORTHWEST				
BLK	BLOCK	NIC	NOT IN CONTRACT				
BM	BENCHMARK	NO.#	NUMBER				
B, BOT	BOTTOM	NTS	NOT TO SCALE				
BOP	BEGINNING OF PAVEMENT	NWSL	NORMAL WATER SURFACE LEVEL				
BVCS	BEGINNING OF VERTICAL CURVE STATION	OC	ON CENTER				
		OD	OUTSIDE DIAMETER				
C	CONDUIT	OVF	OVERFLOW				
CI	CAST IRON	PC	POINT OF CURVE				
CIP	CAST IRON PIPE	PD	PROCESS DRAIN				
CJ	CONSTRUCTION JOINT	PDMH	PROCESS DRAIN MANHOLE				
CL	CENTERLINE, CLASS	PE	PLAIN END				
CMU	CONCRETE MASONRY UNIT	PI	POINT OF INTERSECTION				
CONC	CONCRETE	PL, PLS	PLATE, PLACES				
CONN	CONNECTION	PO	PUSH ON				
CONT	CONTINUOUS	PP	POWER POLE				
CP	CONTROL POINT	PRC	POINT OF REVERSE CURVE				
CPB	CHEMICAL PULL BOX	PROP	PROPOSED				
CSL	CHEMICAL SAMPLE LINE, CHLORINE SOLUTION	PSI	POUNDS PER SQUARE INCH				
		PT	POINT OF TANGENT, POINT				
DA	DRAINAGE AREA	PVC	POLYVINYL CHLORIDE				
DI	DUCTILE IRON	PVI	POINT OF VERTICAL INTERSECTION				
DIA	DIAMETER	R, RAD	RADIUS				
DIP	DUCTILE IRON PIPE	RCP	REINFORCED CONCRETE PIPE				
EA	EACH	RED	REDUCER				
EFF	EFFLUENT	REINF	REINFORCEMENT				
EL, ELEV	ELEVATION	REQD	REQUIRED				
ELEC	ELECTRICAL	RJ	RESTRAINED JOINT				
EPB	ELECTRICAL PULLBOX	ROW, R/W	RIGHT-OF-WAY				
EOP	END/EDGE OF PAVEMENT	RP	RADIUS POINT				
ESMT	EASEMENT	RS	RESILIENT SEAT				
EVCS	END OF VERTICAL CURVE STATION	RT	RIGHT				
EQ	EQUAL	S	SOUTH, SLUDGE				
EW	EACH WAY	SCH	SCHEDULE				
EX	EXISTING	SD	STORM DRAIN				
EXP	EXPANSION	SDMH	STORM DRAIN MANHOLE				
FCJ	FLOOR CONSTRUCTION JOINT	SE	SOUTHEAST				
FES	FLARED END SECTION	SECT	SECTION				
FFE	FINISHED FLOOR ELEVATION	SF	SQUARE FEET, SILT FENCE				
FH	FIRE HYDRANT	SHT	SHEET				
FG, FIN GR	FINISH GRADE	SL	SAMPLE LINE				
FL	FLOWLINE	SPC	STANDARD PROCTOR DENSITY				
FLG	FLANGED	SPEC	SPECIFICATIONS				
FRP	FIBERGLASS REINFORCED PIPE	SQ	SQUARE				
FT	FEET, FOOT	SS	SANITARY SEWER				
FTG	FOOTING	SSL	SANITARY SEWER SERVICE LINE				
G	GUTTER	SSMH	SANITARY SEWER MANHOLE				
GL	GAS LINE	STA	STATION				
GR	GRADE	STD	STANDARD				
GV	GATE VALVE	SW	SIDEWALK, SOUTHWEST				
HDWL	HEADWALL	T&B	TOP AND BOTTOM				
HMA	HOT MIX ASPHALT	TBM	TEMPORARY BENCHMARK				
HORIZ	HORIZONTAL	TC	TIME CLOCK, TOP OF CURB				
HWY	HIGHWAY	TEMP	TEMPORARY, TEMPERED				
ID	INSIDE DIAMETER	THK	THICKNESS				
IN	INCHES	TS	TOP OF SIDEWALK				
INF	INFLUENT	TS&V	TAPPING SLEEVE AND VALVE				
INV	INVERT	TW	TOP OF WALL				
JT	JOINT	TYP	TYPICAL				
LEN	LENGTH	UNO	UNLESS NOTED OTHERWISE				
LF	LINEAR FEET	V	VOLT, VALVE				
LG	LONG	VC	VERTICAL CURVE				
LIN	LINEAL, LINEAR	VERT	VERTICAL				
LOC	LOCATION	VPI	VERTICAL POINT OF INTERSECTION				
LT	LEFT	VT	VENTILATOR				
MANUF	MANUFACTURER	W	WIDTH, WATER				
MAX	MAXIMUM	W/	WITH				

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DATE				
REV.				

COWETA COUNTY WATER & SEWERAGE AUTHORITY  
NEWMAN, GA

COWETA COUNTY  
CHATTAHOOCHEE RIVER  
INTAKE, PUMP STATION  
AND RAW WATER PIPELINE

CIVIL NOTES AND  
LEGEND 1

JOB NO.: 22W38085  
DATE: SEPT. 2022  
DESIGNED BY: CDG  
DRAWN BY: AGD  
CHECKED BY: JPS

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DRAWING NUMBER  
01-G004

SHEET  
NUMBER 04





Revit File: BW 360/122W36095 - Coweta County Water MP\Coweta\_General.rvt  
Plot Date: 9/12/2022 2:58:46 PM

EQUIPMENT

SYMBOL	DESCRIPTION
	ELECTRIC MOTOR
	VARIABLE SPEED DRIVE
	REDUCED VOLTAGE SOFT STARTER
	FULL VOLTAGE NON-REVERSING STARTER
	FULL VOLTAGE REVERSING STARTER
	SUBMERSIBLE PUMP CONTROL AND STATUS MODULE
	OPERATOR INTERFACE TERMINAL
	GRAPHIC DISPLAY TERMINAL (VFD)
	ELECTRIC GENERATOR
	HYDRAULIC MOTOR
	AIR MOTOR
	SHAFT
	COUPLING
	DYNAMIC PUMP
	SCREW PUMP OR CONVEYOR

SYMBOL	DESCRIPTION
	DYNAMIC COMPRESSOR
	POSITIVE DISPLACEMENT PUMP
	POSITIVE DISPLACEMENT COMPRESSOR
	EDUCTOR/EJECTOR
	HEATER, GENERAL
	HEATER W/FAN (INDUCED DRAFT)
	HEATER W/FAN (FORCED DRAFT)
	HEAT EXCHANGER, GENERAL
	HYDRAULIC CYLINDER
	AIR CYLINDER
	MIXER

ACCESSORIES AND APPURTENANCES

SYMBOL	DESCRIPTION
	UNION
	PLUG
	BLIND FLANGE
	HOSE CONNECTION
	SPRAY NOZZLE/DIFFUSER
	DRAIN
	FLEXIBLE CONNECTION, GENERAL
	FLEXIBLE HOSE
	QUICK CONNECTOR
	THREADED TAP
	FILTER
	Y TYPE STRAINER
	STRAINER
	EXPANSION JOINT
	FLOOR CLEANOUT
	FCO
	GAUGE P = PRESSURE V = VACUUM T = TEMPERATURE dP = PRESSURE DIFFERENTIAL
	SILENCER

VALVES

SYMBOL	DESCRIPTION
	CHECK VALVE
	GATE VALVE
	BUTTERFLY VALVE
	BALL CHECK
	BALL VALVE
	PLUG VALVE
	NEEDLE VALVE
	ROTARY VALVE
	KNIFE GATE VALVE
	MUD VALVE
	PINCH VALVE
	THREE-WAY VALVE
	FOUR-WAY VALVE
	GLOBE VALVE
	CHARACTERIZED OR VEE-BALL VALVE

VALVE OPERATORS

SYMBOL	DESCRIPTION
	HAND OPERATOR
	HAND OPERATOR (LONG)
	CHAIN OPERATOR
	FLOAT OPERATOR
	AIR DIAPHRAGM OPERATOR
	POSITIONER
	SOLENOID OPERATOR
	CYLINDER OPERATOR
	PRESSURE BALANCED DIAPHRAGM OPERATOR

DIGITAL SYSTEM INTERFACES

SYMBOL	DESCRIPTION
	ANALOG INPUT
	ANALOG OUTPUT
	DISCRETE INPUT
	DISCRETE OUTPUT

SYMBOL	DESCRIPTION
	STOP CHECK VALVE
	DIAPHRAGM VALVE (SELF CONTAINED)
	AIR RELIEF
	PRESSURE CONTROL
	PRESSURE RELIEF VALVE
	VACUUM RELIEF VALVE
	STOP GATE
	SLIDE GATE
	HOSE VALVE (HV-X) OR (V-X) X = NO. IN SPECS
	SAMPLE
	TELESCOPING VALVE

SYMBOL	DESCRIPTION
	MOTOR OPERATOR
	DIGITAL OPERATOR
	ELECTRO-HYDRAULIC OPERATOR
	FAIL ARROWS INDICATE OPEN PORTS
	LIMIT SWITCH
	MOTOR OPERATOR WITH INTEGRAL CONTROL DEVICES

INSTRUMENTS OR FUNCTIONS

SYMBOL	DESCRIPTION
	HMI SCREEN DISPLAY ELEMENT
	PANEL MOUNTED DEVICE
	FIELD DEVICE

INSTRUMENT PRIMARY ELEMENTS

SYMBOL	DESCRIPTION
	THREADED TAP
	THERMOWELL
	WEIR
	SIGHT FLOW INDICATOR
	ROTAMETER
	FLOW ORIFICE
	FLOW ORIFICE IN QUICK CHANGE FITTING
	SINGLE PORT PITOT
	AVERAGING PITOT STATION
	VENTURI TUBE
	FLUME
	POSITIVE DISPLACEMENT FLOWMETER
	MASS FLOWMETER
	SONIC FLOWMETER
	INSERTION MAGNETIC FLOWMETER
	pH ELECTRODE ASSEMBLY
	SUBMISSIVE PRESSURE SENSOR
	ULTRASONIC LEVEL TRANSMITTER
	RADAR LEVEL TRANSMITTER
	CONDUCTIVITY LEVEL PROBE
	FLOAT SWITCH
	TURBINE ELEMENT
	VORTEX SENSOR
	TARGET ELEMENT
	WATER HAMMER ARRESTOR

AUX INSTRUMENTS OR FUNCTIONS

SYMBOL	DESCRIPTION
	TEST POINT, TERMINAL BLOCK WITH SLIDING LINK AND MINI-BANANA SOCKETS
	PURGE OR FLUSHING DEVICE
	INTERLOCK LOGIC WITH REFERENCE
	ANNULAR SEAL
	DIAPHRAGM SEAL
	RESET FOR LATCH TYPE OPERATOR
	LOOP POWER SUPPLY

GENERAL IDENTIFICATION	
SYMBOL	DESCRIPTION
	EQUIPMENT TAG
SHEET INTERFACE	
	SHEET CODE DRAWING TYPE SHEET NUMBER
	PROCESS/DESCRIPTION
NON-SHEET INTERFACE	
	PROCESS/DESCRIPTION

EQUIPMENT LINE TYPES

LINE	DESCRIPTION
	PROPOSED OR NEW EQUIPMENT
	EXISTING EQUIPMENT

PIPING LINE TYPES

LINE	DESCRIPTION
	PROCESS LINE - PRIMARY
	PROCESS LINE - SECONDARY
	PROCESS LINE - EXISTING
	PACKAGE BOUNDARY
	AREA/BUILDING BOUNDARY
	SHOWN ELSEWHERE
	AIR

PIPING, MISCELLANEOUS

SYMBOL	DESCRIPTION
	SLOPE ARROW, ARROW DIRECTION DOWN
	OUTSIDE GRADE LEVEL
	LINE HEAT TRACED AND INSULATED ST = STEAM TRACED ET = ELECTRIC TRACED

INSTRUMENT LINE TYPES

LINE	DESCRIPTION
	PROCESS CONNECTION
	UNDEFINED SIGNAL
	PNEUMATIC SIGNAL
	ELECTRIC SIGNAL
	HYDRAULIC SIGNAL
	CAPILLARY OR FILLED TUBE
	ELECTROMAGNETIC OR SONIC SIGNAL (GUIDED)
	ELECTROMAGNETIC OR SONIC SIGNAL (UNGUIDED)
	INTERNAL SYSTEM LINK (SOFTWARE OR DATA LINK)
	MECHANICAL LINK

INSTRUMENT POWER SUPPLY

	IA - POWER SUPPLY, TYPE AND LEVEL SHOWN, ABBREVIATIONS AS FOLLOWS: AS - AIR SUPPLY IA - INSTRUMENT AIR PA - PLANT AIR ES - ELECTRIC SUPPLY GS - GAS SUPPLY
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BY	DESCRIPTION	DATE	REV

COWETA COUNTY WATER & SEWERAGE AUTHORITY  
NEWNAN, GA  
COWETA COUNTY  
CHATTahoochee RIVER  
INTAKE; PUMP STATION  
AND RAW WATER PIPELINE

PROCESS & INSTRUMENTATION  
DIAGRAM NOTES,  
LEGENDS, AND  
ABBREVIATIONS

JOB NO.: 22W38085  
DATE: SEPT. 2022  
DESIGNED BY: GWK  
DRAWN BY: EGB  
CHECKED BY: JAP

BAR IS ONE INCH ON ORIGINAL DRAWING  
0" 1"  
IF NOT ONE INCH ON THIS SHEET, ADJUST SCALES ACCORDINGLY.

DRAWING NUMBER  
**01-G006**  
SHEET NUMBER  
**06**



Revit File: BW 3601/22W38095 - Coweta County Water MP\Coweta\_General.rvt  
Plot Date: 9/12/2022 2:58:46 PM

GENERAL NOTES:

1.

GENERAL NOTES AND STANDARD DETAILS SHALL NOT REPLACE OR OVER RULE ANY STRUCTURE SPECIFIC NOTE, DETAIL, OR SPECIFICATION. STRUCTURE SPECIFIC NOTES AND DETAILS SHALL GOVERN OVER GENERAL NOTES AND STANDARD DETAILS.
2.

BUILDING RISK CATEGORY----- III
3.

DESIGN LIVE LOADS - 2009 IBC  
ROOF WITHOUT REDUCTION----- 25 PSF  
FLOORS:  
CORRIDORS----- 100 PSF  
ASSEMBLY AREAS----- 100 PSF  
BALCONIES----- 100 PSF  
RESTROOMS----- 80 PSF  
OFFICES----- 50 PSF  
STAIRS----- 100 PSF  
MOVABLE FILE ROOMS----- 150 PSF  
INDUSTRIAL AREAS----- 250 PSF  
EQUIPMENT ROOMS----- 250 PSF  
AREAS WITH UNRESTRICTED VEHICULAR ACCESS----- AASHTO HS20
4.

WIND LOAD PARAMETERS - ASCE 7-10  
BASIC WIND SPEED----- 90 MPH  
IMPORTANCE FACTORE, I----- 1.15  
EXPOSURE CATEGORY----- C  
GCP1 +/- 0.18 (ENCLOSED BUILDINGS)
5.

SEISMIC DESIGN PARAMETERS ----- IBC 2009  
IMPORTANCE FACTOR, I----- 1.25  
SITE CLASS----- D  
SEISMIC SPECTRAL ACCELERATIONS  
S<sub>s</sub> ----- 0.165g  
S<sub>1</sub> ----- 0.073g  
SEISMIC DESIGN CATEGORY----- B  
DESIGN SPECTRAL ACCELERATIONS  
S<sub>DS</sub> ----- 0.176g  
S<sub>D1</sub> ----- 0.117g  
RESPONSE MODIFICATION FACTOR, R----- SEE INDIVIDUAL PLANS  
BASIC SEISMIC FORCE RESISTING SYSTEM----- SEE INDIVIDUAL PLANS  
SEISMIC RESPONSE COEFFICIENT, C<sub>s</sub>----- SEE INDIVIDUAL PLANS  
ANALYSIS PROCEDURE----- EQUIVALENT LATERAL FORCE
6.

SNOW LOADS PARAMETERS ----- ASCE 7-05  
GROUND SNOW LOAD, P<sub>G</sub> ----- 10 PSF  
IMPORTANCE FACTOR, I ----- 1.10  
EXPOSURE FACTOR, C<sub>e</sub> ----- 0.90  
THERMAL FACTOR, C<sub>t</sub> ----- 1.0  
SLOPED ROOF SNOW LOAD, P<sub>s</sub> ----- XX
7.

THE STRUCTURE SHOULD NOT BE CONSIDERED TO BE STABLE DURING CONSTRUCTION UNTIL ALL ELEMENTS ARE IN PLACE AND CONNECTED. THE CONTRACTOR IS RESPONSIBLE FOR DESIGNING ALL TEMPORARY CONSTRUCTION BRACING, AS REQUIRED.
8.

CONSTRUCTION METHODS, PROCEDURES, AND SEQUENCES ARE THE CONTRACTOR'S RESPONSIBILITY. THE CONTRACTOR SHALL TAKE THE ALL NECESSARY MEANS TO MAINTAIN AND PROTECT THE STRUCTURAL INTEGRITY OF ALL CONSTRUCTION, NEW AND EXISTING, AT ALL STAGES.
9.

CONTRACTOR SHALL FIELD VERIFY ALL EXISTING DIMENSIONS AND CONDITIONS PRIOR TO ANY PERTINENT WORK. ALL EXISTING CONDITIONS AND DIMENSIONS SHALL BE NOTED ON THE SHOP DRAWINGS.
10.

COORDINATE WITH THE ARCHITECTURAL, CIVIL, MECHANICAL, STRUCTURAL, AND ELECTRICAL DRAWINGS, AND VERIFY THE LOCATIONS AND SIZES OF THE CHASES, OPENING, INSERTS, SLEEVES, FINISHES, CONDUITS, DEPRESSIONS AND OTHER PROJECT REQUIREMENTS.
11.

THE CONTRACTOR IS RESPONSIBLE FOR REVIEWING THE DRAWINGS AND EXISTING CONDITIONS TO DETERMINE WHERE OPENINGS ARE REQUIRED IN WALLS AND SLABS.
12.

STANDARD DETAILS APPLY UNLESS INDICATED OTHERWISE ON SPECIFIC STRUCTURE DRAWINGS.

STRUCTURAL STEEL NOTES:

1.

UNLESS OTHERWISE SPECIFIED, HOT-ROLLED STEEL BUILDING MEMBERS USING W-SHAPES SHALL BE ASTM A992; M-, S-, AND C- SHAPES ASTM A36; SQUARE, RECTANGULAR & ROUND HSS SHAPES ASTM A500 GRADE B; ANGLES AND MISCELLANEOUS STIFFENER PLATES ASTM A36.
2.

ALL SHEAR CONNECTIONS NOT DETAILED OR OTHERWISE NOTED SHALL BE STANDARD AISC WELDED OR AISC BOLTED CONNECTIONS AND SHALL HAVE SUFFICIENT CAPACITY TO SUPPORT THE END REACTION EQUAL TO ONE - HALF THE TOTAL UNIFORM CAPACITY SHOWN IN THE ALLOWABLE UNIFORM LOAD TABLES OF THE AISC STEEL CONSTRUCTION MANUAL - CURRENT EDITION.
3.

WELDING SHALL CONFORM WITH AWS D1.1 STRUCTURAL WELDING CODE.
4.

ALL BOLTS FOR BEAM CONNECTIONS SHALL BE ASTM A325 WITH A MINIMUM DIAMETER OF 1/2" UNO. ALL BOLTED CONNECTIONS SHALL BE BEARING TYPE CONNECTIONS UNLESS NOTED AS SLIP CRITICAL. WASHERS SHALL BE INSTALLED UNDER NUTS OF FASTENERS WHEN REQUIRED BY THE SPECIFICATION FOR STRUCTURAL JOINTS.
5.

ALL ANCHOR RODS SHALL BE ASTM F1554, GRADE 36 UNO.

GENERAL CONCRETE NOTES:

1.

STRUCTURAL CONCRETE FOR BUILDING MEMBERS SHALL HAVE A SPECIFIED COMPRESSIVE STRENGTH OF 4,500 PSI UNO.
2.

CONCRETE FOR SLABS SUBJECTED TO VEHICULAR WHEEL LOADS SHALL HAVE A SPECIFIED COMPRESSIVE STRENGTH OF 4,500 PSI.
3.

HOLD SLUMP TO 3 TO 4 INCHES IN ALL FLOOR SLABS.
4.

ALL EXPOSED CONCRETE EDGES SHALL BE CHAMFERED 3/4".
5.

NON-PRESTRESSED CONCRETE REINFORCEMENT SHALL CONFORM TO ASTM A615 GRADE 60.
6.

REINFORCEMENT LAP SPLICES SHALL CONFORM TO ACI 318 & ACI 350.
7.

CONCRETE COVER OVER REINFORCEMENT SHALL CONFORM TO THE MINIMUM REQUIRED BY ACI 318 & ACI 350, UNO.
8.

REINFORCEMENT DETAILING AND PLACEMENT SHALL CONFORM TO ACI 318 AND ACI 315.
9.

NO REINFORCING BAR SHALL BE WELDED OR FIELD BENT IN ANY MANNER, UNLESS SPECIFICALLY SHOWN OR NOTED ON THE DRAWINGS.
10.

PROVIDE FULL EMBEDMENT FOR ALL DOWELS. IF NOT OTHERWISE SPECIFIED, DOWEL SIZE AND SPACING SHALL BE THE SAME AS MAIN REINFORCING.
11.

MECHANICAL EQUIPMENT PADS ON FLOOR SLABS SHALL BE 6" THICK AND REINFORCED WITH #4 @ 12" EW, UNO.
12.

WATERSTOP PIPE SLEEVES REQUIRED ON ALL WATERTIGHT WALLS AND FLOORS.
13.

TREMIES REQUIRED ON ALL POURS DEEPER THAN 5 FEET.
14.

ALL WATERSTOPS TO BE 6" PVC FLAT RIBBED OR 9" PVC CENTER BULB AND PLACED AT ALL WATERTIGHT POURS, UNO. REFER TO DETAILS D03/3000-102A & B FOR WATERSTOP DETAILS.
15.

ALL WATERTIGHT "HYDRAULIC" CONCRETE STRUCTURES SHALL PASS A 72 HOUR LEAKAGE TEST PRIOR TO BACKFILLING AROUND STRUCTURE.
16.

WHEN WATERSTOP IS PLACED HORIZONTALLY IN SLABS, THE CONTRACTOR SHALL TEMPORARILY TIE UP OR CLAMP UP THE WATERSTOP UNTIL THE CONCRETE IS PLACED TO SLIGHTLY ABOVE THE DEPTH OF THE WATERSTOP.
17.

VERTICAL WATERSTOP SHALL BE FULLY EMBEDDED IN SLAB POUR AND WELDED TO ALL ADJACENT WATERSTOP.
18.

PROVIDE A MINIMUM OF SEVEN (7) DAYS BETWEEN ADJACENT POURS. CONCRETE SHALL MEET OR EXCEED DESIGN COMPRESSIVE STRENGTH PRIOR TO PLACING ADJACENT POURS.
19.

CONTRACTOR SHALL SUBMIT TO ENGINEER FOR APPROVAL A SCHEDULE AND SEQUENCE OF CONCRETE PLACEMENT. SEQUENCE SHALL INCLUDE PERMITTING CURE TIME BETWEEN PLACEMENTS AT ADJACENT PROPOSED PLACEMENTS.
20.

WALKWAYS AND SIDEWALKS SHALL BE POURED WITH SLIGHT SLOPE AND NO LOW SPOTS SO THEY WILL DRAIN FREE. ALL SLOPES SHALL COMPLY WITH ADA REQUIREMENTS.
21.

ALL CONSTRUCTION JOINTS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE INCORPORATED INTO THE STRUCTURE. ADDITIONAL CONSTRUCTION JOINTS TO FACILITATE CONSTRUCTION SHALL BE LOCATED AND DETAILED ON THE SHOP DRAWINGS FOR REVIEW. UNLESS INDICATED OTHERWISE, ALL CONSTRUCTION JOINTS TO BE KEYED. HORIZONTAL CONSTRUCTION JOINTS SHALL NOT BE PERMITTED IN WALLS AND BEAMS, UNLESS SHOWN ON THE STRUCTURAL DRAWINGS.
22.

SUBSTITUTION OF EXPANSION OR DRILLED AND GROUTED-IN ANCHORS FOR EMBEDDED ANCHORS SHOWN ON THE DRAWINGS WILL NOT BE PERMITTED UNLESS APPROVED BY ENGINEER.
23.

USE MANUFACTURER'S CERTIFIED DRAWINGS AND SPECIFICATIONS FOR EQUIPMENT ANCHORAGE AND DETAILS. VERIFY EQUIPMENT SIZE AND WEIGHTS WITH ENGINEER PRIOR TO CONSTRUCTION OF ANY AND ALL EQUIPMENT PADS.

FOUNDATION NOTES:

1.

FLOOR SLAB CONSTRUCTION JOINTS (C.J.) SHALL BE PLACED AS SHOWN ON FOUNDATION PLANS AND SUBMITTED TO ENGINEER FOR APPROVAL PRIOR TO CONCRETE PLACEMENT.
2.

FLOOR SLAB ISOLATION JOINTS SHALL BE 30# FELT UNO.
3.

CONCRETE FLOOR AND SLAB ON GRADE MAY BE PLACED IN LANES. SPACING OF JOINTS SHALL BE AS SHOWN ON THE FOUNDATION PLAN. WHEN LANE PLACEMENT IS USED, CONSTRUCTION JOINTS SHALL BE USED FOR THE JOINTS BETWEEN LANES. SAW CUT CRACK CONTROL JOINTS SHALL BE PROVIDED ACROSS EACH LANE AT SPACING SHOWN ON PLANS.
4.

ALL CONCRETE CORNERS SHALL BE CHAMFERED 3/4" ON THE EXTERIOR EXPOSED CORNER.
5.

COMPACTED GRANULAR FILL OR BASE COURSE ROCK AS INDICATED AND SPECIFIED.
6.

ALL PRESSURE PIPING BENEATH SLABS SHALL BE CONCRETE ENCASED.
7.

VAPOR BARRIER REQUIRED BENEATH ALL INTERIOR BUILDING SLABS.

GENERAL CONCRETE MASONRY NOTES:

1.

HOLLOW CMU UNITS SHALL CONFORM TO ASTM C90 TYPE 1 OF THE NOMINAL THICKNESS SHOWN ON THE DRAWINGS. ALL CMU SHALL BE 2 CELL BLOCK AND HAVE A SPECIFIED MINIMUM COMPRESSIVE STRENGTH OF 1900 PSI ON NET AREA AT 28 DAYS.
2.

MORTAR FOR CMU SHALL CONFORM TO ASTM C270, TYPE S UNO.
3.

GROUT FOR CMU GROUTED CELLS, LINTELS, COLUMNS, PILASTERS, BOND BEAMS AND BLOCKS WITH EMBEDDED ANCHORS SHALL BE 3,000 PSI PEA GRAVEL CONCRETE UNO.
4.

CMU REINFORCING BARS SHALL CONFORM TO ASTM A 615 GRADE 60. HORIZONTAL JOINT REINFORCEMENT SHALL BE COLD DRAWN WIRE WITH A MINIMUM OF 9 GAUGE LONGITUDINAL WIRE SIZE, UNO, WITH THE TYPE AND SPACING AS SHOWN ON THE DRAWINGS OR SPECIFIED.
5.

VERTICAL CELLS TO BE FILLED SHALL HAVE VERTICAL ALIGNMENT SUFFICIENT TO MAINTAIN A CLEAR UNOBSTRUCTED CONTINUOUS VERTICAL CELL NOT LESS THAN 2" X 3" IN PLAN DIMENSIONS.
6.

FOUNDATION DOWELS SHALL EXTEND INTO THE FOUNDATION CONCRETE A MINIMUM OF THE DEVELOPMENT LENGTH FOR BAR SIZE USED. LAPS OR SPLICES OF REINFORCING STEEL IN MASONRY SHALL BE AS INDICATED BELOW. THERE SHALL BE A FOUNDATION DOWEL FOR EACH VERTICAL WALL REINFORCEMENT.
7.

NORMAL VERTICAL WALL REINFORCING SHALL EXTEND CONTINUOUSLY FROM THE FOUNDATION TO EMBED AT LEAST 6" INTO THE TOP OF WALL BOND BEAM. AN ADDITIONAL ONE #4 HOOKED DOWEL SHALL BE INSTALLED IN THE TOP OF ALL MASONRY WALLS AT EACH VERTICAL WALL CELL CONTAINING VERTICAL REINFORCING. THE DOWELS SHALL PROJECT 24" INTO THE WALL AND HOOK 6" INTO THE WALL TOP BOND BEAM.
8.

CONTROL JOINTS SHALL BE OF THE TYPE AND AT THE LOCATIONS SHOWN ON THE DRAWINGS.
9.

CONTROL JOINTS SHALL BE AS DETAILED ON D04/2200-306. PROVIDE CONTROL JOINTS IN ALL MASONRY WALLS UNO. CONTROL JOINT SPACING SHALL BE AS RECOMMENDED BY THE NATIONAL CONCRETE MASONRY ASSOCIATION WITH A MAXIMUM SPACING OF 25'. SUBMIT JOINT LAYOUT PLAN FOR REVIEW PRIOR TO MASONRY WALL CONSTRUCTION.
10.

CORNER BLOCKS SHALL BE INTERWOVEN BETWEEN TWO WALLS.
11.

EVERY PIER OR WALL SECTION WHOSE WIDTH IS 3'-0" OR LESS WILL HAVE HORIZONTAL SHEAR STEEL IN THE FORM OF TIES. REF D04/2200-007.
12.

PROVIDE (2) ADDITIONAL #5 BARS ALONG SIDES, TOP AND BOTTOM OF ALL CMU WALL OPENINGS. EXTEND REINFORCING 24" BEYOND OPENING, UNO.
13.

VERTICAL WALL REINFORCING SHALL BE AS FOLLOWS: GROUT CELLS CONTAINING REINFORCEMENT, SOLID FULL HEIGHT UNO.
14.

UNO, LAP SPlice #5'S 3'-0"; #4'S 2'-0".


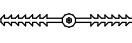




ABBREVIATIONS

ABBREV	DESCRIPTION
AL	ALUMINUM
ARCH	ARCHITECT, ARCHITECTURAL
BG	BACK GOUGE
BM	BEAM
CCJ	CRACK CONTROL JOINT
CJP	COMPLETE JOINT PENETRATION
COL	COLUMN
EF	EACH FACE
EJ	EXPANSION JOINT
ES	EVENLY SPACED, EACH SIDE
EW	EACH WAY
EXST	EXISTING
EXT	EXTERIOR
FD	FLOOR DRAIN
FND	FOUNDATION
FS	FOOTING STEP, FAR SIDE
IJ	ISOLATION JOINT
INT	INTERIOR
KIP	1,000 POUNDS
KLF	KIPS PER LINEAR FOOT
KSF	KIPS PER SQUARE FOOT
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LSL	LONG SLOT
MECH	MECHANICAL
NS	NEAR SIDE
OH	OPPOSITE HAND
OVS	OVERSIZED

ABBREVIATIONS

ABBREV	DESCRIPTION
PCF	POUNDS PER CUBIC FOOT
PFJ	PRE-FORMED JOINT
PJP	PARTIAL JOINT PENETRATION
PLF	POUNDS PER LINEAR FOOT
SIM	SIMILAR
SSL	SHORT SLOT
STL	STEEL
T&B	TOP AND BOTTOM
TOB	TOP OF BEAM
TOC	TOP OF CONCRETE
TOF	TOP OF FOOTING
TOS	TOP OF STEEL
VCJ	VERTICAL CONSTRUCTION JOINT

LEGEND:

	CENTERLINE	%	PERCENT		WATERSTOP
	DEGREES	℞	PLATE		DIRECTION OF DECK SPAN
	FLANGE	±	PLUS / MINUS		
	GRIDLINE				




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REGISTRATION NO.  
PEF007804

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COWETA COUNTY WATER & SEWERAGE AUTHORITY  
NEWNAN, GA

  
COWETA COUNTY  
CHATTahoochee RIVER  
INTAKE; PUMP STATION  
AND RAW WATER PIPELINE

STRUCTURAL NOTES, LEGENDS, AND ABBREVIATIONS

JOB NO.: 22W38085  
DATE: SEPT. 2022  
DESIGNED BY: GWK  
DRAWN BY: EGB  
CHECKED BY: JAP

BAR IS ONE INCH ON ORIGINAL DRAWING  
0"  1"  
IF NOT ONE INCH ON THIS SHEET, ADJUST SCALES ACCORDINGLY

DRAWING NUMBER  
**01-G007**  
SHEET NUMBER **07**

Revit File: BM 3601/22W38095 - Coweta County Water MP/Coweta\_General.rvt  
Plot Date: 9/12/2022 2:58:47 PM

<div><div><div>VALVE SYMBOLS</div><div><div></div><div>GATE</div></div><div><div></div><div>KNIFE GATE</div></div><div><div></div><div>BUTTERFLY</div></div><div><div></div><div>GLOBE</div></div><div><div></div><div>BALL</div></div><div><div></div><div>VEE-BALL</div></div><div><div></div><div>PLUG OR COCK</div></div><div><div></div><div>NEEDLE</div></div><div><div></div><div>DIAPHRAGM</div></div><div><div></div><div>PINCH</div></div><div><div></div><div>SWING CHECK</div></div><div><div></div><div>BALL CHECK</div></div><div><div></div><div>ROTARY</div></div><div><div></div><div>HOSE VALVE (HV-X) OR (V-X) X = NO. IN SPECS</div></div><div><div></div><div>SAMPLE</div></div><div><div></div><div>MUD</div></div><div><div></div><div>PRESSURE RELIEF</div></div><div><div></div><div>AIR RELEASE</div></div><div><div></div><div>VACUUM RELIEF</div></div><div><div></div><div>PRESSURE CONTROL</div></div><div><div></div><div>TELESCOPING</div></div><div><div></div><div>SLIDE GATE</div></div><div><div></div><div>STOP GATE</div></div><div><div></div><div>THREE-WAY / FOUR-WAY VALVE</div></div></div></div> <div><div><div>VALVE DESIGNATIONS</div><div><div>MANUAL VALVES AND CHECK VALVES</div><div><div>8" - V 500</div></div></div><div><div>SIZE OF VALVE VALVE DESIGNATION VALVE TYPE, SEE SPECIFICATIONS</div></div></div><div><div><div>CONTROL VALVES</div><div><div>X X</div></div></div><div><div>FLOW DIRECTION</div></div><div><div>VALVE SYMBOL</div></div><div><div>BUBBLE WITH FUNCTION AND THE TAG NUMBER DESIGNATION (SEE I&amp;C LEGEND FOR TAGGING INSTRUMENT IDENTIFICATION)</div></div></div><div><div><div>ACTUATOR SYMBOLS</div><div><div>PNEUMATIC DIAPHRAGM SPRING-OPPOSED, SINGLE OR DOUBLE ACTING</div></div><div><div>PNEUMATIC CYLINDER SINGLE OR DOUBLE ACTING ACTUATED BY ONE INPUT</div></div><div><div>ELECTRIC MOTOR</div></div><div><div>HYDRAULIC</div></div><div><div>MANUAL</div></div><div><div>SOLENOID</div></div></div></div></div> <div><div><div>PIPE AND FITTING SYMBOLS</div><div><div>DOUBLE LINE</div><div>SINGLE LINE</div></div><div><div>EXISTING PIPE</div><div>NEW PIPE</div><div>EXISTING PIPE TO BE ABANDONED</div><div>EXISTING PIPE TO BE REMOVED</div><div>WELDED JOINT</div><div>GROOVED END JOINT</div><div>FLANGED JOINT</div><div>MECHANICAL JOINT</div><div>BELL &amp; SPIGOT JOINT (LEADED)</div><div>HUB &amp; SPIGOT JOINT (RUBBER GASKET)</div><div>BALL JOINT</div><div>ADAPTER SIDE GROOVED END ADAPTER FLANGE</div><div>FLANGED COUPLING ADAPTER</div><div>FLANGED COUPLING ADAPTER WITH THRUST TIES</div><div>FLEXIBLE COUPLING</div><div>FLEXIBLE COUPLING WITH THRUST TIES</div><div>STEEL BELLOWS EXP. JOINT</div><div>ELASTOMER BELLOWS EXP. JOINT</div></div><div><div>DOUBLE LINE</div><div>SINGLE LINE</div></div><div><div>ELBOW UP</div><div>ELBOW DOWN</div><div>TEE UP</div><div>TEE DOWN</div><div>LATERAL UP</div><div>LATERAL DOWN</div><div>CONCENTRIC REDUCER</div><div>ECCENTRIC REDUCER</div><div>REDUCING BUSHING</div><div>UNION</div><div>CAP</div><div>ANCHOR</div><div>ELBOW, 90 DEGREE</div><div>CROSS</div><div>TEE</div><div>ELBOW, 45 DEGREE</div><div>LATERAL</div></div></div><div><div><div>ABBREVIATIONS</div><div><div>ABBREV</div><div>DESCRIPTION</div></div><div><div>AWWA</div><div>AMERICAN WATER WORKS ASSOCIATION</div></div><div><div>CPVC</div><div>CHLORINATED POLYVINYL CHLORIDE</div></div><div><div>DX</div><div>DIRECT EXPANSION</div></div><div><div>ECC</div><div>ECCENTRIC</div></div><div><div>EQUIP</div><div>EQUIPMENT</div></div><div><div>FLEX</div><div>FLEXIBLE</div></div><div><div>FOB</div><div>FLAT ON BOTTOM</div></div><div><div>FPM</div><div>FEET PER MINUTE</div></div><div><div>GAL</div><div>GALLON</div></div><div><div>GPD</div><div>GALLONS PER DAY</div></div><div><div>GPH</div><div>GALLONS PER HOUR</div></div><div><div>GPM</div><div>GALLONS PER MINUTE</div></div><div><div>N.O.</div><div>NORMALLY OPEN</div></div><div><div>NC</div><div>NORMALLY CLOSED</div></div><div><div>OS&amp;Y</div><div>OUTSIDE STEM AND YOKE</div></div><div><div>RPM</div><div>REVOLUTIONS PER MINUTE</div></div><div><div>SP</div><div>STATIC PRESSURE</div></div><div><div>VAC</div><div>VACUUM</div></div><div><div>VTR</div><div>VENT THROUGH ROOF</div></div><div><div>WC</div><div>WATER COLUMN</div></div><div><div>WPD</div><div>WATER PRESSURE DROP</div></div></div></div><div><div><div>FLOW STREAM IDENTIFICATION</div><div><div>ABBREV</div><div>DESCRIPTION</div></div><div><div>AHP</div><div>AIR, HIGH PRESSURE</div></div><div><div>ALP</div><div>AIR, LOW PRESSURE</div></div><div><div>BS</div><div>BLENDED SLUDGE</div></div><div><div>NG</div><div>NATURAL GAS</div></div><div><div>RW</div><div>RAW WATER</div></div><div><div>VENT</div><div>VENT</div></div></div><div><div><div>FLOW STREAM IDENTIFICATION</div><div><div>ABBREV</div><div>DESCRIPTION</div></div></div></div></div><div><div><div>MISCELLANEOUS PIPING SYMBOLS</div><div><div></div><div>STRAINER</div></div><div><div></div><div>SIGHT GLASS</div></div><div><div></div><div>FLEXIBLE (ELASTOMER) PIPE CONNECTION</div></div><div><div></div><div>GAUGE WITH COCK</div></div><div><div></div><div>THERMOMETER</div></div><div><div></div><div>ROTAMETER</div></div><div><div></div><div>PIG LAUNCHER</div></div><div><div></div><div>PIG CATCHER</div></div><div><div></div><div>BACKFLOW PREVENTER</div></div><div><div></div><div>REDUCER</div></div><div><div></div><div>FLEXIBLE HOSE</div></div><div><div></div><div>FLEXIBLE CONNECTOR</div></div><div><div></div><div>TYPICAL INSTRUMENT SYMBOL (SEE I&amp;C LEGEND)</div></div></div></div><div><div><div>PIPING DESIGNATION</div><div><div>EXAMPLE:</div><div></div><div>16" RAS</div><div>16" RAS</div><div>FLOW STREAM IDENTIFICATION</div><div>PIPE DIAMETER</div></div></div><div><div><div>PIPE FITTING AND END PATTERNS</div><div><div>B</div><div>BELL</div></div><div><div>F</div><div>FLANGE</div></div><div><div>GE</div><div>GROOVED END</div></div><div><div>MJ</div><div>MECHANICAL JOINT</div></div><div><div>PE</div><div>PLAIN END</div></div><div><div>S</div><div>SPIGOT</div></div><div><div>SOC</div><div>SOCKET</div></div><div><div>THD</div><div>THREADED</div></div></div><div><div><div>EXAMPLE:</div><div></div><div>MJ</div><div>F</div><div>PE</div></div></div></div></div><div><div><div>NOTES:</div><div><div>1. ONLY FLANGED END CONNECTIONS ARE SHOWN HERE FOR DOUBLE LINE FITTINGS; FITTINGS WITH OTHER END PATTERNS ARE SHOWN SIMILARLY ON THE CONSTRUCTION DRAWINGS. ALSO SEE PIPING SPECIFICATIONS.</div><div>2. SYMBOLS SHOWN HERE FOR SINGLE LINE FITTINGS ARE GENERIC ONLY. REFER TO PIPING SPECIFICATIONS FOR SPECIFIC END CONNECTIONS FOR SINGLE LINE PIPE AND FITTINGS.</div><div>3. EXISTING PIPE AND EQUIPMENT IS SHOWN WITH A DASHED LINE AND/OR SCREENED AND IS NOTED AS EXISTING. NEW PIPING AND EQUIPMENT IS SHOWN WITH A HEAVY LINE.</div></div></div><div><div><div>GENERAL PIPING NOTES:</div><div><div>1. LAY PIPE TO UNIFORM GRADE BETWEEN INDICATED ELEVATION POINTS.</div><div>2. SIZE OF FITTINGS SHOWN ON PLANS SHALL CORRESPOND TO ADJACENT STRAIGHT RUN OF PIPE, UNLESS OTHERWISE INDICATED. TYPE OF JOINT AND FITTING MATERIAL SHALL BE THE SAME AS SHOWN FOR ADJACENT STRAIGHT RUN OF PIPE.</div><div>3. LOCATION AND NUMBER OF PIPE HANGERS AND PIPE SUPPORTS SHOWN IS ONLY APPROXIMATE. FINAL SUPPORT REQUIREMENTS SHALL BE DETERMINED IN THE FIELD AND REVIEWED BY THE ENGINEER PRIOR TO INSTALLATION. MAXIMUM SPACING SHALL BE AS SPECIFIED.</div><div>4. ALL JOINTS SHALL BE WATERTIGHT. WALL PIPES OR PENETRATION SEALS SHALL BE USED WHEREVER PIPING PASSES FROM A STRUCTURE TO BACKFILL.</div><div>5. ALL FLEXIBLE CONNECTORS OR FLANGED COUPLING ADAPTERS SHALL BE PROVIDED WITH THRUST TIES, BLOCKS, OR ANCHORS, UNLESS OTHERWISE NOTED. THRUST PROTECTION SHALL BE ADEQUATE FOR TEST PRESSURES SPECIFIED.</div><div>6. SYMBOLS, LEGENDS, AND PIPE USE IDENTIFICATIONS SHOWN SHALL BE FOLLOWED THROUGHOUT THE PLANS, WHEREVER APPLICABLE. NOT ALL OF THE VARIOUS PIPING COMPONENTS ARE NECESSARILY USED IN THE PROJECT.</div><div>7. NUMBER AND LOCATION OF UNIONS SHOWN ON PLANS IS ONLY APPROXIMATE. PROVIDE ALL UNIONS NECESSARY TO FACILITATE CONVENIENT REMOVAL OF VALVES AND MECHANICAL EQUIPMENT.</div><div>8. WHERE A GROOVED END COUPLING IS SHOWN, IT SHALL BE THE RIGID JOINT TYPE, UNLESS OTHERWISE SPECIFIED. WHERE A FLANGED COUPLING ADAPTER IS SHOWN, A STANDARD FLANGE SHALL BE JOINED TO THE COUPLING ADAPTER.</div></div></div><div><div><div>GATE SYMBOLS</div><div><div>ELEVATION VIEW</div><div>PLAN VIEW</div></div><div><div></div><div>SLUICE</div></div><div><div></div><div>BUTTERFLY</div></div><div><div></div><div>FLAP</div></div><div><div></div><div>SHEAR</div></div><div><div></div><div>FABRICATED SLIDE</div></div><div><div></div><div>STOP LOG</div></div></div></div><div><div><div>SPECIAL INSTALLATION NOTE:</div><div><div>INSTALLATION DETAILS FOR DIVISION 26 ELECTRICAL ARE NOT SHOWN ON MECHANICAL DRAWINGS FOR CLARITY. REFER TO DIVISION 26 INSTRUMENT SPECIFICATIONS, INSTRUMENT LIST, AND DESIGN DETAILS. COORDINATE MATERIAL AND INSTALLATION REQUIREMENTS.</div></div></div></div></div><div><div><div>10% DESIGN - NOT FOR CONSTRUCTION</div><div><div>THIS DOCUMENT IS RELEASED FOR THE PURPOSE OF INTERIM REVIEW UNDER THE AUTHORITY OF: JOHN A. 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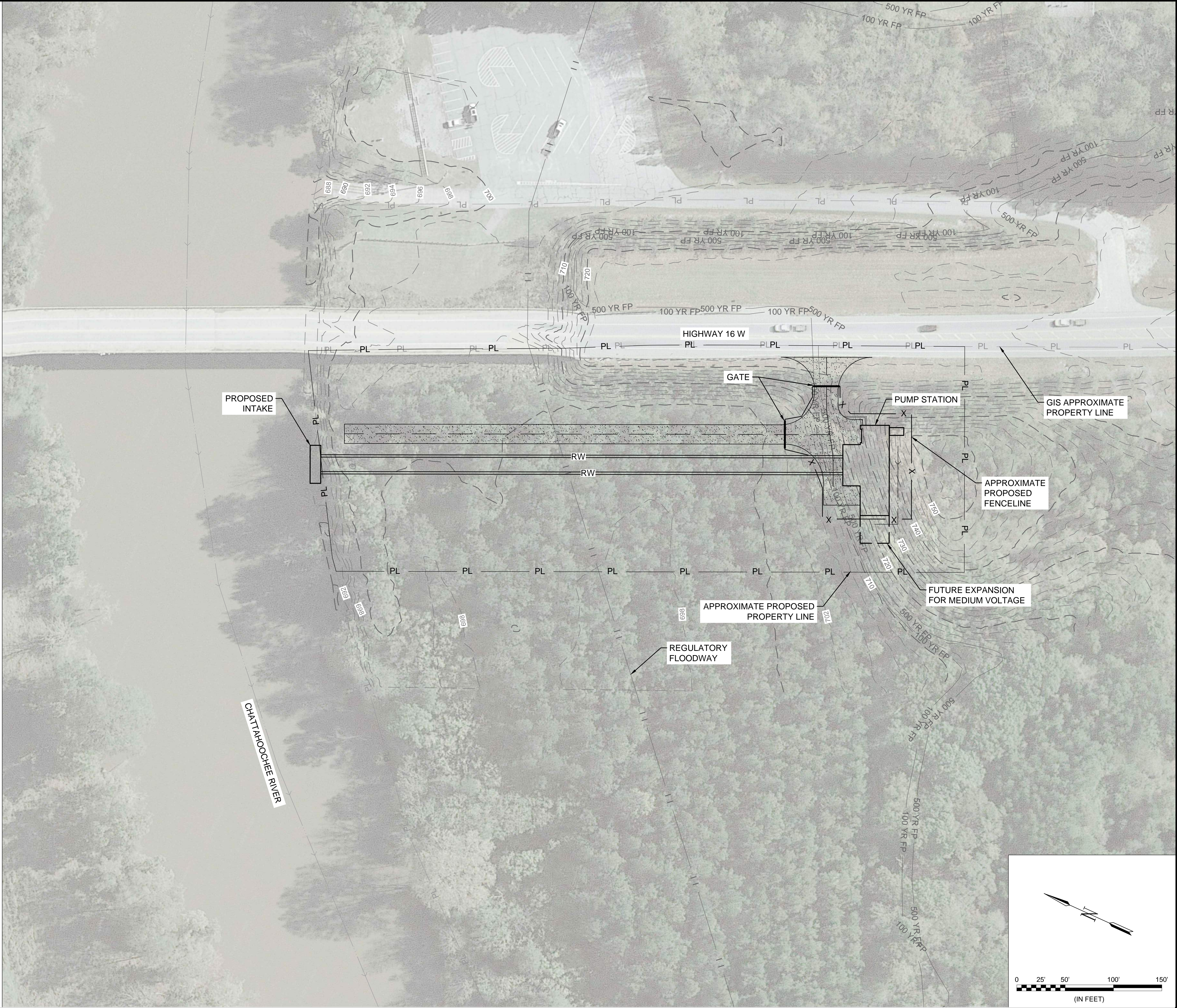






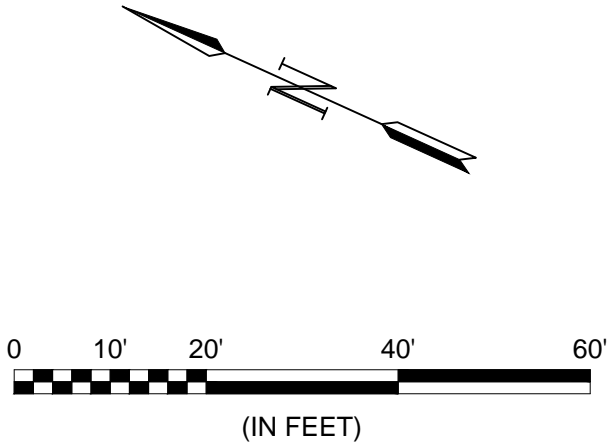
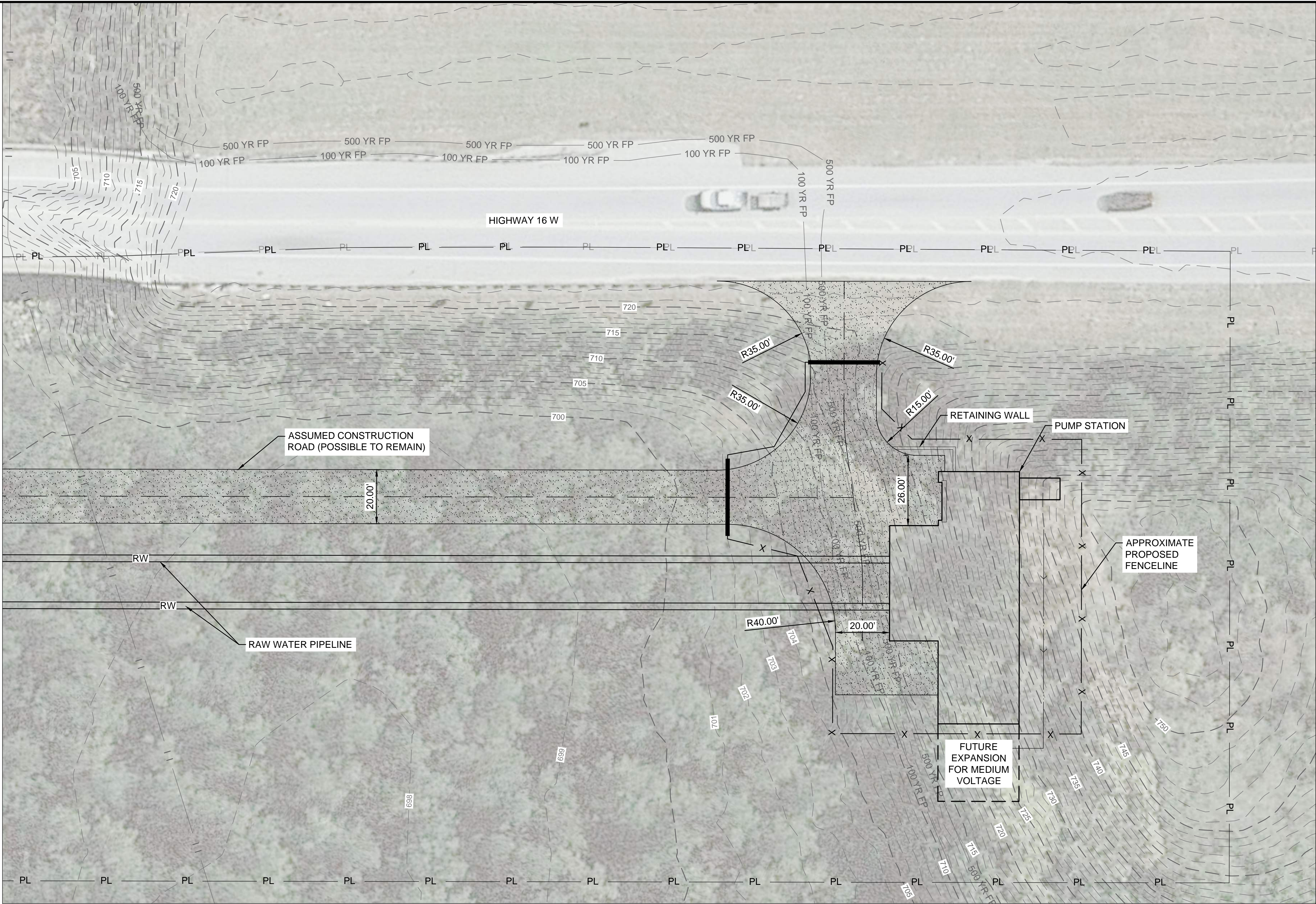



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REV.	DATE	DESCRIPTION	BY



COWETA COUNTY WATER & SEWERAGE AUTHORITY  
NEWNAN, GA

COWETA COUNTY  
CHATTAHOOCHEE RIVER  
INTAKE, PUMP STATION  
AND RAW WATER PIPELINE

PROPOSED SITE  
PLAN - ENLARGED

JOB NO.: 22W38085  
DATE: SEPT. 2022  
DESIGNED BY: CDG  
DRAWN BY: AGD  
CHECKED BY: JPS

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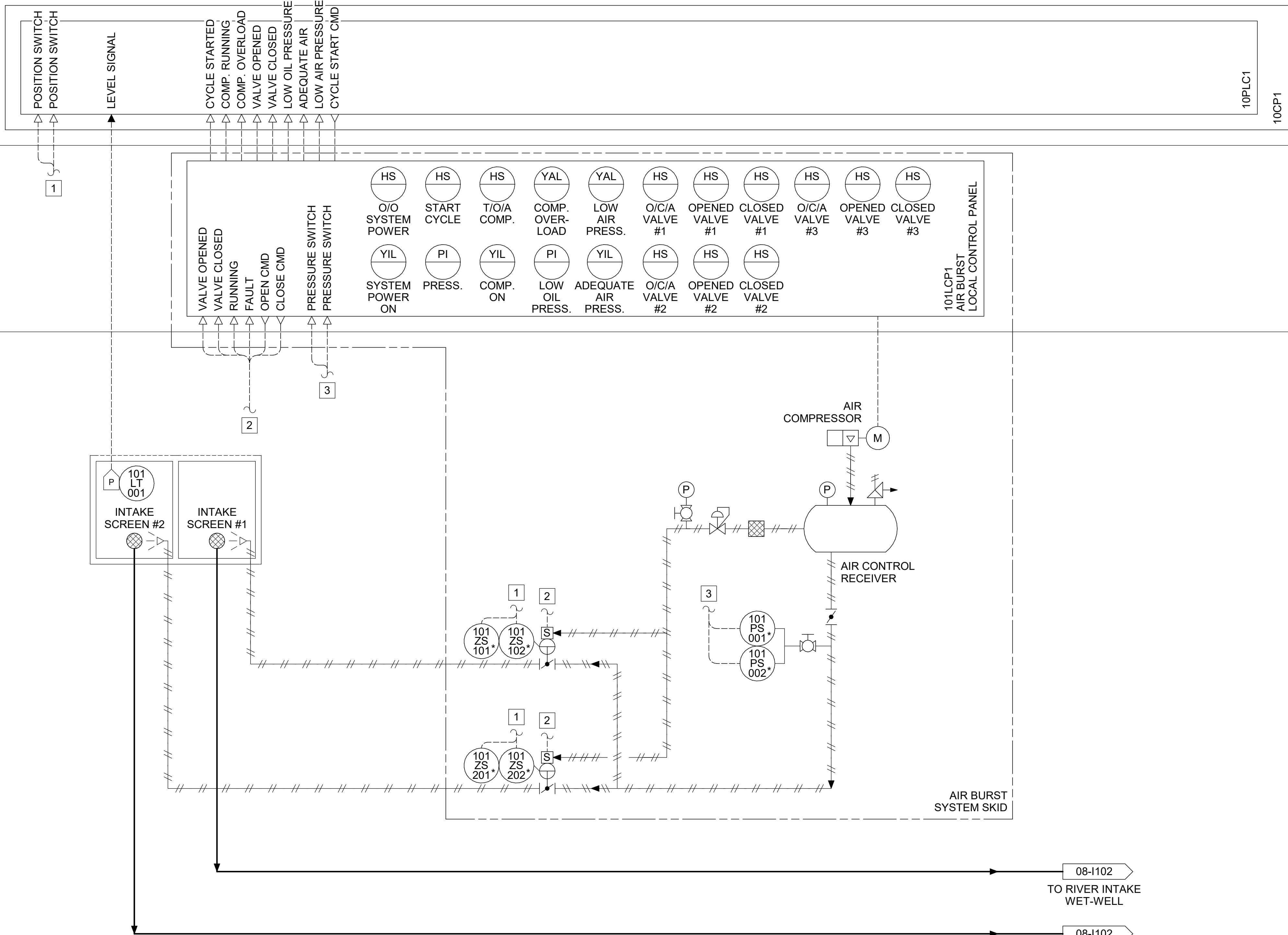
SHEET NUMBER  
13



1. SIGNAL LINES TO EQUIPMENT INTENDED ONLY TO INDICATE A REQUIRED ELECTRICAL CONNECTION. REFER TO PLAN SHEETS, CONTROL SCHEMATICS, AND I/O LISTS FOR ADDITIONAL INFORMATION ABOUT NUMBER AND TYPES OF SIGNAL WIRING.
2. EQUIPMENT WITH SYMBOL " \* " APPENDED TO IT IS MANUFACTURER SUPPLIED AS PART OF EQUIPMENT PACKAGE.

PLC  
LEVEL

1



08-I102  
TO RIVER INTAKE  
WET-WELL

08-I102  
TO RIVER INTAKE  
WET-WELL



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BY

DESCRIPTION

DATE \_\_\_\_\_

REV

COWETA COUNTY WATER &amp;

## INTAKE SCREENS

### P&ID

JOB NO.: 22W38095  
DATE: SEPT. 2022  
DESIGNED BY: JCW  
DRAWN BY: WBW  
CHECKED BY: JAP

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DRAWING NUMBER

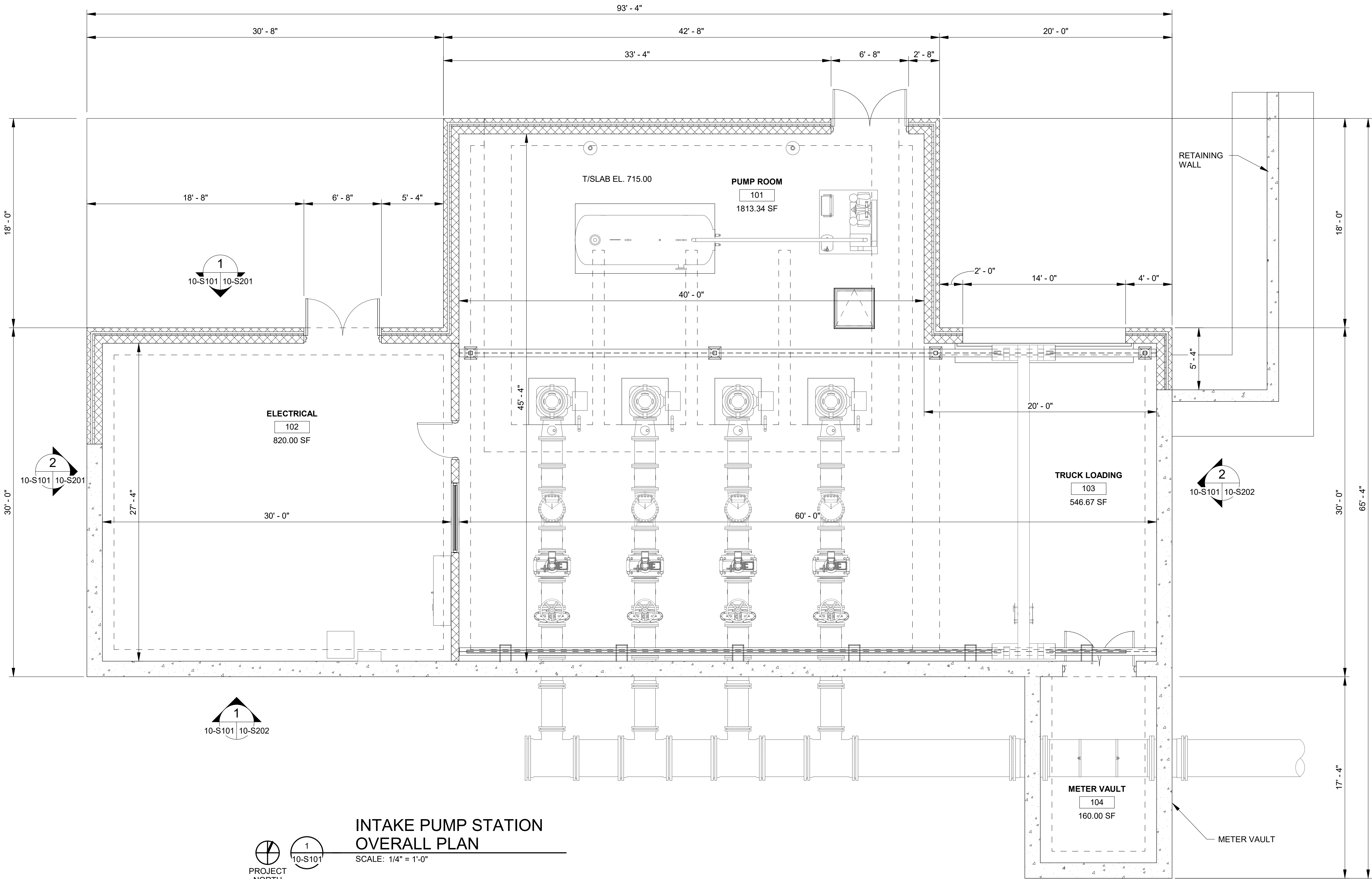
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SHEET  
NUMBER **14**



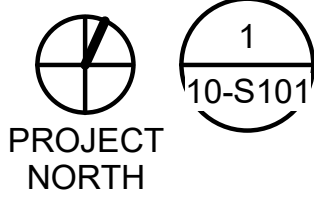


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## INTAKE PUMP STATION OVERALL PLAN

SCALE: 1/4" = 1'-0"



PROJECT  
NORTH



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REV	DATE	DESCRIPTION	BY



COWETA COUNTY WATER &  
SEWERAGE AUTHORITY  
NEWNAN, GA  
COWETA COUNTY  
CHATTahoochee RIVER  
INTAKE, PUMP STATION  
AND RAW WATER PIPELINE

INTAKE PUMP  
STATION OVERALL  
PLAN

JOB NO.: 22W38095  
DATE: SEPT. 2022  
DESIGNED BY: SJC  
DRAWN BY: EGB  
CHECKED BY: KAM

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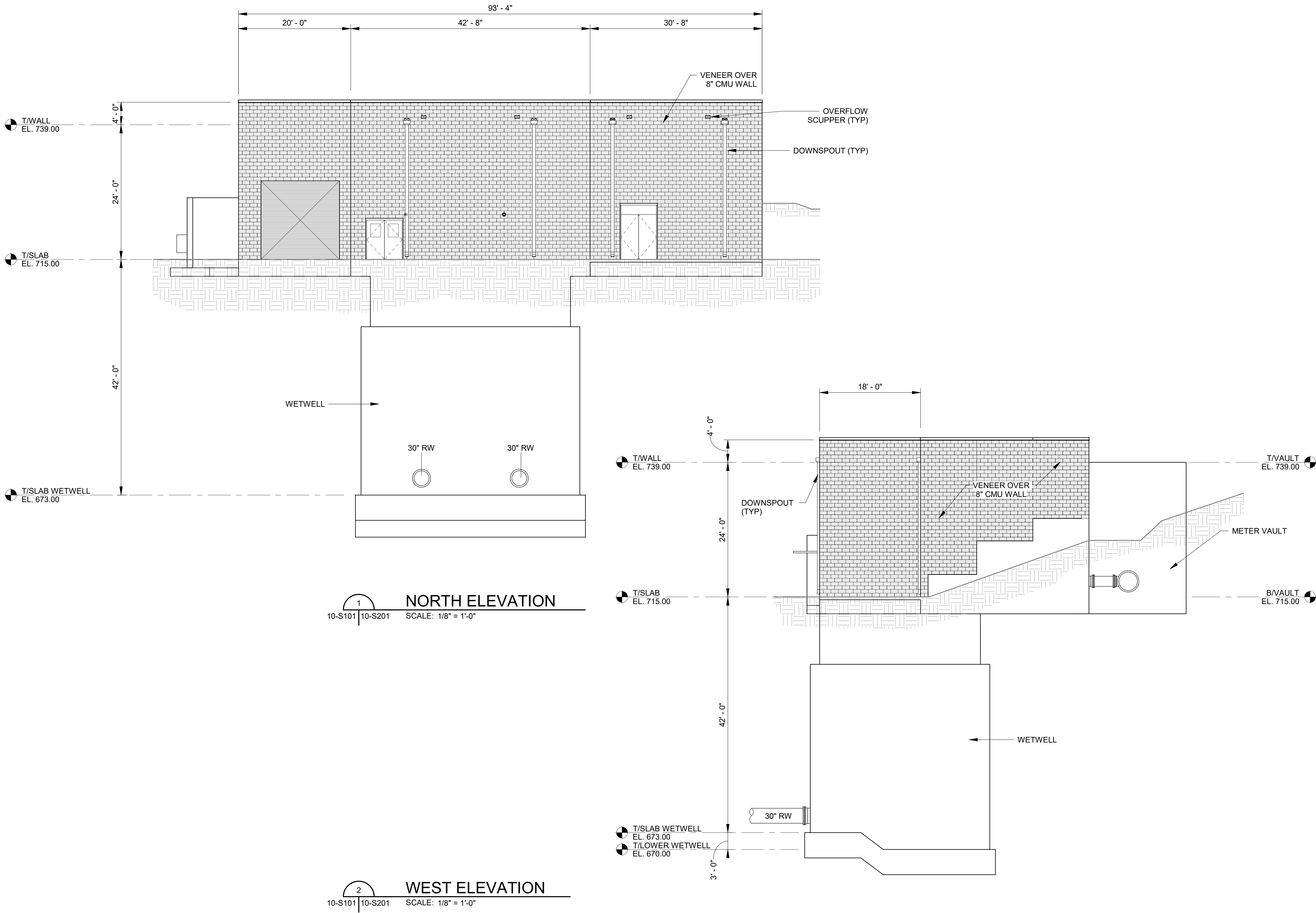
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**10-S101**

SHEET  
NUMBER **16**



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BY	DESCRIPTION	DATE	REV



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NEWNAN, GA  
COWETA COUNTY  
CHATTAHOOCHEE RIVER  
INTAKE, PUMP STATION  
AND RAW WATER PIPELINE

INTAKE PUMP  
STATION BUILDING  
ELEVATIONS 1

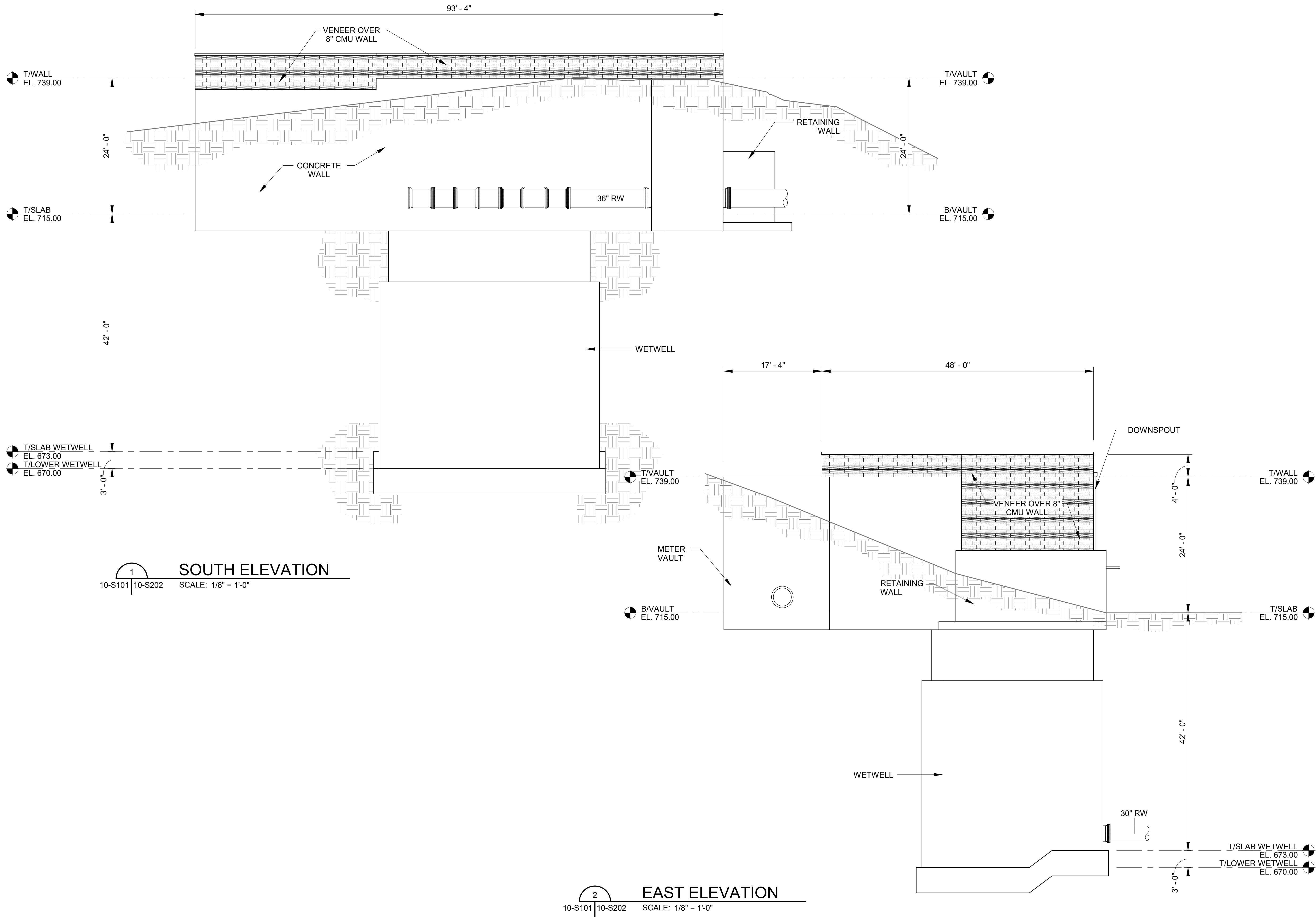
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DATE: SEPT. 2022  
DESIGNED BY: SJC  
DRAWN BY: EGB  
CHECKED BY: KAM


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SHEET NUMBER  
**17**

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COWETA COUNTY  
CHATTahoochee RIVER  
INTAKE, PUMP STATION  
AND RAW WATER PIPELINE

INTAKE PUMP  
STATION BUILDING  
ELEVATIONS 2

JOB NO.: 22W38095  
DATE: SEPT. 2022  
DESIGNED BY: SJC  
DRAWN BY: EGB  
CHECKED BY: KAM

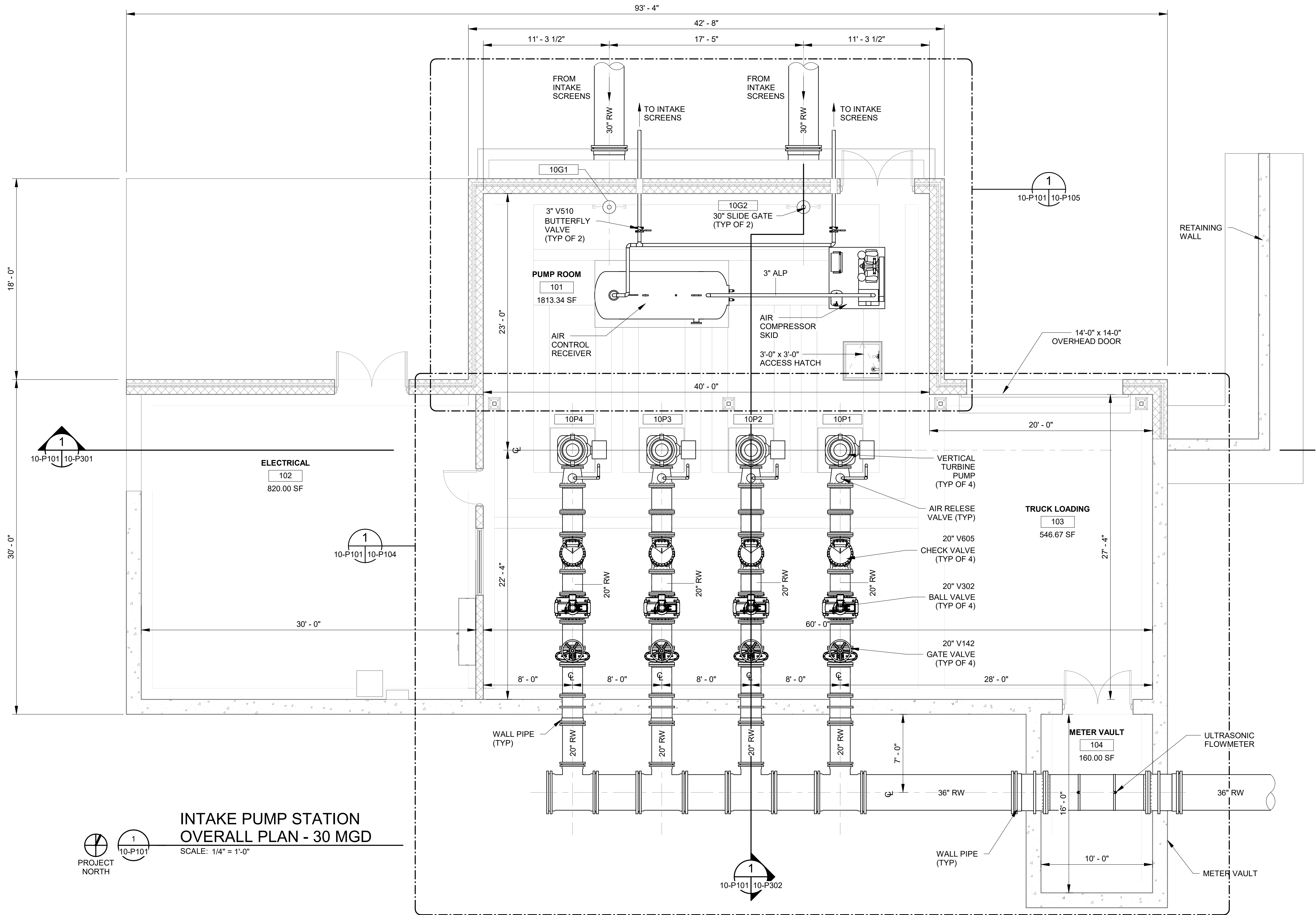
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NUMBER **18**



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INTAKE PUMP STATION  
OVERALL PLAN - 30 MGD

SCALE: 1/4" = 1'-0"



1  
10-P101



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NEWNAN, GA  
COWETA COUNTY  
CHATTAHOOCHEE RIVER  
INTAKE, PUMP STATION  
AND RAW WATER PIPELINE

INTAKE PUMP  
STATION OVERALL  
PLAN - 30 MGD

JOB NO.: 22W38095  
DATE: SEPT. 2022  
DESIGNED BY: GWK  
DRAWN BY: EGB  
CHECKED BY: JAP

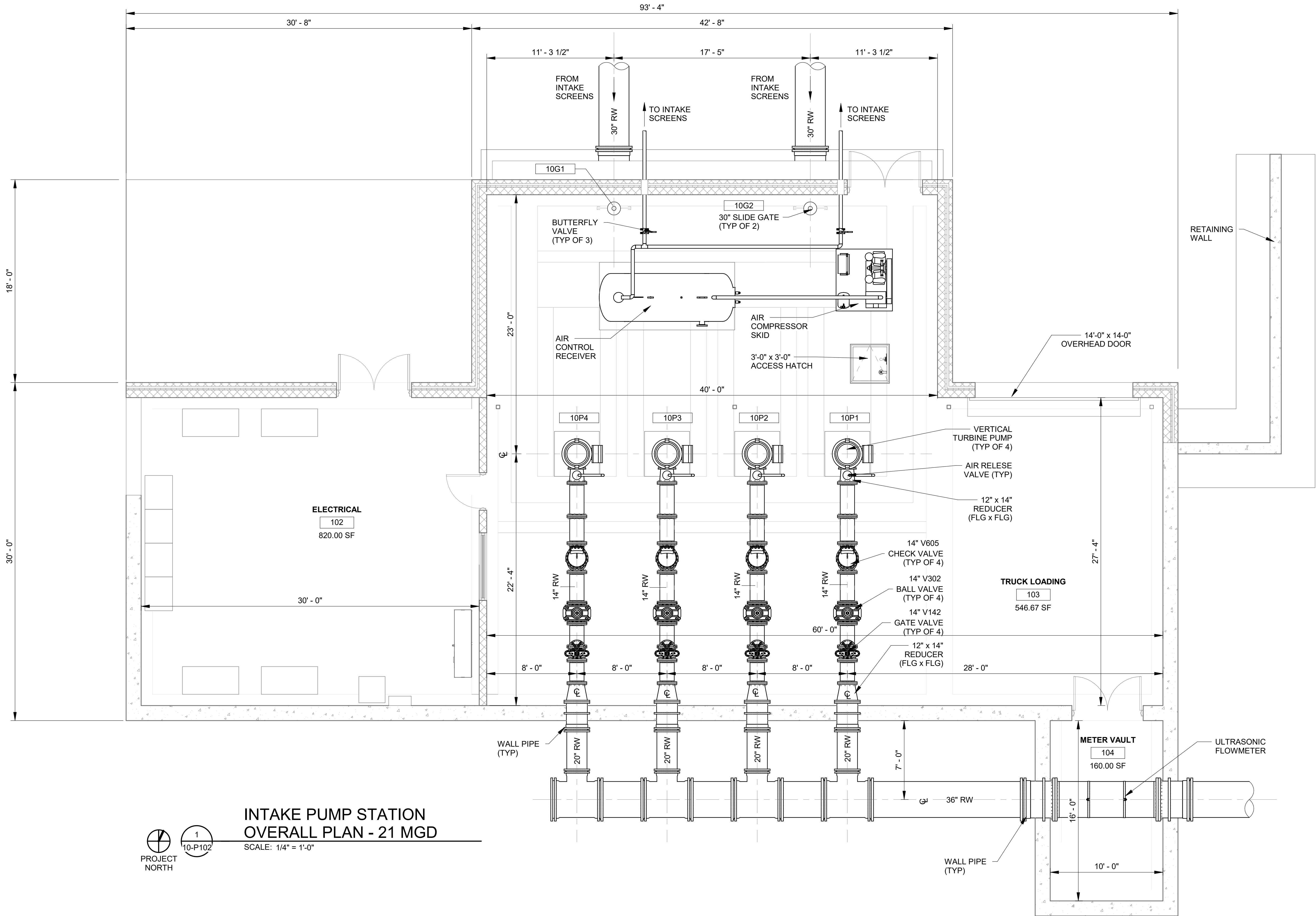
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DRAWING NUMBER

10-P101

SHEET  
NUMBER 19

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1  
10-P102

## INTAKE PUMP STATION OVERALL PLAN - 21 MGD

SCALE: 1/4" = 1'-0"



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REV	DATE	DESCRIPTION	BY



COWETA COUNTY WATER &  
SEWERAGE AUTHORITY  
NEWNAN, GA  
COWETA COUNTY  
CHATTahoochee RIVER  
INTAKE, PUMP STATION  
AND RAW WATER PIPELINE

INTAKE PUMP  
STATION OVERALL  
PLAN - 21 MGD

JOB NO.: 22W38095  
DATE: SEPT. 2022  
DESIGNED BY: GWK  
DRAWN BY: EGB  
CHECKED BY: JAP

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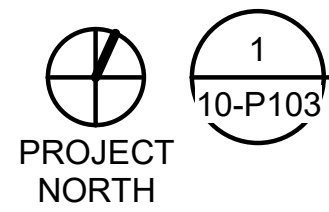
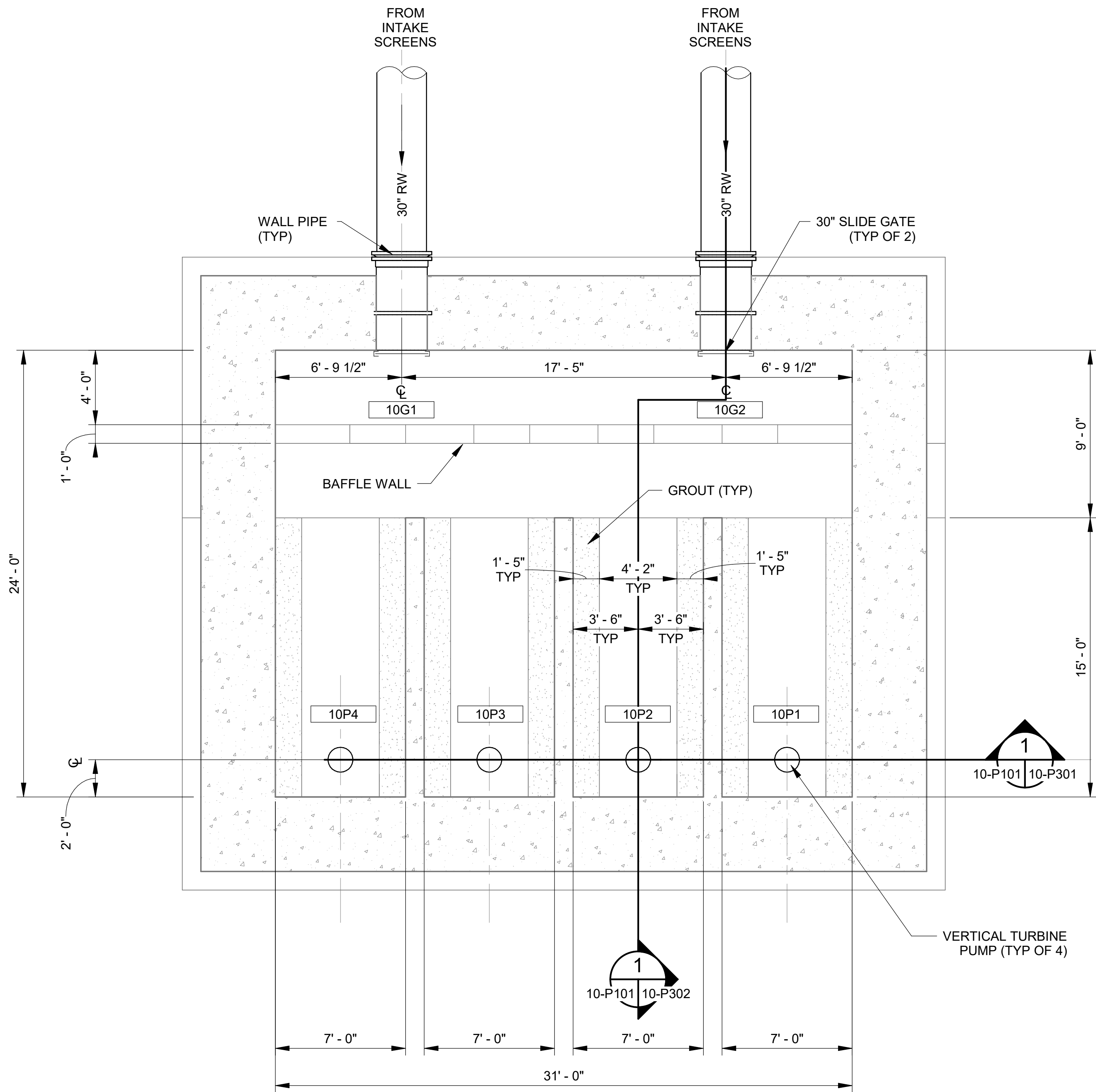
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**10-P102**

SHEET  
NUMBER **20**



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# WETWELL PLAN

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SEWERAGE AUTHORITY  
NEWNAN, GA  
COWETA COUNTY  
CHATTahoochee RIVER  
INTAKE, PUMP STATION  
AND RAW WATER PIPELINE

## WETWELL PLAN

JOB NO.: 22W38095  
DATE: SEPT. 2022  
DESIGNED BY: GWK  
DRAWN BY: EGB  
CHECKED BY: JAP

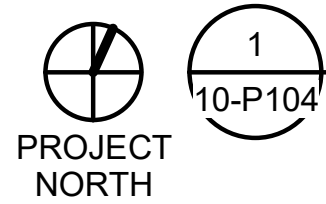
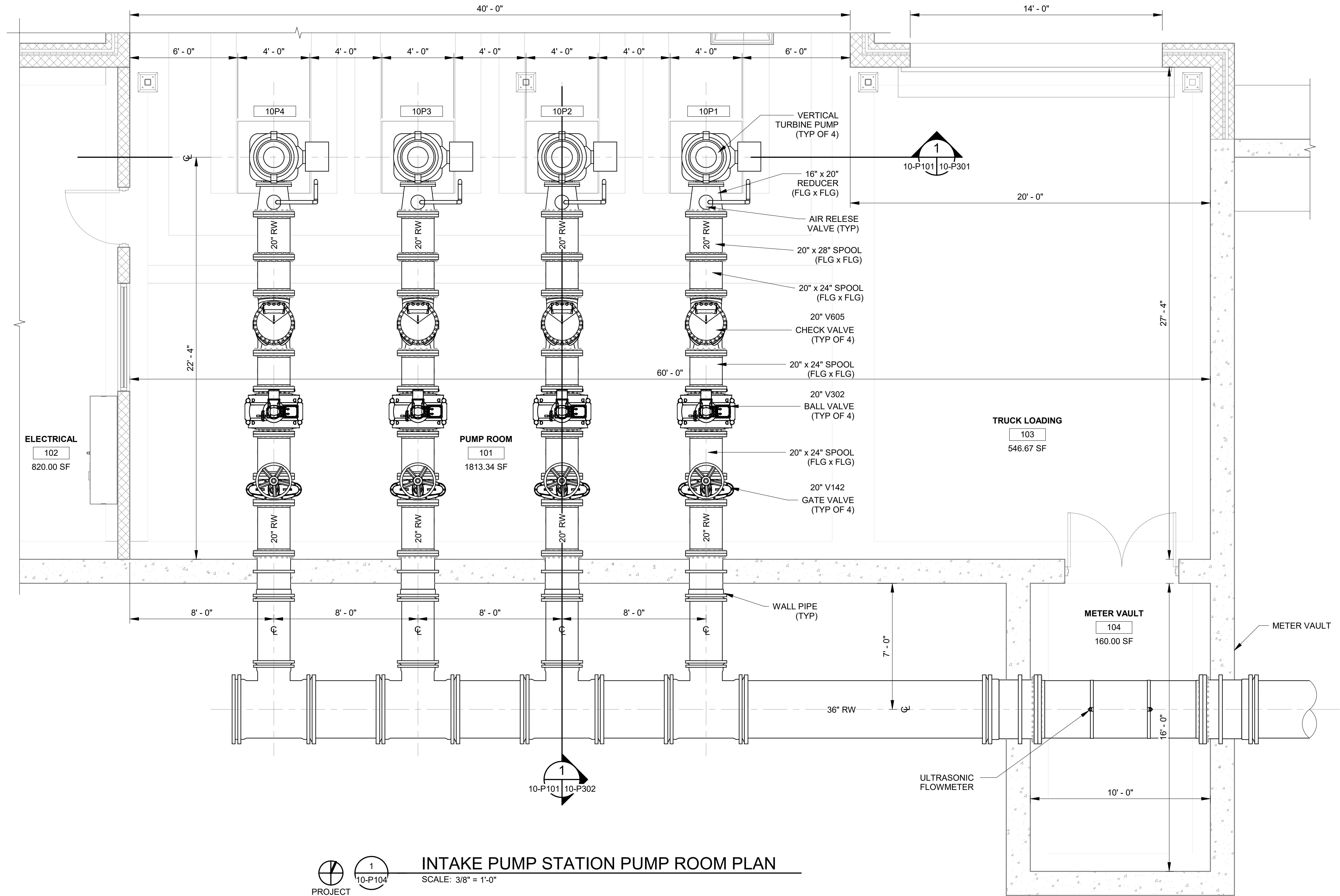
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DRAWING NUMBER

**10-P103**

SHEET  
NUMBER **21**

Revit File: BM 360/22W38095 - Coweta County Water MP/CowetaWTP\_INTAKE AND WETWELL.rvt  
Plot Date: 9/12/2022 3:52:39 PM



# INTAKE PUMP STATION PUMP ROOM PLAN

SCALE: 3/8" = 1'-0"



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COWETA COUNTY  
CHATTAHOOCHEE RIVER  
INTAKE, PUMP STATION  
AND RAW WATER PIPELINE

INTAKE PUMP  
STATION PUMP ROOM  
PLAN

JOB NO.: 22W38095  
DATE: SEPT. 2022  
DESIGNED BY: GWK  
DRAWN BY: EGB  
CHECKED BY: JAP

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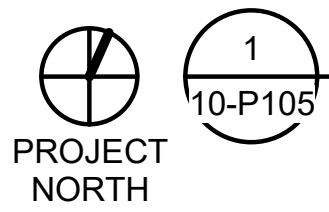
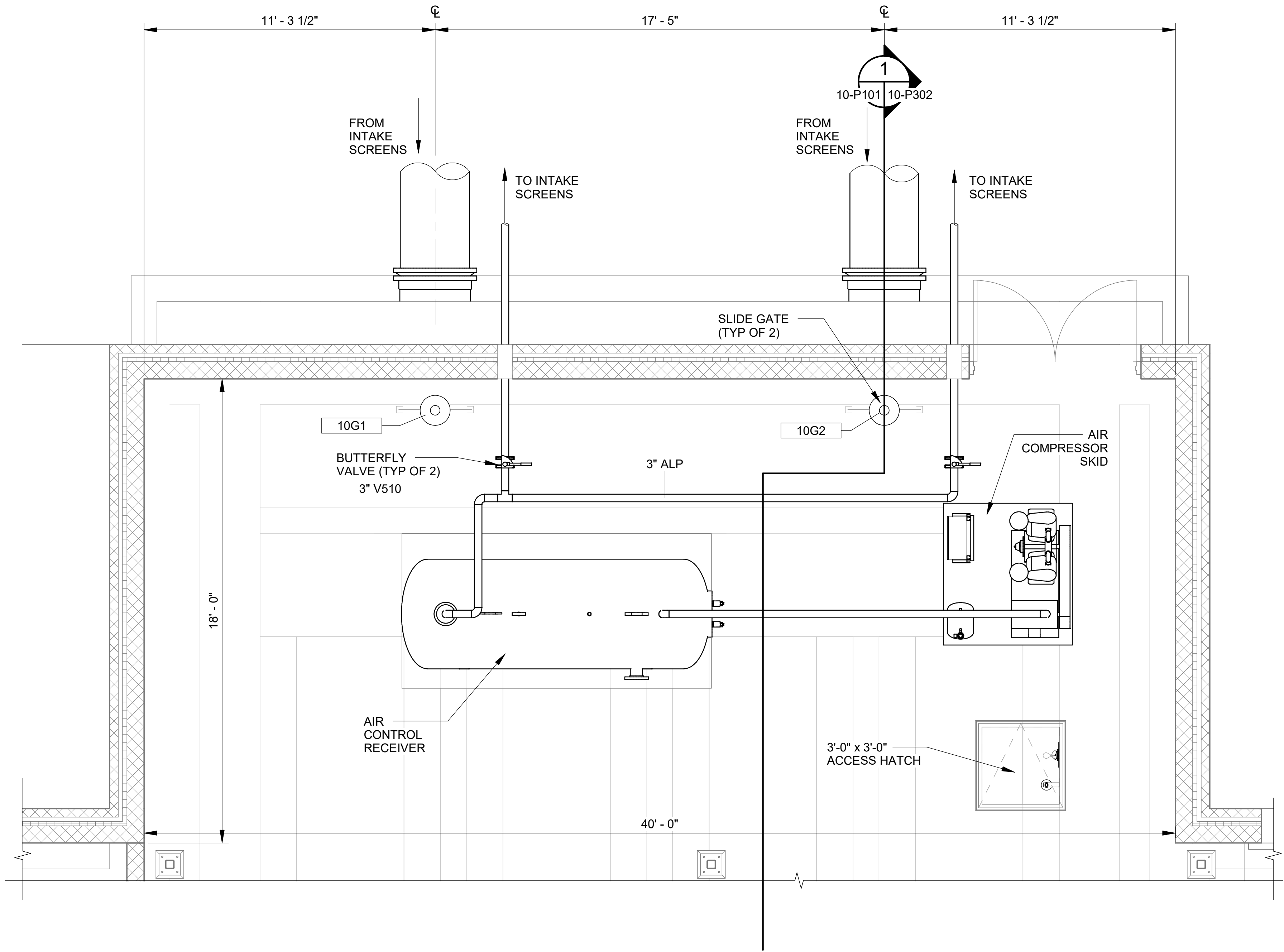
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**10-P104**

SHEET  
NUMBER **22**



Revit File: BM 360/22W38095 - Coweta County Water MP/CowetaWTP\_INTAKE AND WETWELL.rvt  
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PROJECT  
NORTH

## INTAKE PUMP STATION SURGE TANK PLAN

SCALE: 3/8" = 1'-0"



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CHATTahoochee RIVER  
INTAKE, PUMP STATION  
AND RAW WATER PIPELINE

INTAKE PUMP  
STATION SURGE  
TANK PLAN

JOB NO.: 22W38095  
DATE: SEPT. 2022  
DESIGNED BY: GWK  
DRAWN BY: EGB  
CHECKED BY: JAP

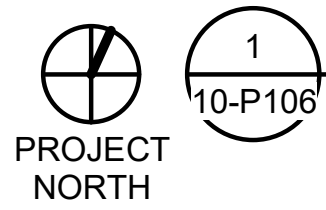
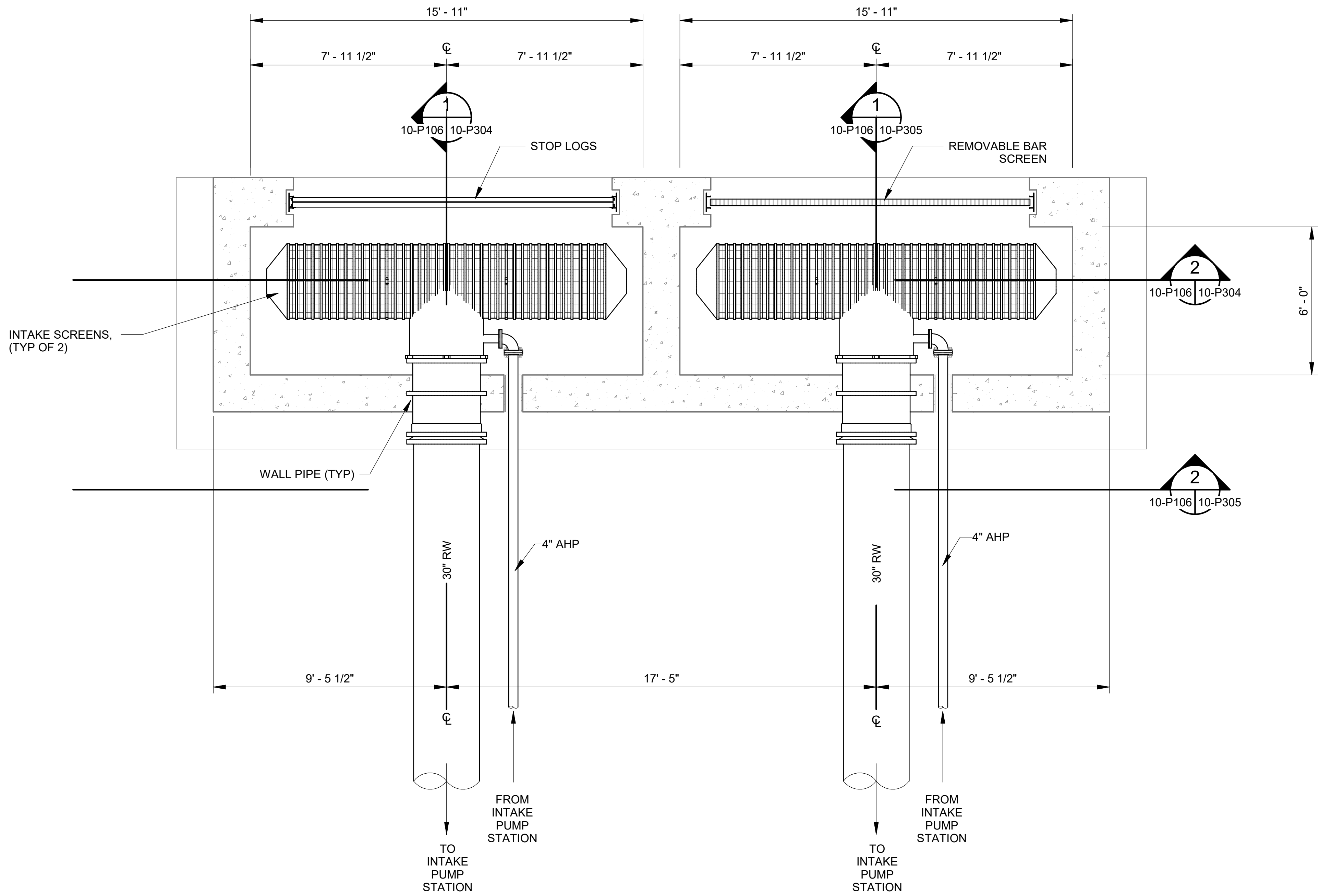
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DRAWING NUMBER

**10-P105**

SHEET  
NUMBER **23**

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**INTAKE SCREENING PLAN**

SCALE: 3/8" = 1'-0"



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CHATTahooCHEE RIVER  
INTAKE, PUMP STATION  
AND RAW WATER PIPELINE

INTAKE SCREENING  
PLAN

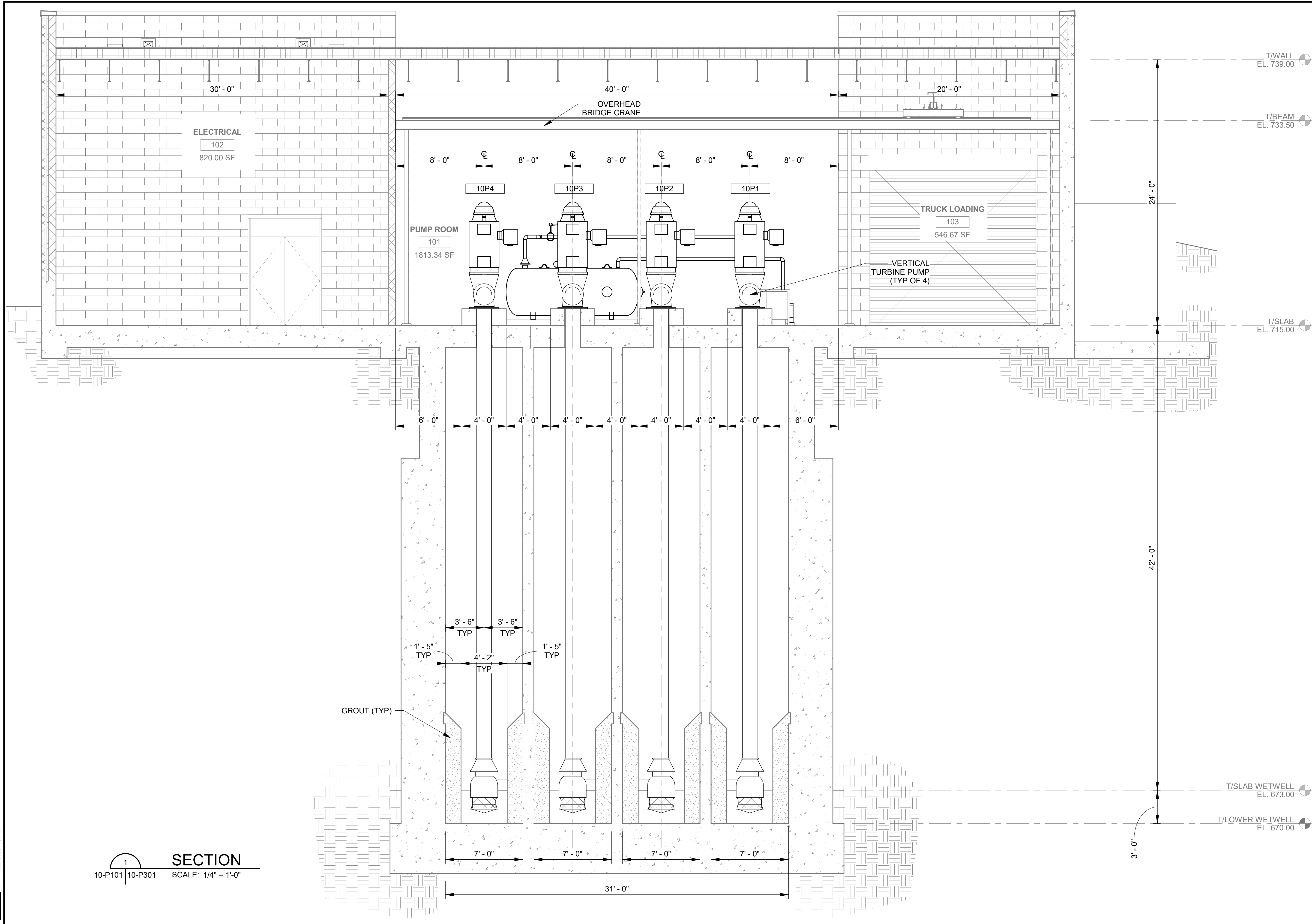
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DATE: SEPT. 2022  
DESIGNED BY: GWK  
DRAWN BY: JAS  
CHECKED BY: JAP

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DRAWING NUMBER  
**10-P106**  
SHEET  
NUMBER **24**



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CHATTahoochee RIVER  
INTAKE, PUMP STATION  
AND RAW WATER PIPELINE

INTAKE PUMP  
STATION SECTIONS 1

JOB NO.: 22W38095  
DATE: SEPT. 2022  
DESIGNED BY: GWK  
DRAWN BY: EGB  
CHECKED BY: JAP

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DRAWING NUMBER

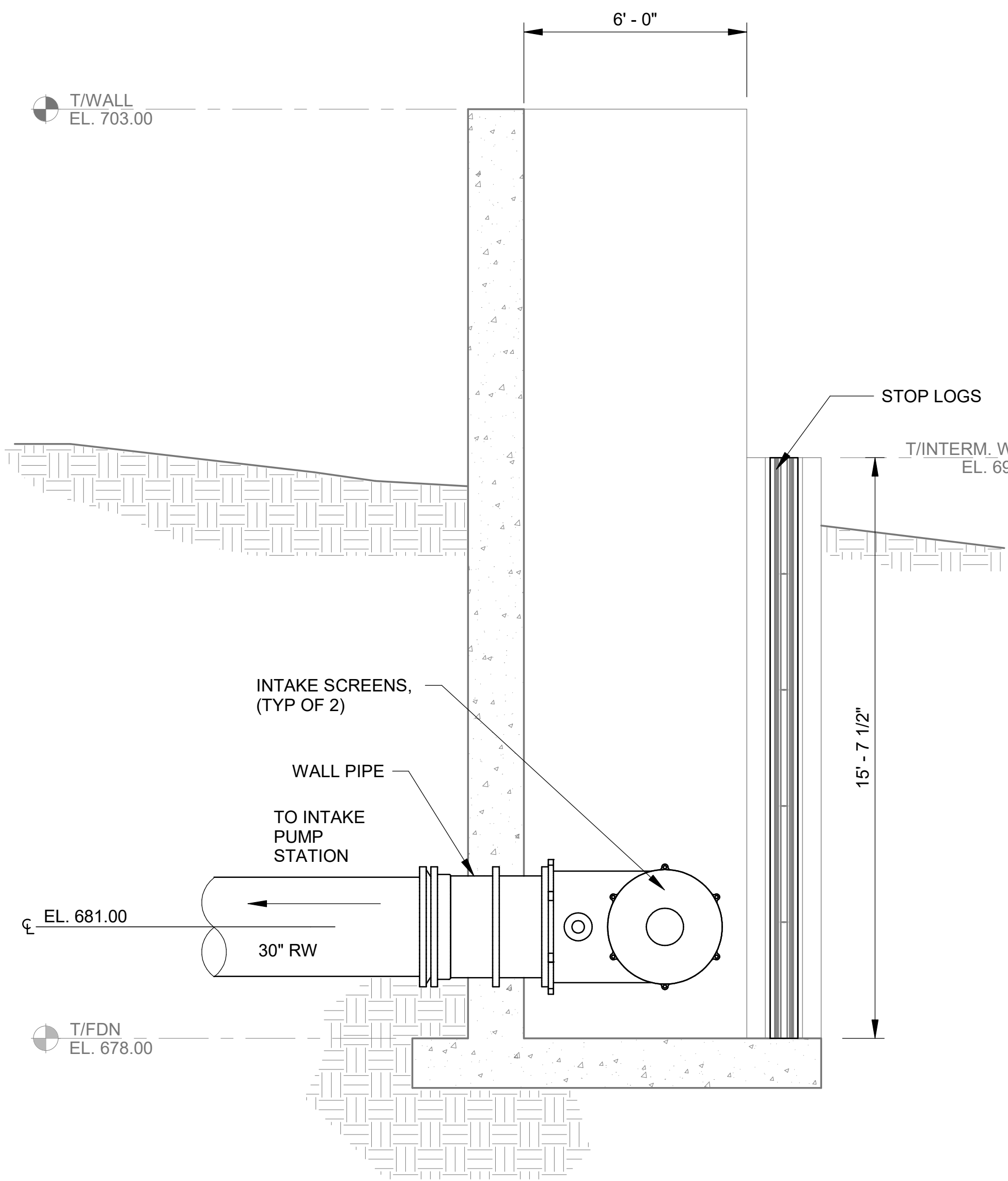
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SHEET  
NUMBER **25**

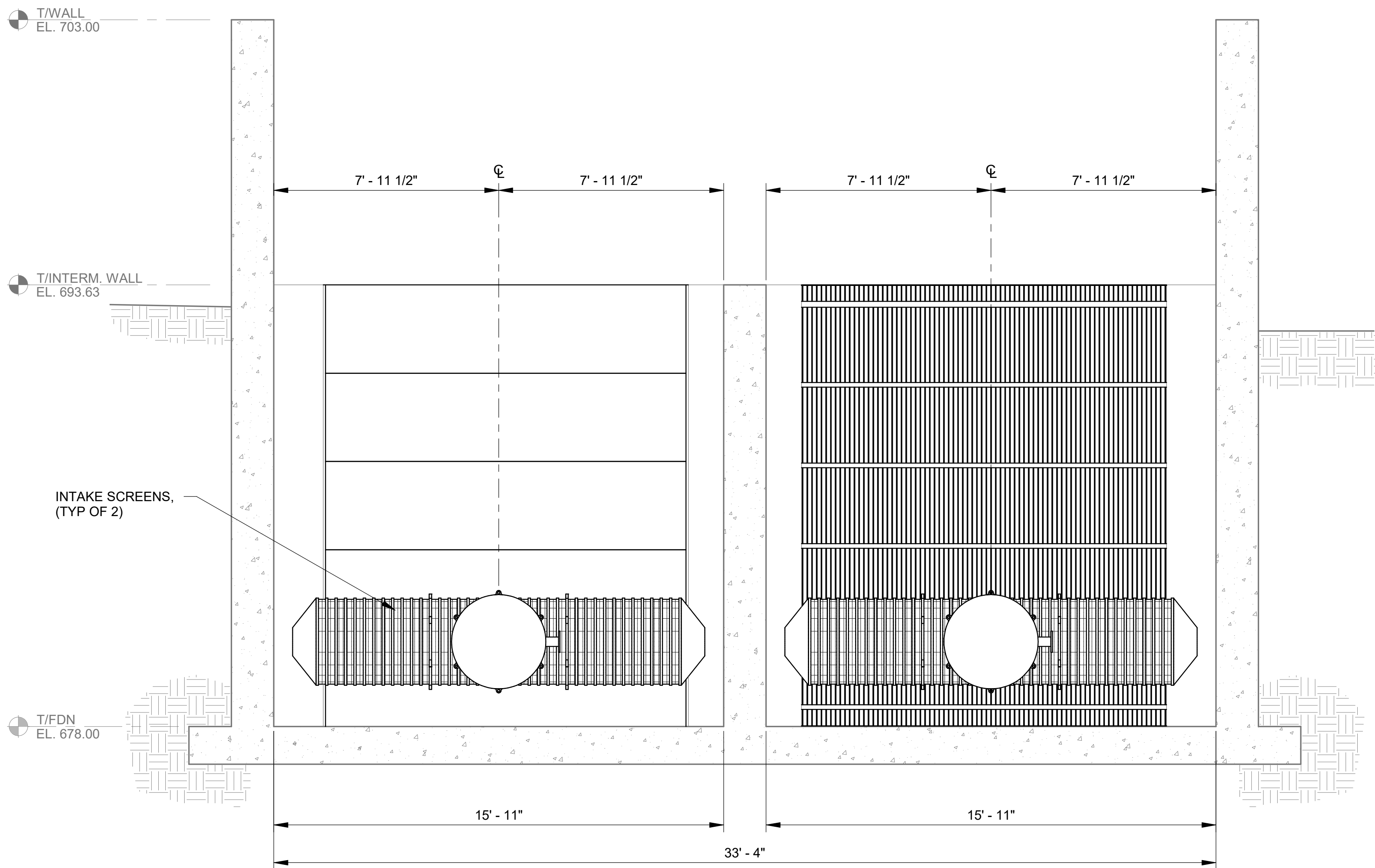




Revit File: BM 360/22W38095 - Coweta County Water MP/CowetaWTP\_Intake Screens.rvt  
Plot Date: 9/2/2022 3:03:04 PM



SECTION 1  
10-P106 | 10-P304  
SCALE: 3/8" = 1'-0"



SECTION 2  
10-P106 | 10-P304  
SCALE: 3/8" = 1'-0"



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INTAKE, PUMP STATION  
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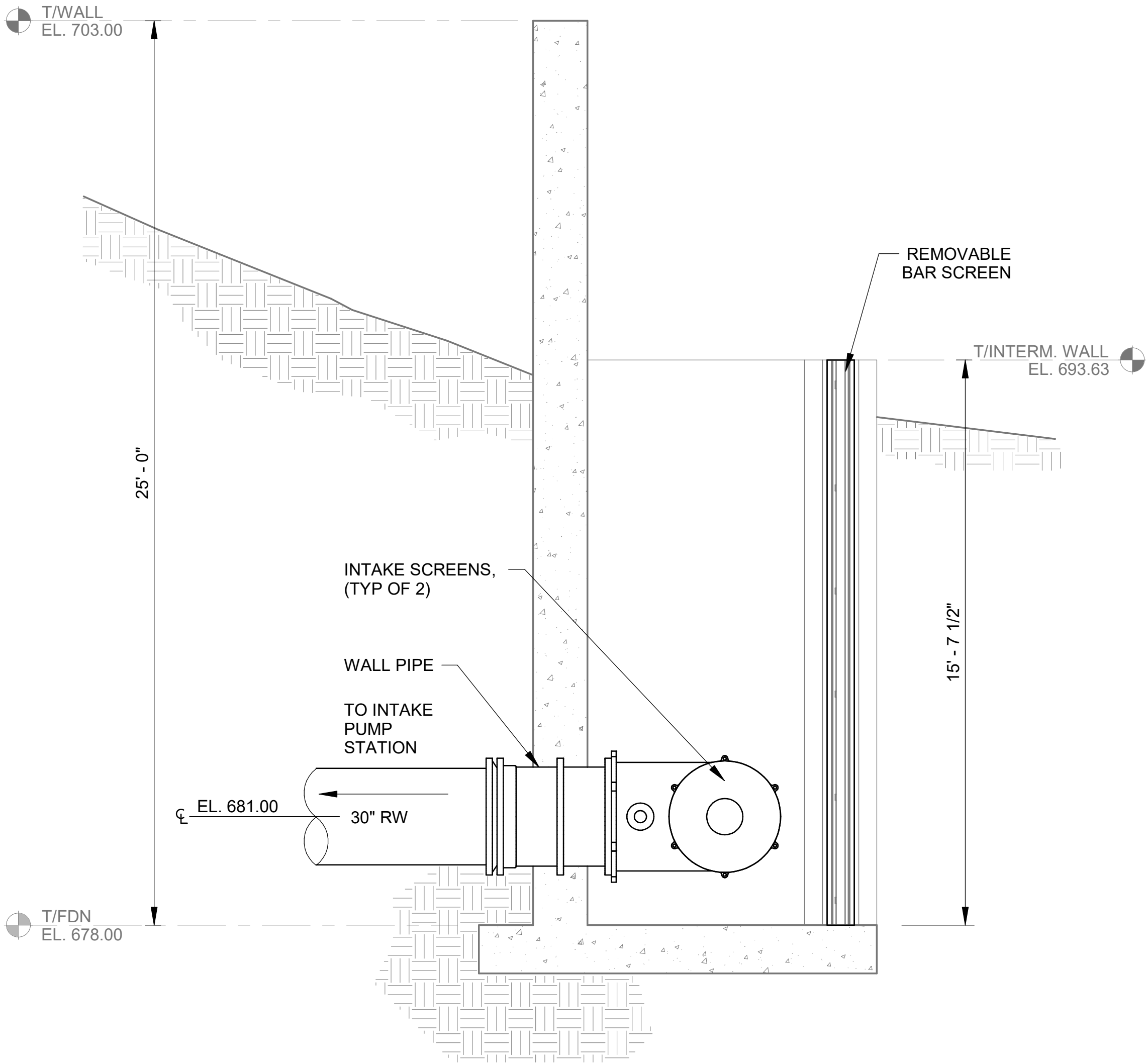
INTAKE SCREENING  
SECTIONS AND  
DETAILS 1

JOB NO.: 22W38095  
DATE: SEPT. 2022  
DESIGNED BY: GWK  
DRAWN BY: JAS  
CHECKED BY: JAP

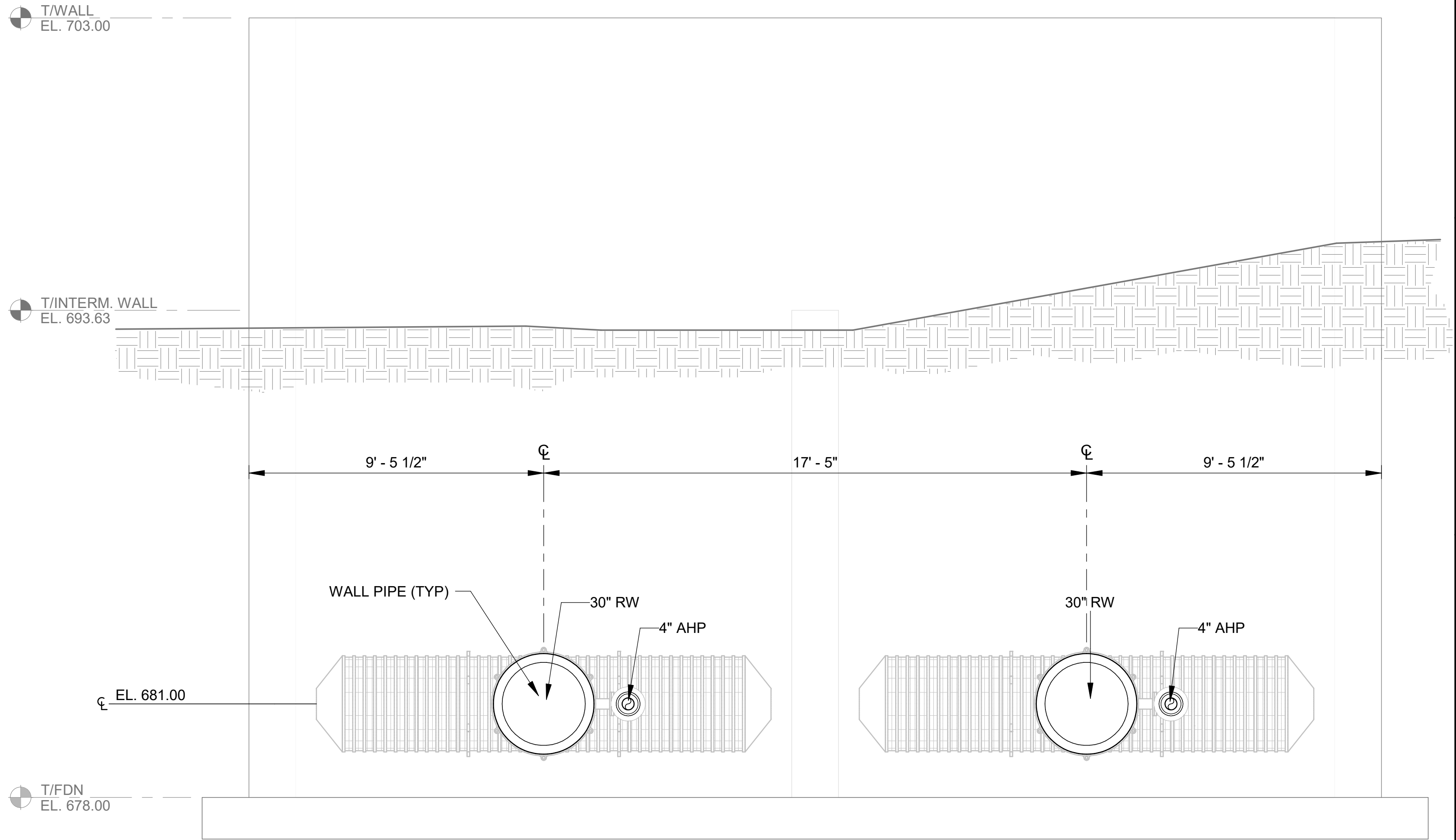
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DRAWING NUMBER  
**10-P304**  
SHEET  
NUMBER **27**

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Plot Date: 9/22/2022 3:03:05 PM



SECTION 1  
10-P106 | 10-P305  
SCALE: 3/8" = 1'-0"



SECTION 2  
10-P106 | 10-P305  
SCALE: 3/8" = 1'-0"



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CHATTahoochee RIVER  
INTAKE, PUMP STATION  
AND RAW WATER PIPELINE

INTAKE SCREENING  
SECTIONS AND  
DETAILS 2

JOB NO.: 22W38095  
DATE: SEPT. 2022  
DESIGNED BY: GWK  
DRAWN BY: EGB  
CHECKED BY: JAP

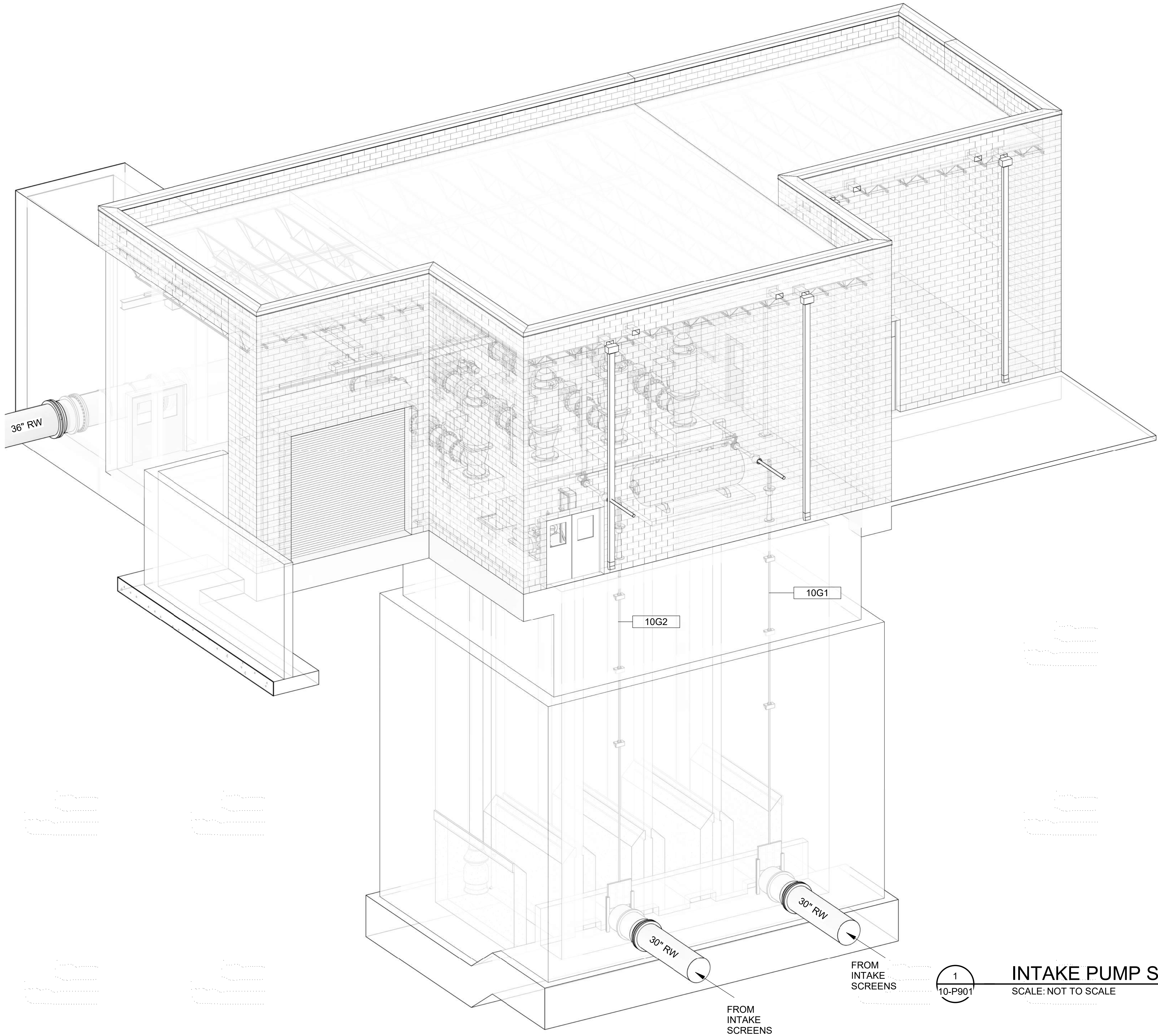
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DRAWING NUMBER  
**10-P305**

SHEET  
NUMBER **28**




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1  
10-P901

INTAKE PUMP STATION ISOMETRIC  
SCALE: NOT TO SCALE




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NEWNAN, GA

COWETA COUNTY  
CHATTahoochee RIVER  
INTAKE, PUMP STATION  
AND RAW WATER PIPELINE

INTAKE PUMP  
STATION ISOMETRIC  
1

JOB NO.: 22W38095  
DATE: SEPT. 2022  
DESIGNED BY: GWK  
DRAWN BY: EGB  
CHECKED BY: JAP

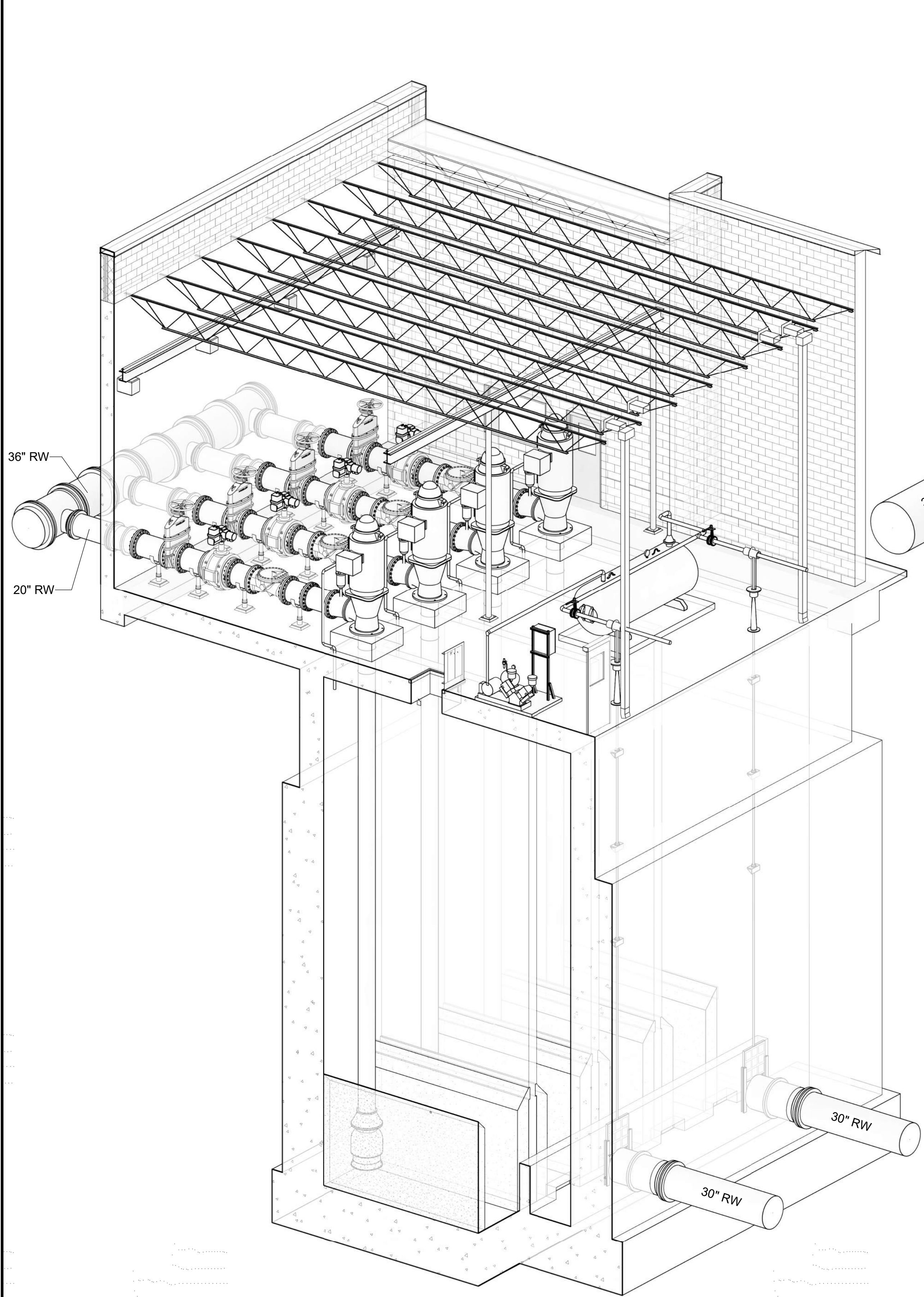
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DRAWING NUMBER  
**10-P901**

SHEET  
NUMBER **29**

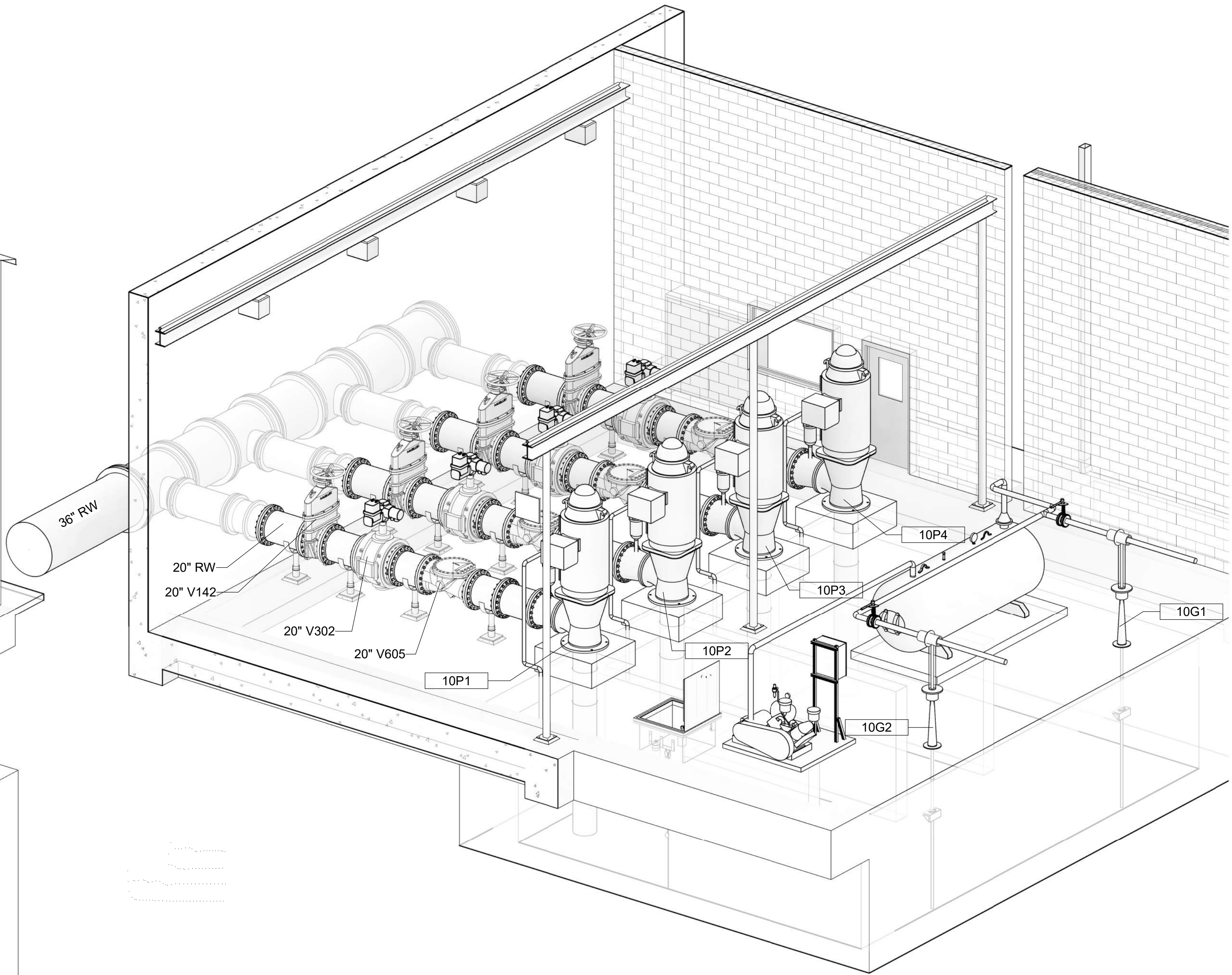


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1  
10-P902

**INTAKE PUMP STATION ISOMETRIC**  
SCALE: NOT TO SCALE



2  
10-P902

**INTAKE PUMP STATION ISOMETRIC**  
SCALE: NOT TO SCALE



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CHATTahoochee RIVER  
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AND RAW WATER PIPELINE

INTAKE PUMP  
STATION ISOMETRIC  
2

JOB NO.: 22W38095  
DATE: SEPT. 2022  
DESIGNED BY: GWK  
DRAWN BY: EGB  
CHECKED BY: JAP

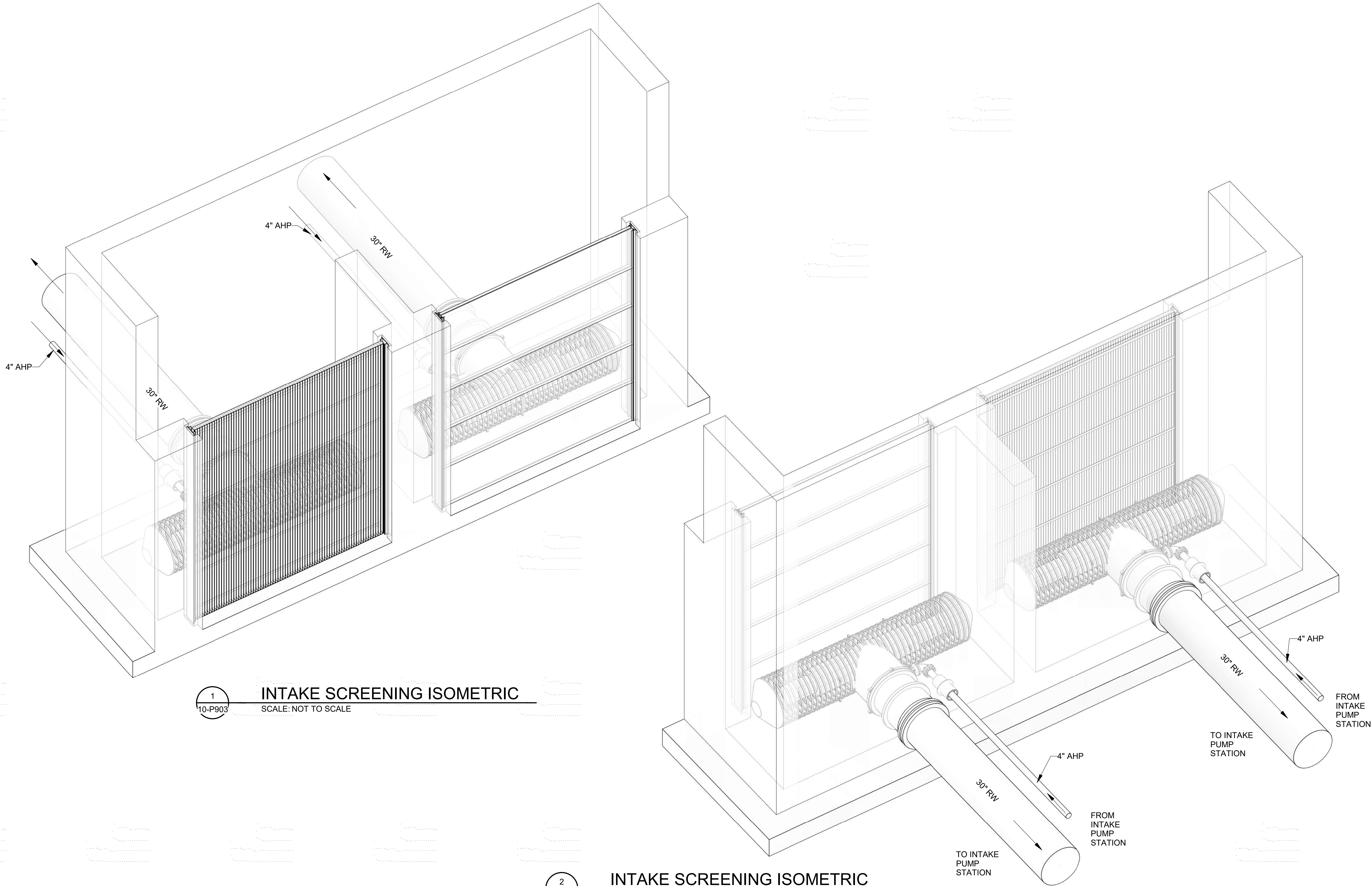
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DRAWING NUMBER  
**10-P902**

SHEET  
NUMBER **30**




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Plot Date: 9/22/2022 3:03:07 PM



1  
10-P903  
**INTAKE SCREENING ISOMETRIC**  
SCALE: NOT TO SCALE

2  
10-P903  
**INTAKE SCREENING ISOMETRIC**  
SCALE: NOT TO SCALE




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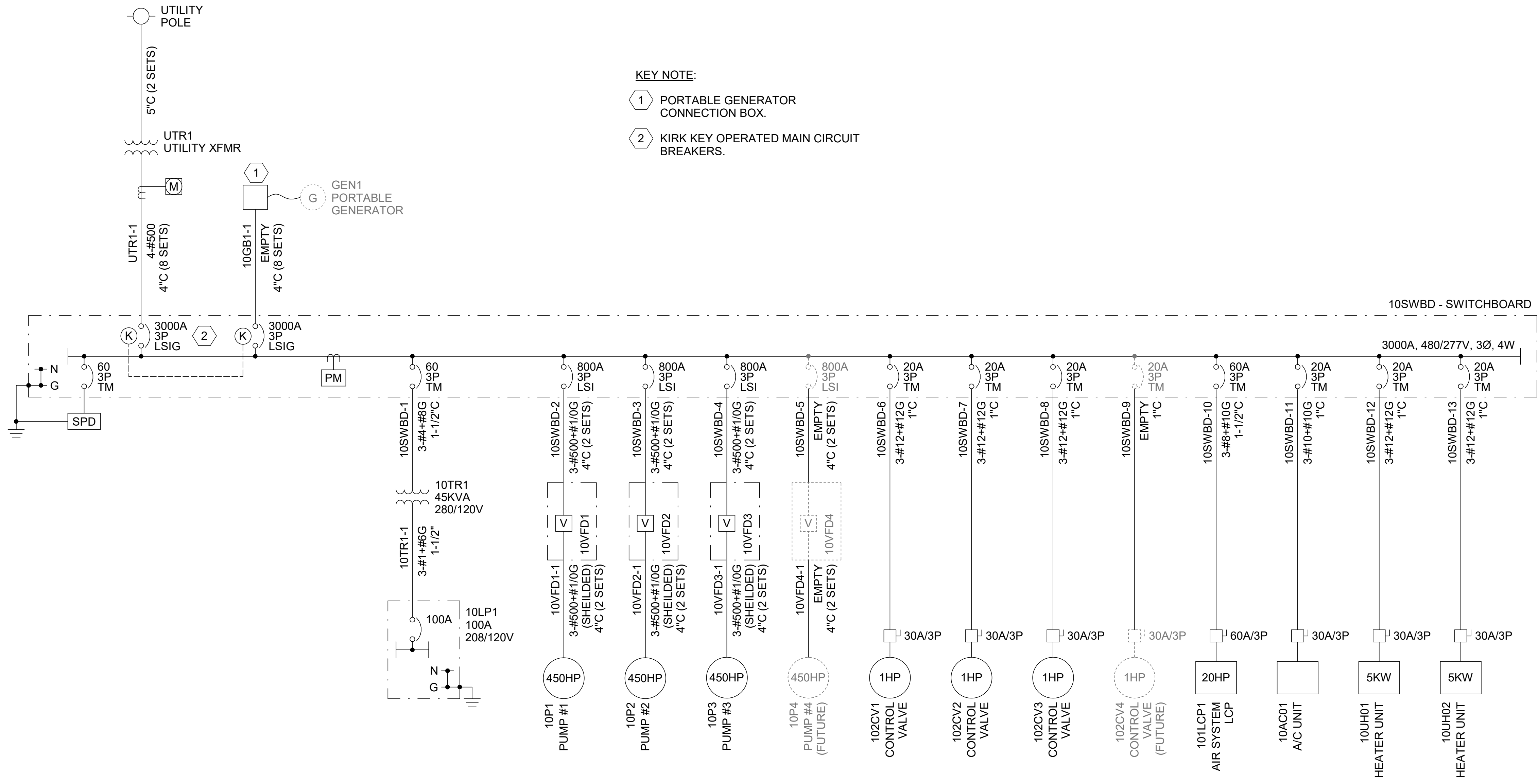
INTAKE SCREENING  
ISOMETRIC

JOB NO.: 22W38095  
DATE: SEPT. 2022  
DESIGNED BY: GWK  
DRAWN BY: JAS  
CHECKED BY: JAP

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DRAWING NUMBER  
**10-P903**

SHEET  
NUMBER **31**



KEY NOTE:

- 1 PORTABLE GENERATOR CONNECTION BOX.
- 2 KIRK KEY OPERATED MAIN CIRCUIT BREAKERS.

1  
10-E501

ONLINE DIAGRAM - 21 MGD  
SCALE: NOT TO SCALE



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COWETA COUNTY  
CHATTahooCHEE RIVER  
INTAKE, PUMP STATION  
AND RAW WATER PIPELINE

ONLINE DIAGRAM -  
21 MGD

JOB NO.: 22W38095  
DATE: SEPT. 2022  
DESIGNED BY: JCW  
DRAWN BY: WBW  
CHECKED BY: JAP

BAR IS ONE INCH ON ORIGINAL DRAWING  
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DRAWING NUMBER

10-E501

SHEET NUMBER 32